

**2013 ANNUAL MEETING SUMMARY**  
**APDES Permit No. AKS-052558**  
**February 25, 2013**

<p>Municipality of Anchorage</p>  <p>Daniel A. Sullivan, Mayor</p>	<p>Alaska Department of Transportation and Public Facilities</p> 
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Watershed Management Services  
Project Management and Engineering Department  
Municipality of Anchorage



# Meeting Agenda

# 2013 Watershed Update

## Final Agenda

Monday, February 25, 2013 Municipality  
of Anchorage  
Alaska Department of Transportation and Public Facilities  
At the BP Energy Center

**Arrival:** 9:15 – 9:30, coffee provided, program will start promptly at 9:30.

**Welcome** Municipality of Anchorage and Alaska Department of Transportation

**Opening Remarks** Ron Thompson, P.E., Municipal Public Works Director

**Program** APDES Storm Water Permit Compliance

Poster Session of Projects

- Monitoring
  - Wet Weather Monitoring
  - Dry Weather Monitoring
  - OGS/Sed Basin Assessment
- LID
- Mapping
- Education

**Q&A** Panel Discussion – project team will be available to address questions

Adjourn

**Afternoon Training** Low Impact Development – Making it Work in Anchorage.  
For those of you signed up to attend this workshop it will begin at 1:00 p.m. at the BP Energy Center.

## Program Slides

# *2013 Watershed Update*

**Municipality of Anchorage**

**Alaska Department of Transportation and Public Facilities**

*A.laska  
P.ollutant  
D.ischarge  
E.limination  
S.ystem*

*Welcome to the APDES Annual  
Meeting!*

**Jerry Hansen**

Municipality of Anchorage

Project Management and Engineering

# *Opening Remarks*

- **Ron Thompson, P.E.**

Municipality of Anchorage

Director of Public Works

# *Today's Program*

## APDES Storm Water Permit Compliance Poster Session of Projects

Monitoring - Wet and Dry Weather Monitoring

Assessment - OGS/Sediment Basin

Low Impact Development - State and Municipal Projects

Mapping

Education

Q&A Session

## *New in 2013*

### APDES Storm Water Permit Compliance

- Runoff Reduction Techniques for Road and Parking Lot Repair (publicly owned or managed)
- Inspection and Enforcement of Permanent Stormwater Controls

# *Today's Program*

## Poster Session of Projects and Refreshments

Monitoring - Wet and Dry Weather Monitoring

Assessment - OGS/Sediment Basin

Low Impact Development - State/Municipal Projects

Mapping – State/Municipal Storm Sewer System

Public Education

Q&A Session ~11:00



# *Q & A Discussion*

Anchorage MS4 Permit

## Posters

# The Municipality of Anchorage - Low Impact Development Projects

## Porous Asphalt and Infiltration Russian Jack Springs Parking Lot

### Project Objectives:

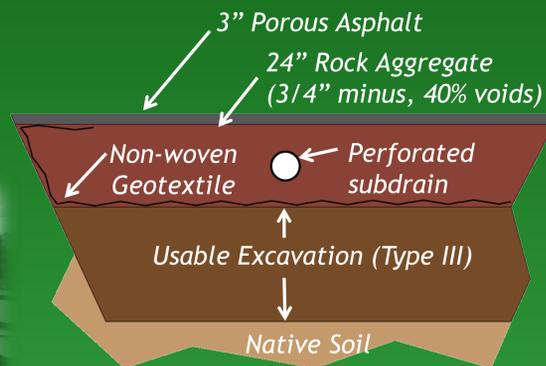
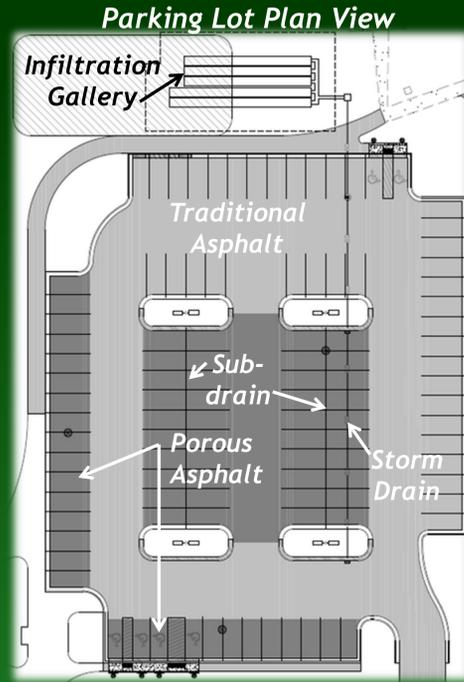
- ~ Improved and safer parking
- ~ Safer pedestrian access around the parking lot
- ~ Manage stormwater using Low Impact Development techniques

### LID Features:

- ~ Porous Asphalt
- ~ Infiltration Gallery

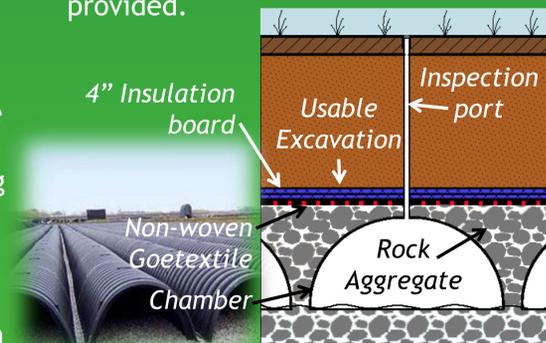
## Porous Asphalt

Porous asphalt was used in combination with traditional asphalt to collect stormwater that falls onto the parking lot. The porous asphalt locations were selected based on coordination with the MOA Parks and Recreation maintenance crew. Because this was the first porous asphalt of its kind in Anchorage, it was placed in locations of low winter use where it would not be regularly plowed and sanded.



## Infiltration Gallery

Excess water not collected and infiltrated by the porous asphalt was directed to a subsurface infiltration gallery. To promote longevity of the facility, a pretreatment OGS was provided.



## Limitations - Native Soils

The characteristics of the native soils varied significantly with depth. Under the porous asphalt, native material was slow-draining with percolation rates generally less than 0.5 in/hr. Because the rainfall is distributed over a large area and because the rock aggregate provides storage capacity, porous asphalt works well even with poorly draining soils.

The bottom of the infiltration gallery was deeper than the porous asphalt subgrade. At this depth, native material was granular and well-draining with percolation rates of approximately 6 to 8 in/hr.

## Rain Garden

### Taku Lake Parking Lot

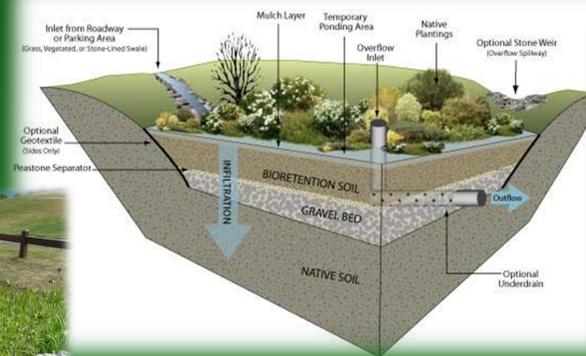
The Taku Lake Rain Garden was constructed to accept and treat stormwater runoff from the Taku Lake parking area and a portion of King Street. The rain garden collects stormwater and provides treatment and retention through plant uptake, top soil saturation, and infiltration. Excess water is collected in a perforated subdrain which outlets near Taku Lake. Preliminary monitoring results show that this pipe only flows under heavy rainfall conditions despite that fact that groundwater in this area is near the surface. This rain garden is functioning very well.

### Project Objectives:

- ~ Water quality treatment of runoff
- ~ Stormwater Retention and Infiltration
- ~ Bioretention (Rain Garden)

### LID Features:

- ~ Bioretention (Rain Garden)



Typical rain garden section  
(Photo from Douglas County, WA)



## Bioretention

The MOA has completed several large and small scale stormwater bioretention areas at commercial, school, and residential sites across Anchorage. The size and design of the bioretention areas vary depending on the intended function, size of the contributing drainage area, and the surface the runoff comes from (parking lots, rooftops, etc.) In each case, the bioretention areas provide an opportunity for stormwater cleaning, infiltration, and storage. At the same time, the areas add aesthetic appeal!



Left and Right: These residential rain gardens are collecting stormwater from roof downspouts and lawns.



Left: This commercial bioretention area is collecting runoff from a parking lot of a local business. The area is designed with an overflow inlet for large rainfall events.



# Dry Weather and Wet Weather Sampling



## Dry Weather Screening:

A monitoring program where samples are collected during dry weather (typically May and June) from outfalls that flow directly into creeks. The objective is to measure indicators of pollutants to compare with thresholds to target outfalls with potentially on-going illicit discharges for follow-up action.

A total of 15 outfalls are sampled each year for:

Parameter	Threshold
pH	≤ 4 or ≥ 9 STD
Total Chlorine	≥ 1.0 mg/L
Detergents	≥ 1.0 mg/L
Total Copper	≥ 1.0 mg/L
Total Phenols	≥ 0.5 mg/L
Turbidity	≥ 250 NTU
Fecal Coliform	≥ 400 cfu/100 mL

When a parameter exceeds the above threshold follow up sampling occurs.

In 2011, Fifteen outfalls were sampled in the following Watersheds:

- Fish Creek
- Campbell Creek
- Eagle River

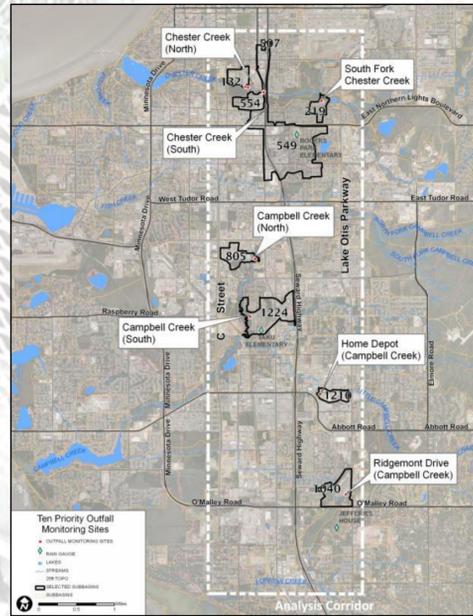
Results of sampling found one outfall on Campbell Creek clogged with sediments and exceeded the turbidity criteria. The outfall was cleaned and resampled and passed testing requirements.

In 2012 outfalls were sampled in the following watersheds:

- Ship Creek
- Chester Creek
- Furrow Creek

Sampling results showed an exceedance for fecal coliform at an outfall on Ship Creek.

- First Round Sampling Result: 76,400 colonies/100 mL
- Follow Up Testing Results: 754 colonies/100 mL
- Follow Up Testing at nearest up gradient manhole Result: 29 colonies/100 mL
  - During follow up sampling the outfall was submerged due to high tide. Sampling was performed after the tide receded.
  - It is likely that the source of fecal coliform is from high tide washing material into the outfall.



Wet Weather Sampling Locations



2011 Campbell Creek Outfall



2012 Ship Creek Outfall

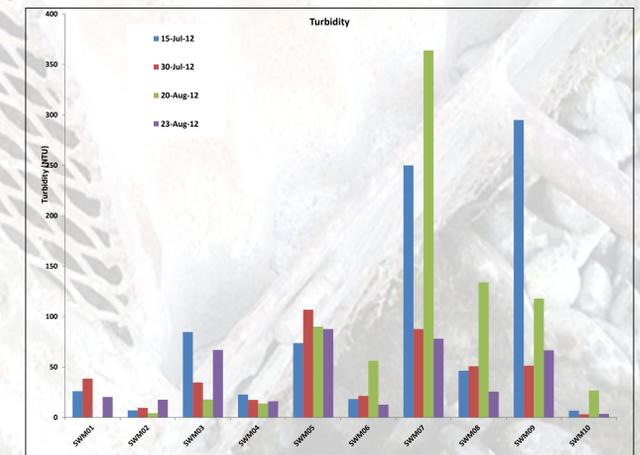
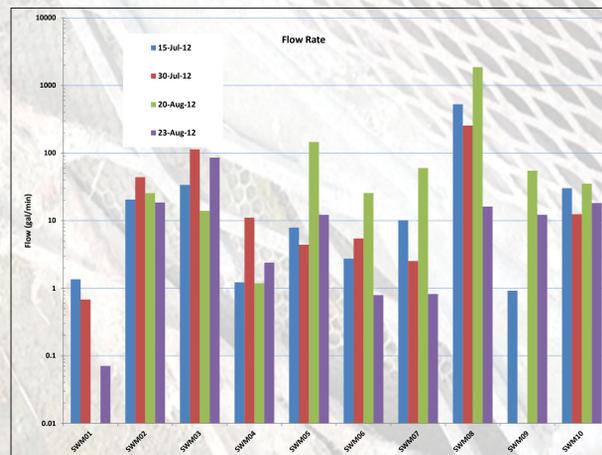
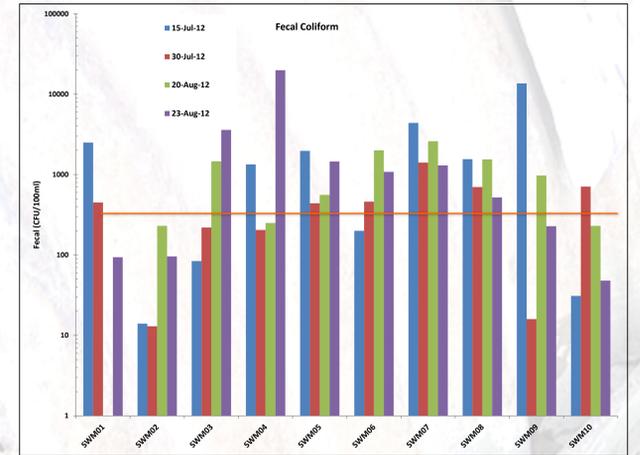
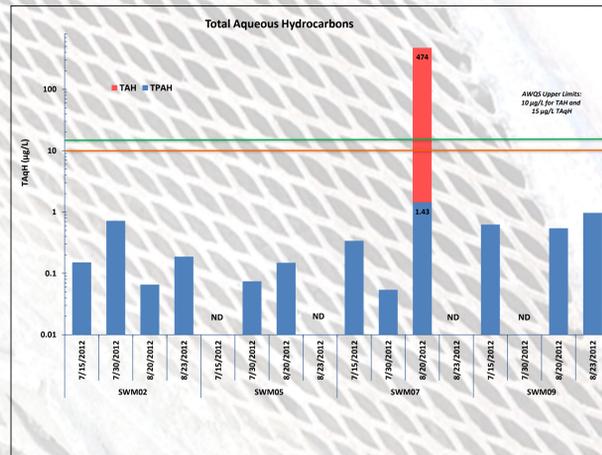
## Wet Weather Sampling:

A monitoring program conducted during four storms (>0.1 inches) at ten outfalls to determine pollutant wash off from streets and parking lots into the MS4. Parameters sampled are:

Parameters	Frequency
Flow	4 times/year
DO	4 times/year
pH	4 times/year
Turbidity	4 times/year
Temperature	4 times/year
BOD <sub>5</sub>	4 times/year
Fecal Coliform	4 times/year
TSS	4 times/year
TAH	4 times/year
TAqH	4 times/year



Some results from 2012 are as follows:



Sampling will continue in 2013 and 2014. In 2014, loading estimates, comparisons between basins, and other analysis will be completed using 4 years of data.



Findings from either 2011 or 2012 suggest there is no need for any special investigations. Except for high TSS/turbidity seen at one location in 2011, and one occurrence of high hydrocarbons in 2012, concentrations of target constituents in the grab samples and in the field measurements are within the range of expected values. Although fecal sampled data was higher than Alaska Water Quality Standards, the AWQS is used as a comparison only until there is enough data to determine trends and does not directly apply to storm water.



# Sedimentation Basin and OGS Efficiency Study

Scott R Wheaton, WMS; Bill Spencer, P.E., HDR Alaska; Cynthia Milligan, HDR Alaska; Jacques Annandale, EIT; HDR Alaska



**All storm water controls work in series along the length of the MS4. Street sediment load, street sweeping practices, and catch basin conditions control the performance of the system at the upstream end. OGS and Sedimentation Basins affect the downstream performance. All devices are part of a treatment train that must be considered as a whole in context with Anchorage's meteorological environment.**



## 1. Storm Runoff:

Rainfall runoff occurs in Anchorage typically from May to October. Storm events increase in occurrence and intensity towards the fall.

Snowmelt runoff occurs in a single seasonal event three to six weeks in length. Snowmelt runoff is generally diurnal early in the season and becomes continuous towards the end of the event.

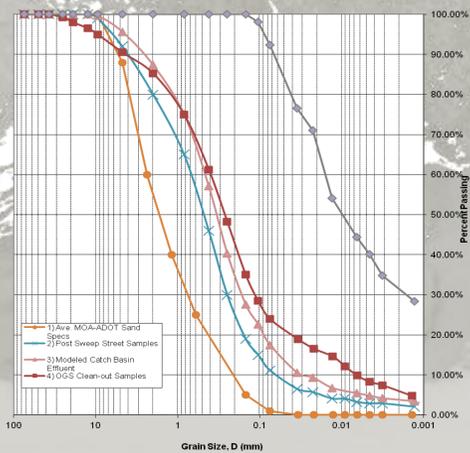
The following are statistics from historic and 2012 rainfall data:

	Historic (1963-2010) Rainfall Statistics	2012 Rainfall Statistics
Mean Storm Volume inches	0.24	0.34
Mean Storm Intensity inches/hour	0.026	0.028
Mean Storm Duration hours	13.17	24.48
Separation time (dry hours between storms)	79	88
90 percent intensity inches/hour	0.12	0.08
Annual number of storms volume >.02 inches	40	29

## 2. Street Sediment Loading and Washoff:

Data suggests that a larger sediment load washes off Anchorage Streets during the summer rainfall season than during the snowmelt season. Although street sediment loads are greater during spring snow melt, the higher flow rates and sediment availability found during summer storms lead to greater wash off during the summer.

Modeling of street washoff suggest that most of the street sediments left after spring street sweeping are washed into the storm system during summer storms.



The graph to the right shows a series of particle size distributions (PSD) that follows the change in character of the sediment load as it moves through the system and components are captured by various devices.

## 4. OGS<sub>h</sub> 2012 Performance:

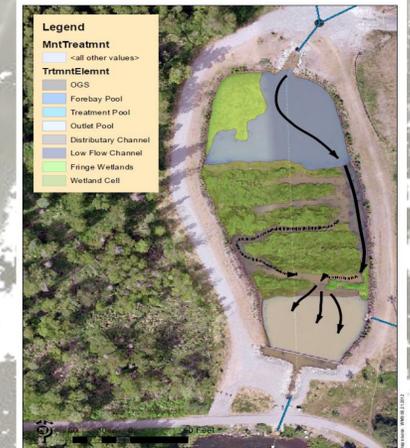
Oil and grit separators are installed into the treatment train to capture sediments, oil, and floatables in the MS4.

This study analyzed and evaluated Anchorage hydrodynamic oil/grit separators (OGS<sub>h</sub>) through field sampling of in-place devices and full-scale benchtop testing.

Accumulated sediment from four OGS were sampled for the following :

- Volumetric measurements
- Particle Size Distribution
- Total Organic Content
- Information on accumulation interval

OGS Basin	Basin Area (sq. ft)	Total Curb miles in basin	Basin Type	OGS Unit Model	Time since last cleaning (years)	Estimated weight of sediments (lb)	Percent passing #200 sieve (75 micron)	Organic Content of sediments
Old Seward and 74 <sup>th</sup> Ave	770,000	.82	Arterial	STC 3600	0.70	1656 lb	10%	3.9%
Juneau Street N. End	4,568,000	7.17	Residential	STC 13000	1	9034 lb	33%	20.7%
Tudor Rd West of Lake Otis	400,000	.57	6 lane arterial with divider	STC900	1.85	1016 lb	17.2	4.4%
Mears Middle School	447,600	1.07	School Parking area	Vault	0.85	602 lb	34.9	9%



## 3. Catch basins:

As the first treatment device in the treatment train, properly designed and maintained inlet catch basins can be very effective at treating headwater-mobilized particulates (40% reduction in the total storm water particulate load). This study's observations show that performance of these devices is directly related to their design geometry and maintenance practices. To perform optimally catch basins must have:

- Minimum spacing between catch basins at off-line locations
- Sump geometries designed to allow sediment settling and storage
- Schedule maintenance to remove accumulated sediments



## 5. Sedimentation Basin 2012 Performance:

Sedimentation basins are installed in the treatment train as a way to capture finer sediment. This study evaluated the sedimentation basins with a modeling effort and to capture real data for comparison.

Sedimentation basin performance was modeled on a sum-of-loads approach and then related to a range of design factors through storm-by-storm analysis of basin hydraulic efficiencies.

Weirs and continuous gages were installed at the inlet and outlet of three sedimentation basins: C Street, Minnesota Street, and Meadow Street. Measurements were taken every 15 minutes for flow, electrical conductivity, and turbidity. During storm events grab samples were collected for TSS, Fecal Coliform, and BOD. The parameters of pH and DO were collected onsite with a YSI 556 multiprobe or equivalent probe. Petroleum organics were collected using passive collection devices.

Data collected during 2012 was then taken and placed into removal model formulas to determine removal efficiencies and compared to the sum-of-loads model. The following are the results:

## Benchtop OGS<sub>h</sub> Test:

This study tested a full-scale hydrodynamic OGS using Anchorage street sediment.

Results from the benchtop test suggested very high removal rates for the OGS under Anchorage conditions of flow and street sediment character. Removal of >40% of 20 microns particles is attainable at flow rates equal to 90% of Anchorage storm runoff.

Inch/Sieve size	Particle Size		OGS Removal Efficiency
	Inch/Sieve size	Microns	
#100+	149+		100.00%
#140	105		95.50%
#200	75		86.60%
	35.2		72.70%
	22.4		48.46%
	13.1		21.67%
	6.6		12.31%
	4.6		9.80%
	3.2		5.29%
	1.3		2.29%

	2012 SEDIMENTATION BASIN LOADING									
	C StreetBasin			Minnesota Basin			Meadow Street Basin			
	C st In	C st Out	% capture	Minn In	Minn Out	% capture	Meadw In	Meadw Out	% capture	
Spring	5.3	2.9	45	8.6	5.7	68%	1.4	1.2	16%	
Summer/Fall	13.5	4.6	66%	6.8	3.8	45%	2.9	2.3	20%	
Sedimentation Basin Model Performance										
	Units	C Street Basin			Minnesota			Meadows		
2012 Calculated Removal Efficiency	%	71.29%			53.48%			28.92%		
2012 Sum of Loads Measured Removal	%	66%			45%			20%		

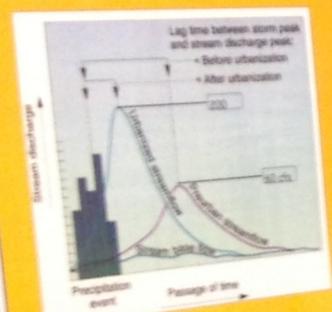
## Conclusions and Recommendations:

Recommendations for planning and design strategies of MS4 treatment devices are::

- Plan and design all water quality controls within a treatment train context.
- Design for and assess performance using seasonal sum-of-loads methods.
- Apply water quality design storms appropriate to Anchorage
- Apply 90<sup>th</sup> percentile rainfall intensity and waste storage criteria to OGS design.
- Identify and implement practicable maintenance SOPs to support designs.

# Water and Development

## Hydrographs!

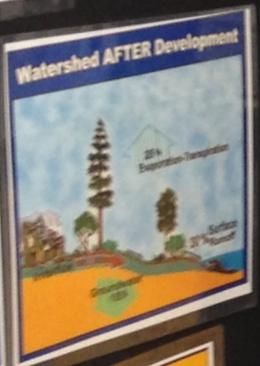


A hydrograph shows flow in a stream, on the vertical axis, with respect to time, on the horizontal.

- Think...
- How does development affect stream flow rates?
  - When more peak and steeper discharge peak are shown together, what can happen?
  - What can we do to mitigate impacts from human development?

### Water Cycle Changes

Hydrologic Variable	BEFORE	AFTER
Surface Runoff	45%	70%
Groundwater Infiltration	37%	25%
Infiltration	20%	30%
Evapotranspiration	38%	25%



### Contaminants in Urban Runoff

Water running off impervious surfaces like asphalt, concrete, roofs, sidewalks, fertilizers and other contaminants go in flow through the storm drain system to the creek.

Some contaminants include:

- Bacteria (from toilets)
- Polynuclear aromatic hydrocarbons (from gasoline)
- Metals and copper (from cars and trucks)
- Oil (from gas stations, parking lots, cars)
- Lead (from the water)
- Fertilizers (from lawns and gardens)
- Pesticides

# Help clean our water

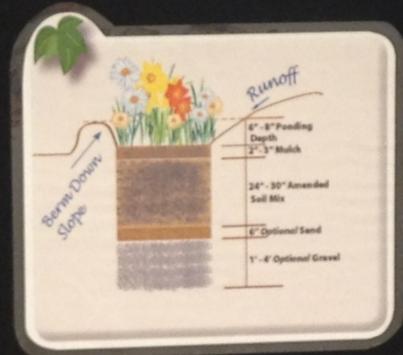
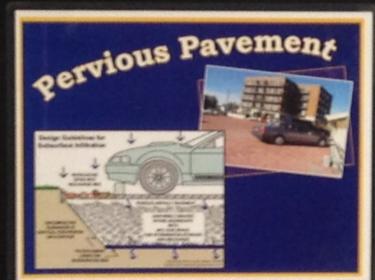


### What is a Rain Garden?

- A Rain Garden is a facility to clean, retain, and utilize storm water runoff. When it rains water runs off hard surfaces and soaks into the rain garden.
- You can recognize a rain garden by its flat or dish shaped surface, native plant coverage, and storm water runoff sources nearby.
- Rain gardens are filled with hardy native plants, sandy soil for good drainage and are sized to fit the amount of runoff that will drain to them.

Think!

How does infiltration and retention relate to stream peak flow?



### What is Pervious Pavement?

- Pervious Pavement is a hard drivable surface that allows rain to flow vertically into the ground where it falls.
- Prevents runoff, provides water treatment, and promotes ground water recharge through infiltration.
- Counteracts typical impacts from pavement.

Did you know...? A new pervious asphalt parking lot and playground are being built at Rasmus Park Spring Park Ball Field this summer. More information available.

# How You Can Help

### Grant Money

Do you live in the Municipality of Anchorage? You could qualify for a Rain Garden Grant!

- Reimbursement for 50% of your cost up to \$750
- One-on-one consultation for every garden
- Trained Landscapers and Nurseries



### Rain Gardens in Anchorage

In the last 4 years, the Rain Garden Program has supported over 50 rain gardens in the Municipality.

### Getting Started

Add your name to the list!

Take a grant application!

### Hydrography Features Mapping Logic



**Watersheds**  
Watershed boundaries represent the area of land that drains into a specific water body.



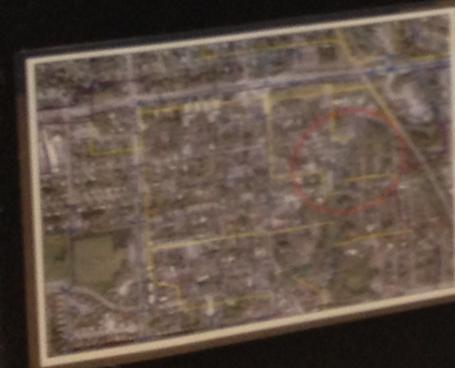
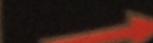
**Subbasins**  
Subbasins are smaller areas within a watershed that drain into a common outlet.



**Drainage Maps**  
Drainage maps show the flow paths of water across a landscape, from individual raindrops to larger water bodies.



**Primary Discharge Points**  
Primary discharge points are locations where water from a watershed or subbasin flows out to a larger water body.



## APDES MAPPING

[www.anchoragestormwater.com/maps.html](http://www.anchoragestormwater.com/maps.html)

#### Data Downloads

##### GIS DATA

- HYDROGRAPHY
  - GEODATABASE (HGDB)
  - SHAPEFILES
  - METADATA
- ##### PUBLISHED MAPS
- WETLAND ATLAS
  - DRAINAGE ATLAS
  - FEMA MAPS

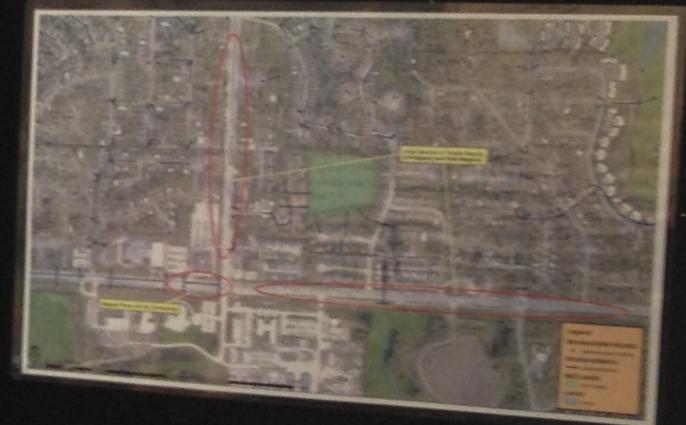
#### Interactive Web Mapper

- STREAMS
- SUBBASINS AND OUTFALLS
- WETLANDS
- MS4 STREETS
- SNOW DISPOSAL SITES
- ...AND MORE!



### DOT Drainageway Network Mapping A Joint Digitization Effort by DOT and MOA

Before:



After:



## Support Slides for Q&A Session

## *Low Impact Development*

- LID is an Ecosystem approach.
- Development should be *part* of a functioning ecosystem, not exist separately from it.

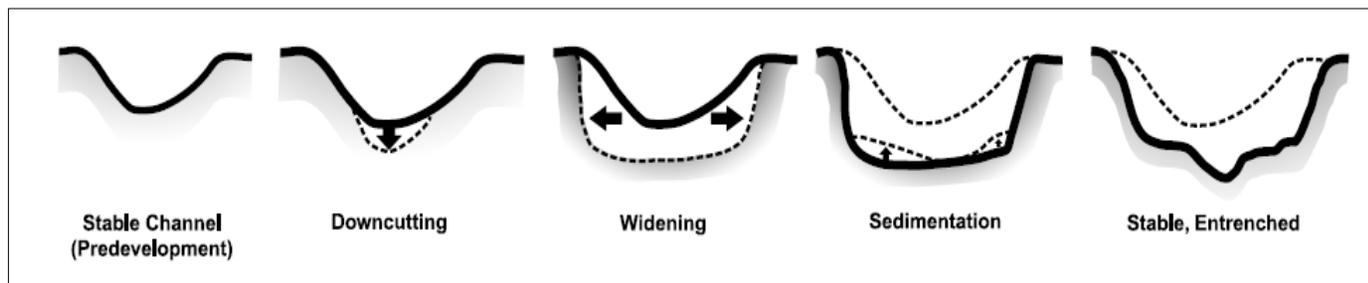


## *Why is LID needed?*

- Development results in:
  - Decreased infiltration
  - Loss of Groundwater Recharge
  - Faster runoff
  - Increased peak flow rates in receiving waters
  - Increased sedimentation and erosion



Source: Georgia Stormwater Manual



Source: Georgia Stormwater Manual

# Why is LID needed?



Source: *Low Impact Development – A Design Manual for Urban Areas*

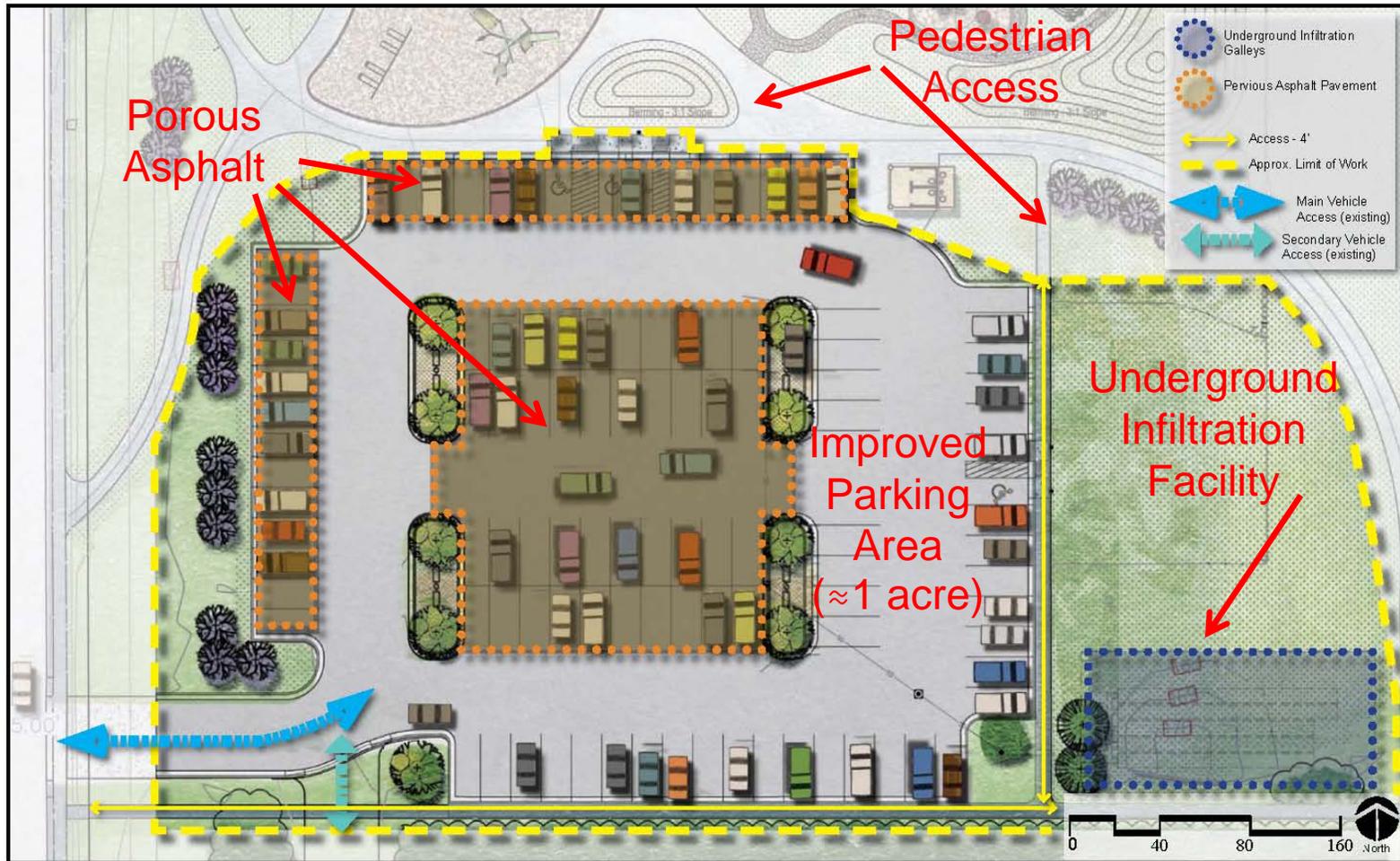
## *How does this impact Anchorage?*

- Anchorage's MS4 permit requires:
  - Onsite capture of small storm events
    - Runoff from 0.52 inches (or less) of rainfall preceded by 24-hours of no precipitation
  - Construction of LID "Pilot Projects"

*Existing Russian Jack Park (Photo Via Google Earth)*



# Proposed Parking Lot Improvements (Photo Courtesy of Corvus Design)



north Russian Jack Springs Park - Parking Lot & Low Impact Development Strategies

## *Proposed LID Features*

- Porous Asphalt
  - Designed to store and infiltrate up to the 10-year, 24-hour event (1.77 inches).
  - Additional flow will surface flow to inlets
- Subsurface infiltration system
  - Designed to store and infiltrate excess runoff from the porous and impervious pavement.

# *Porous Asphalt Pavement*

- What is porous asphalt pavement?



Source: NAPA IS-131

## *Porous Asphalt Pavement*

- What is porous asphalt pavement?

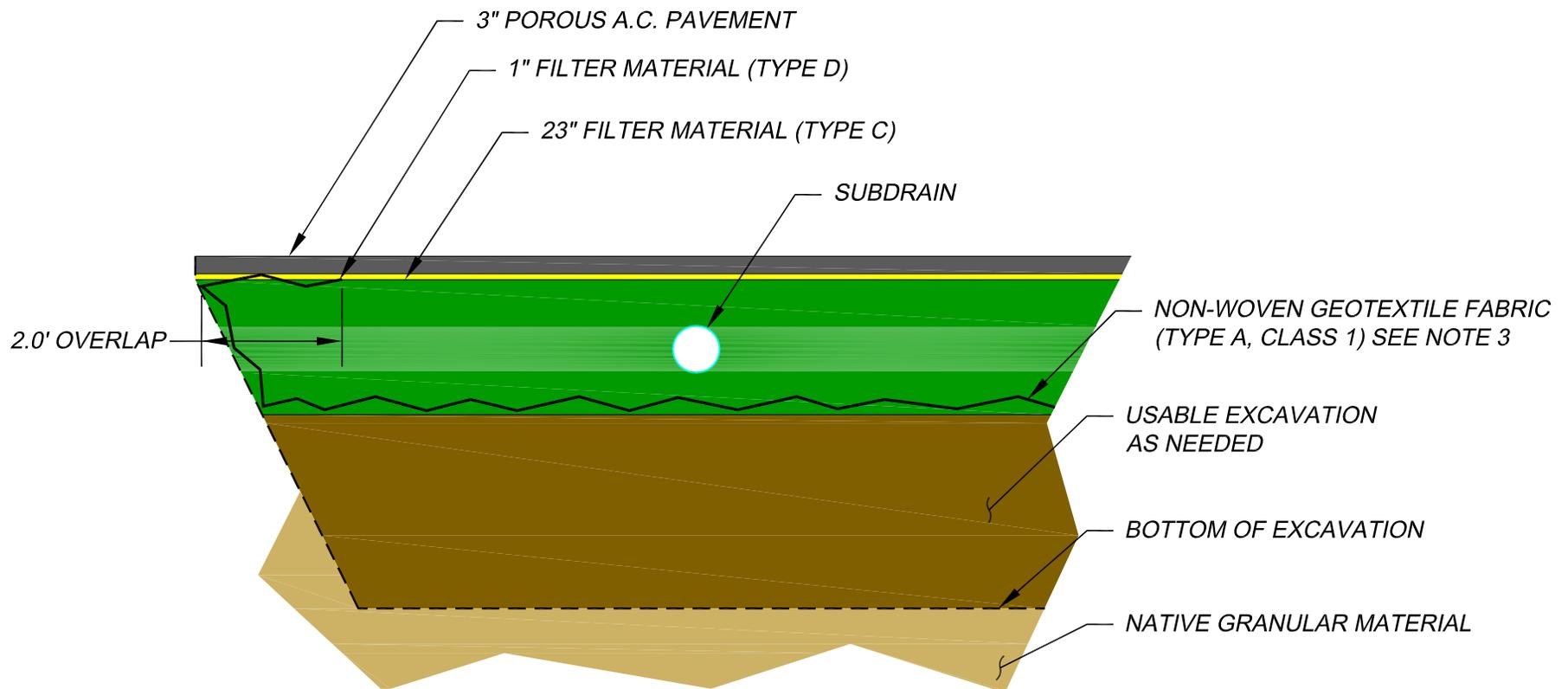
Sieve Size	Porous Mix*	MOA Class E
	Percent Passing	
3/4 - inch	99-100	100
1/2 - inch	85-100	78-96
3/8 - inch	55-75	66-86
#4	10-25	46-66
#8	5-10	34-52
#200	2-4	3-9

\*UNH Stormwater Center Design Specifications

\*\*Section 40.06 of MASS

# *Porous Asphalt Pavement*

- Russian Jack LID Pilot Project Design

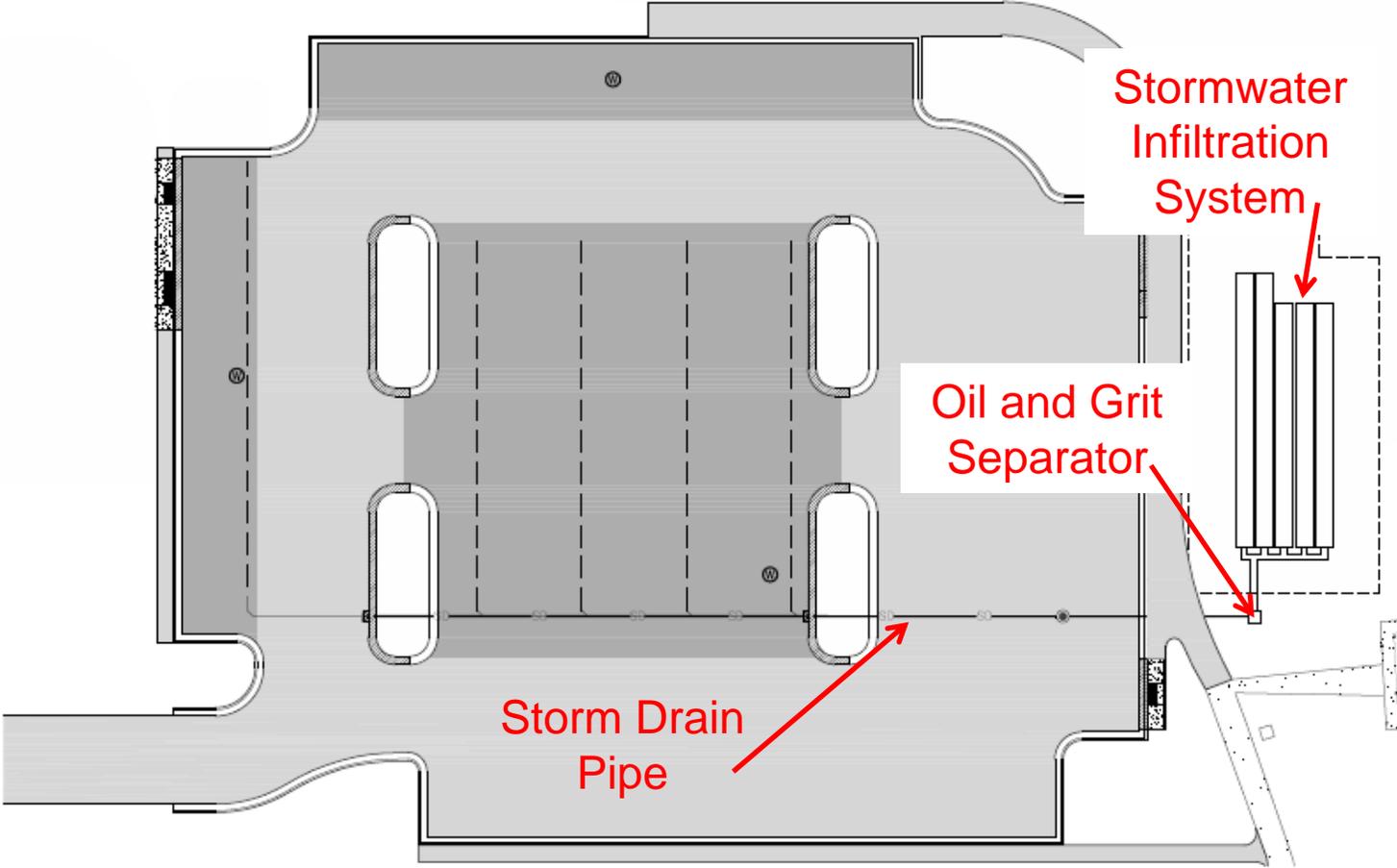


## *Stormwater Infiltration System*

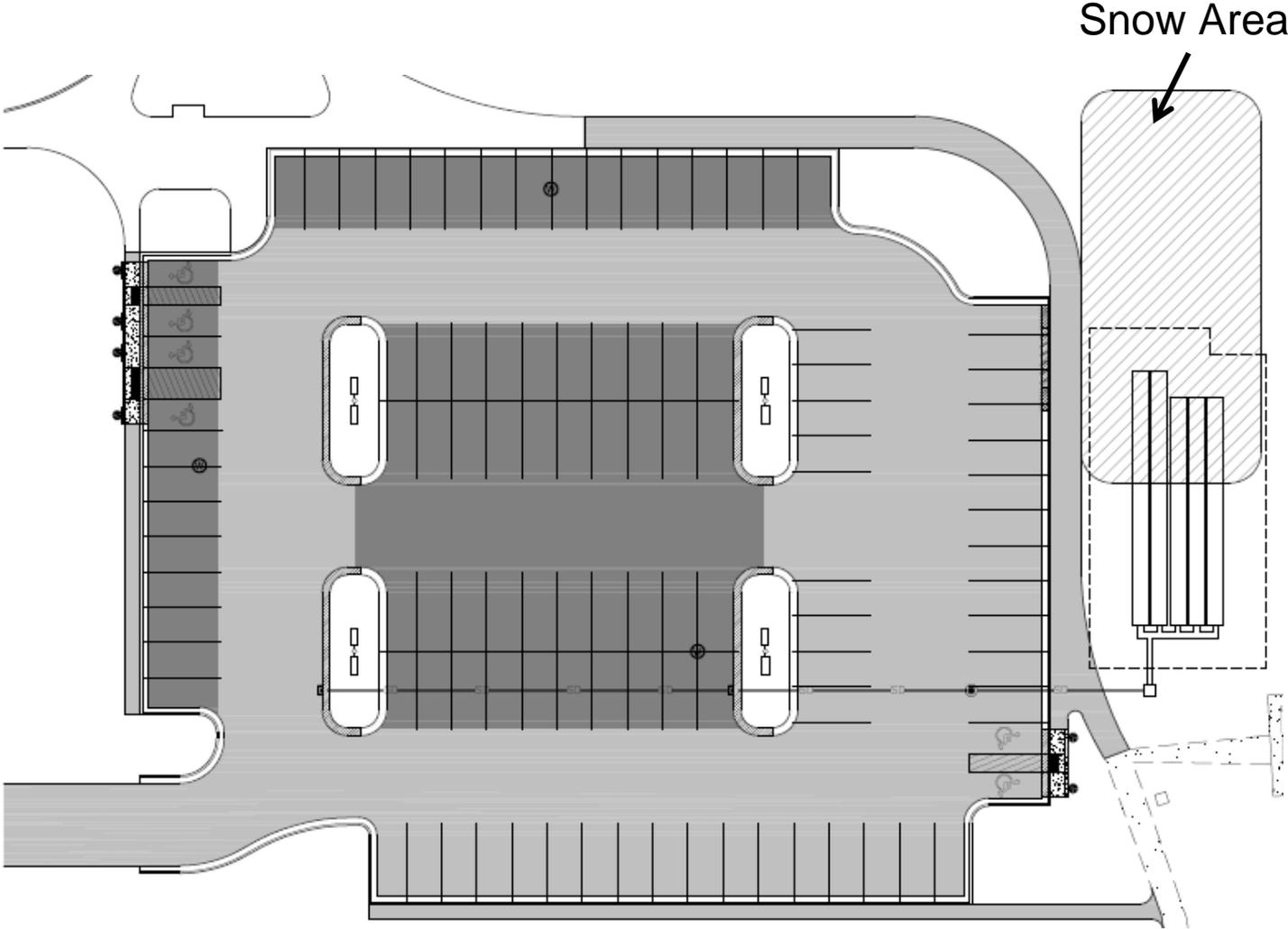
- **Contech ChamberMaxx System**



# Stormwater Infiltration System



*Selection of LID Features*



## *Need for Other LID Project*

- The MOA would like to make YOUR project an LID Pilot project.
  - Any project disturbing more than 10,000 square feet of land is a candidate.
  - MOA Funding is available for design and construction support.
  - Designers would ensure redundancy in case of system failure.

## *Types of LID Infrastructure*

- Bioretention
  - Swales
  - Trenches
  - Rain Gardens
- Infiltration
  - Trenches
  - Soak-away pits
  - Ponds
- Retention
  - Ponds
  - Reservoirs
- Lots more!



*Anchorage Taku Lake Rain Garden*

*Got a Possible LID Project?*

Please contact:

Kristi Bischofberger, MOA WMS

Email:

[BischofbergerKL@ci.anchorage.ak.us](mailto:BischofbergerKL@ci.anchorage.ak.us)

Phone: 343-8058

OR

Janie Dusel, HDL

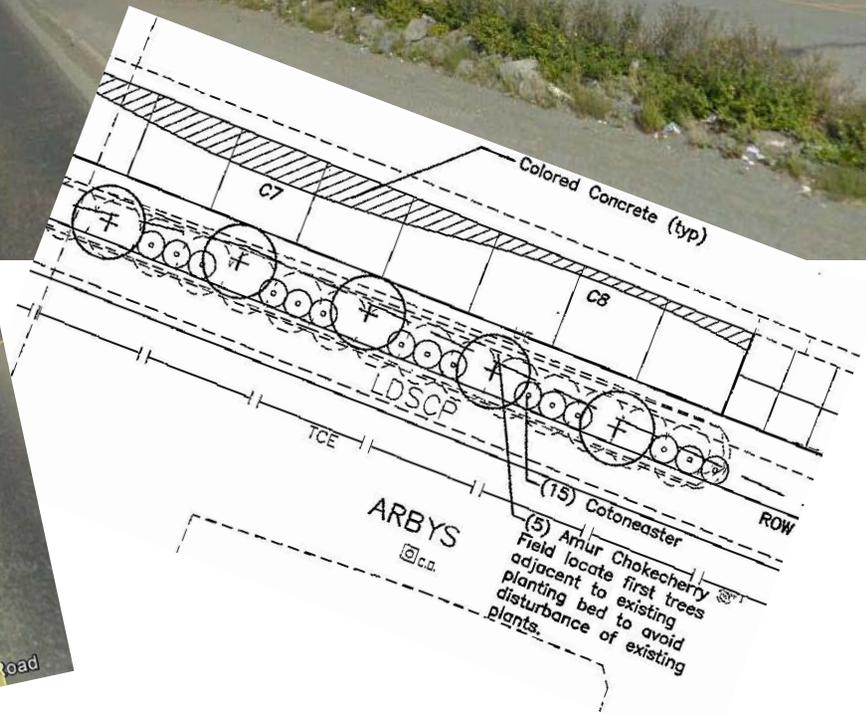
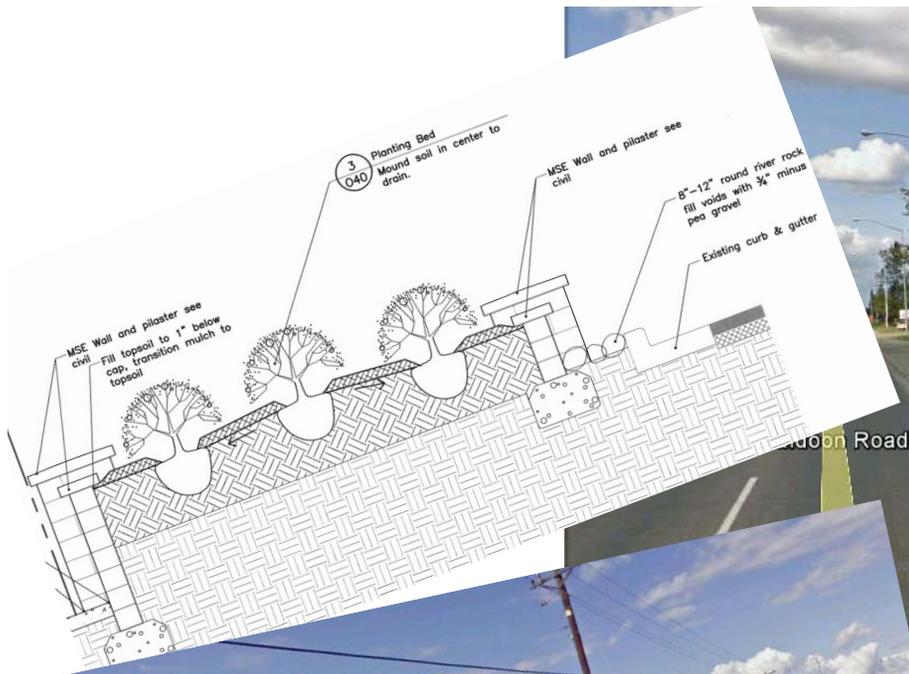
Email: [jdusel@hdlalaska.com](mailto:jdusel@hdlalaska.com)

Phone: 564-2120

# Muldoon Road Project Area



# Muldoon Road Typical LID Elements



# West Dowling Road Phase I

Detention Swale  
with check dams

Rain Garden



Detention  
Pond

# West Dowling Road Phase II



**LEGEND**

- GRASS LINED SWALE
- INFILTRATION BASIN

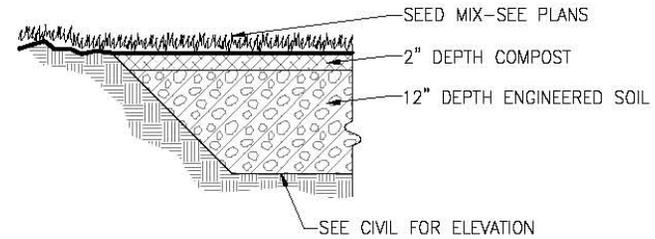
**West Dowling Road Phase II**

Storm Water Treatment

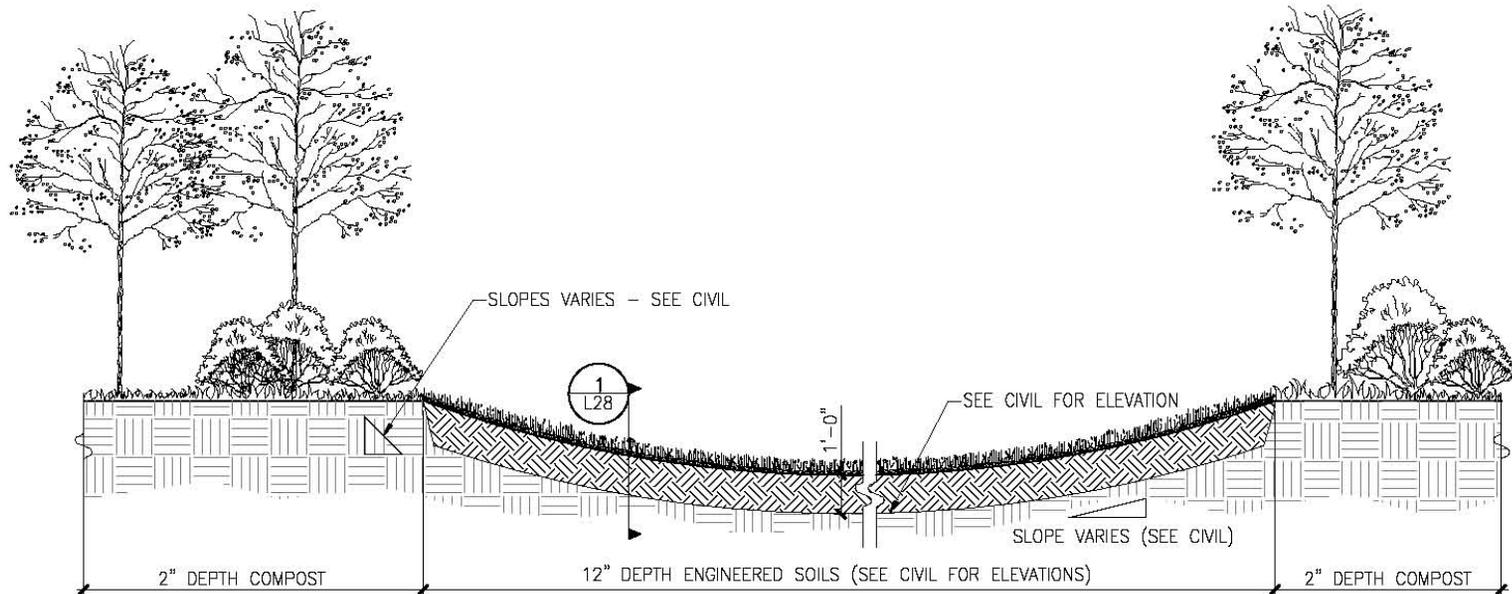
NORTH

0 100 200 400

# West Dowling Phase II Infiltration Basin Detail

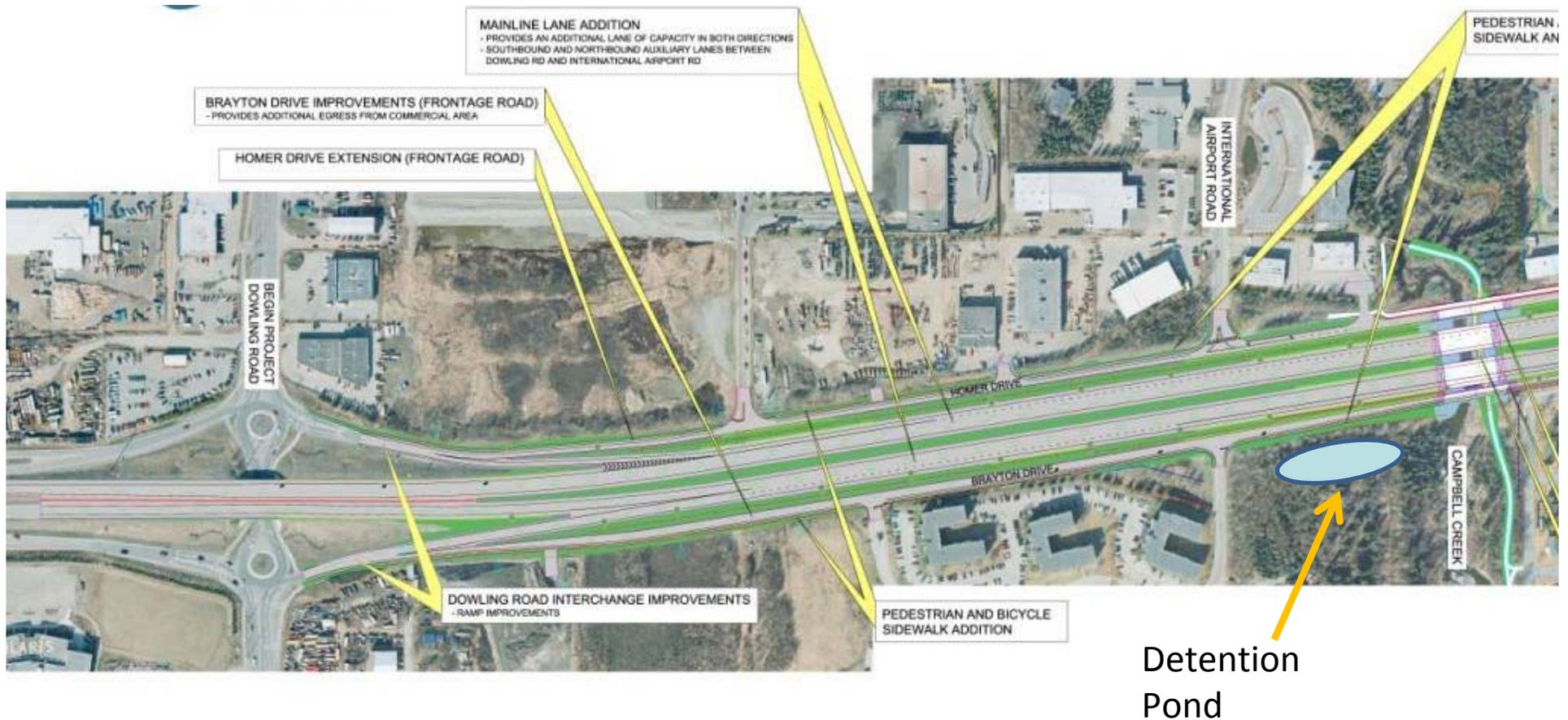


**1** INFILTRATION BASIN SECTION  
L28 SCALE: 1"=1'-0" dt-rain-grdn-sect.dwg

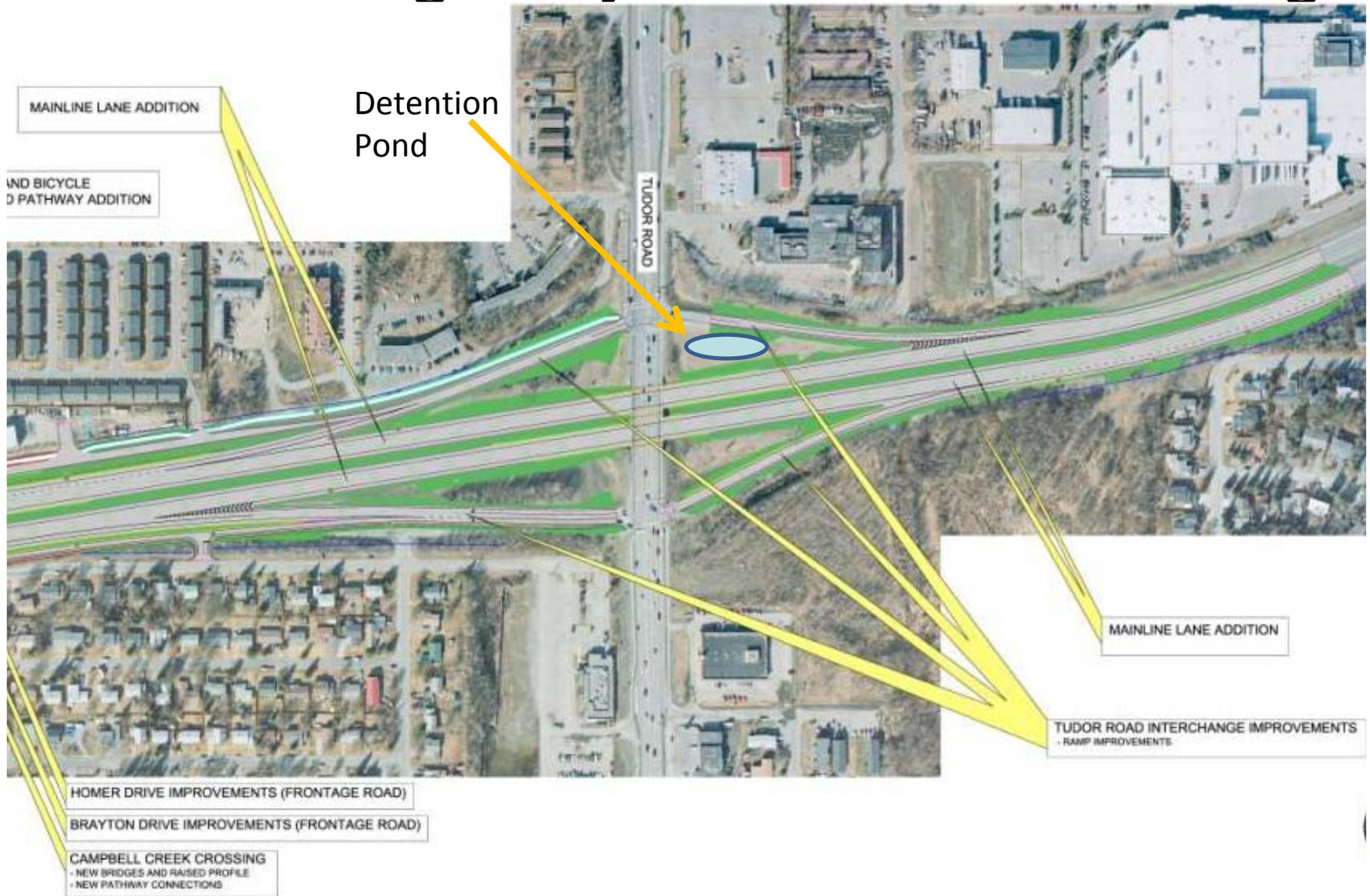


**2** INFILTRATION BASIN  
L28 SCALE: 3/8"=1'-0" dt-stel-tree-stencil.dwg

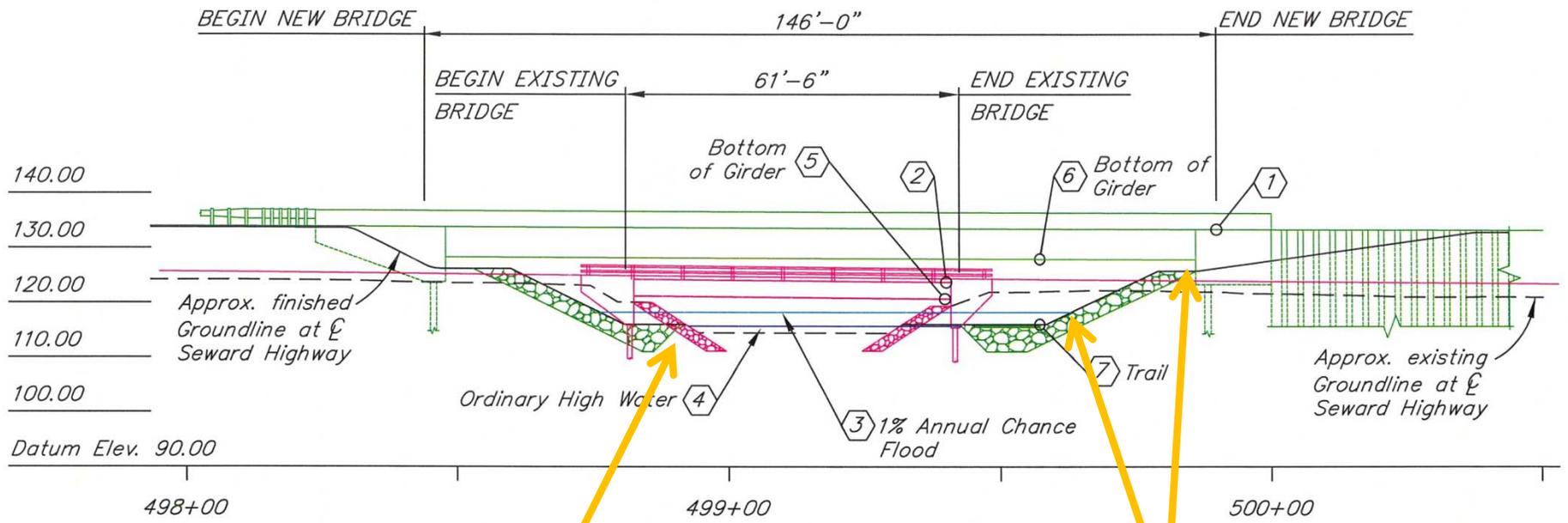
# Seward Highway – Tudor to Dowling



# Seward Highway – Tudor to Dowling



# Seward Highway – New Bridges

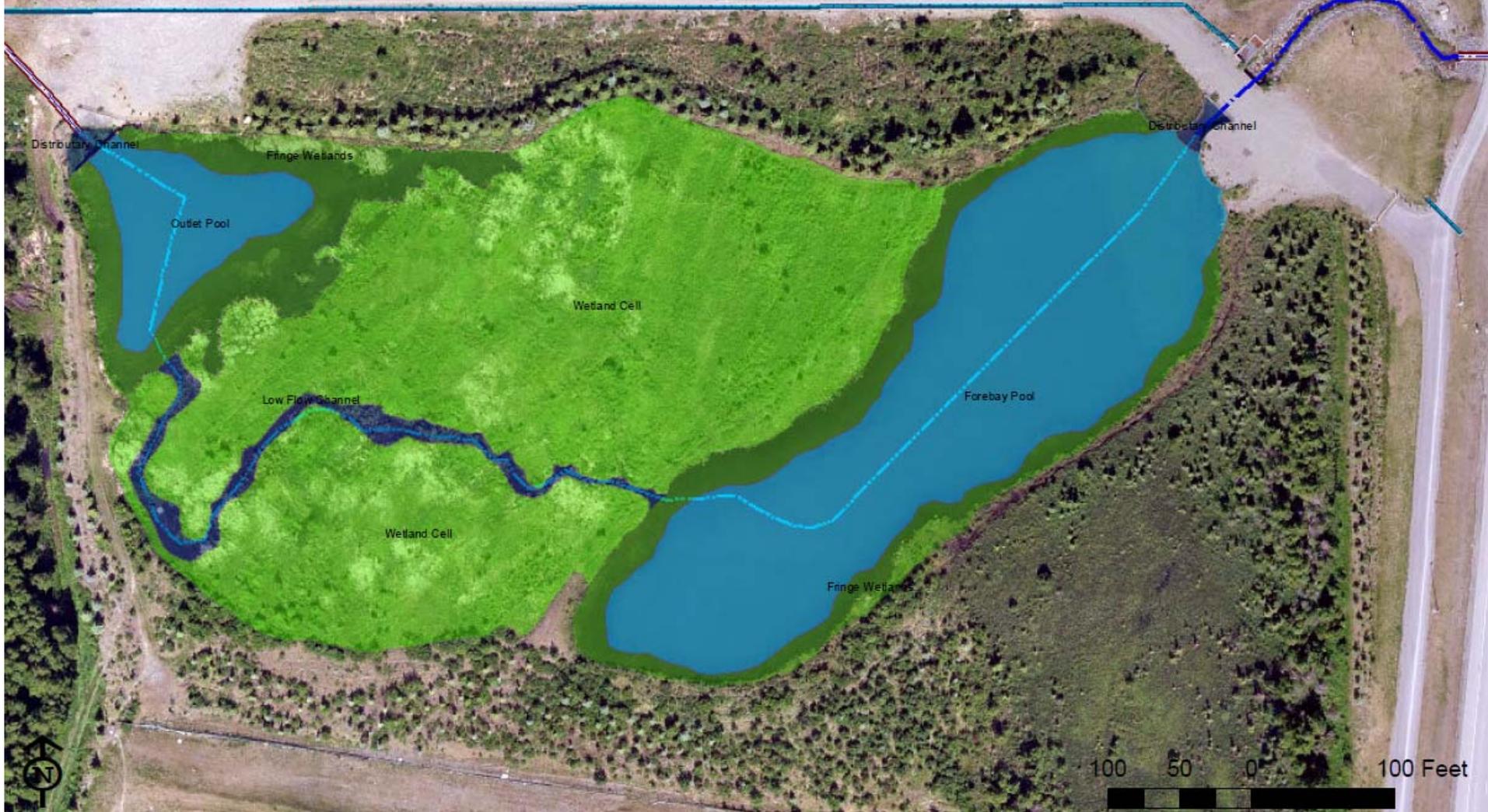


All work outside of existing stream bed

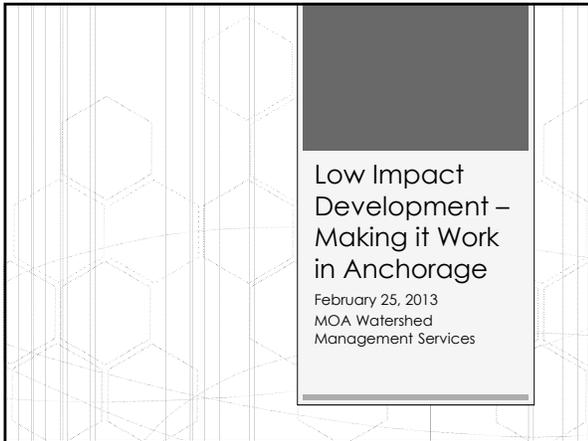
3x the additional infiltration/flow area

# Sedimentation Basin and OGS Effectiveness Evaluation

Monitoring: Structural Controls Assessments  
APDES Permit Requirement IV.A.8.



## Handouts from Afternoon LID Training Session



Low Impact Development – Making it Work in Anchorage

February 25, 2013  
MOA Watershed Management Services

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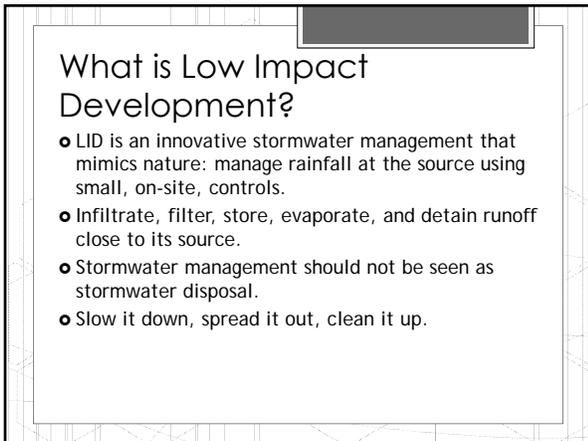
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### What is Low Impact Development?

- LID is an innovative stormwater management that mimics nature: manage rainfall at the source using small, on-site, controls.
- Infiltrate, filter, store, evaporate, and detain runoff close to its source.
- Stormwater management should not be seen as stormwater disposal.
- Slow it down, spread it out, clean it up.

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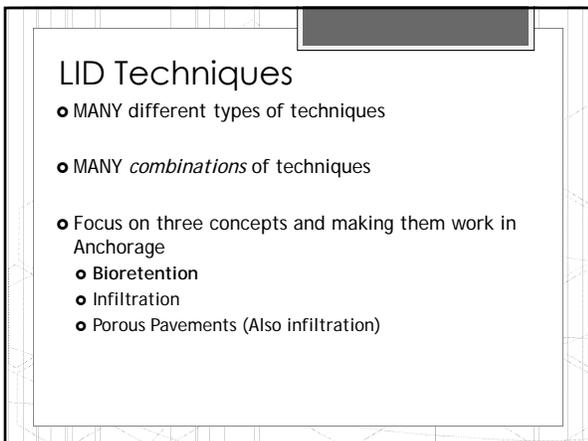
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### LID Techniques

- MANY different types of techniques
- MANY combinations of techniques
- Focus on three concepts and making them work in Anchorage
  - Bioretention
  - Infiltration
  - Porous Pavements (Also infiltration)

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### Considerations for LID in Anchorage

- What are some limitations or constraints that we might face when implementing LID in Anchorage?
  - High Groundwater
  - Poorly infiltrating soils
  - Limited space
  - Bedrock or other limiting strata
  - Sanding and plowing
  - Frozen Ground/Winter Rainfall events

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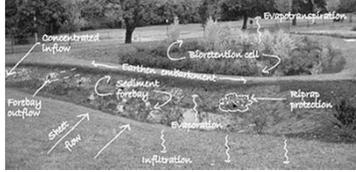
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### Bioretention

- Bioretention: A shallow stormwater basin or landscaped area that utilizes engineered soils and vegetation to capture and treat runoff. Examples include rain gardens and bioretention swales.



- Cleans, Infiltrates, Detains

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### Bioretention Examples

- High Point, Seattle
  - Photo during 25-year storm
  - Accepts water from road and sidewalks



- Prince George's Co. Maryland
  - Adelphi Road
  - Used Bioretention instead of C&G
  - Accepts water from road surface




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**Bioretention - Can we do it in Anchorage?**

- Yes!
- Taku Rain Garden – collects runoff from adjacent parking lot
- Lithia Chrysler Dodge of Anchorage (5<sup>th</sup>) – Bioretention swales collect water from pavement surfaces
- Dozens of rain gardens around Anchorage
- Upcoming West Dowling Road Phase II (C-Street to Minnesota) – Uses bioretention rain gardens in lieu of storm drain




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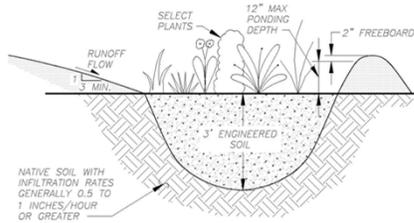
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**Considerations for Anchorage Bioretention**

- Extremely Versatile
- Typical Section – Base Case




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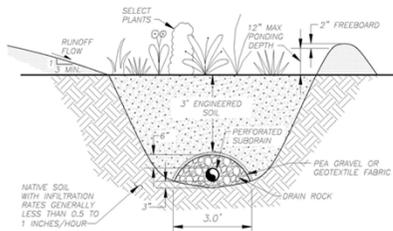
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**Considerations for Anchorage Bioretention**

- Modification 1

- ✓ Poorly infiltrating soils
- ✓ Limiting strata ≈ 4 feet from facility floor




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Considerations for Anchorage  
**Bioretention**

- Modification 2
  - Groundwater less than 2 feet from the facility floor
  - Contaminated sites
  - Can't meet separation distances

✓ High Groundwater  
✓ Limited space  
✓ Contaminated soils

Diagram labels: RUNOFF FLOW, SELECT PLANTS, 12" MAX PONDING DEPTH, 2" FREEBOARD, 18" ENGINEERED SOIL, PERFORATED SUBDRAIN, PEA GRAVEL OR GEOTEXTILE FABRIC, DRAIN ROCK, IMPERMEABLE LINER, NATIVE SOIL. Dimensions: 3', 3.0'.

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Considerations for Anchorage  
**Bioretention**

- Plowing is generally not a problem—better if you don't pile snow in the facility
- Sanding – impact on performance is related to facility size and if pretreatment is provided. (E.g. grass filter strips)
- Recommend that footprint is 5% of contributing area if area is impervious

✓ Plowing and Sanding

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Considerations for Anchorage  
**Bioretention**

- Frozen Ground
  - Two conditions that cause concern
    - Spring break-up
    - Occasional wintertime rain-on-snow event
  - Consider this when you layout the site – what will happen if surface ponding occurs?
  - Is it a problem? Does the Owner care?
  - What is down-gradient from the facility?
  - Put in an overflow if needed

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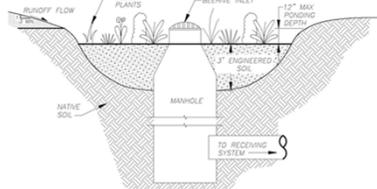
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Considerations for Anchorage  
**Bioretention**

- Overflow ✓ Frozen Ground
- Pipe or surface flow
- May be needed anyway for large storm events, depending on the design.
- Can aid in permitting process



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Considerations for Anchorage  
**Bioretention**

- Overflow



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**Infiltration**

- Also called Retention
- Versatile practice where water is collected, stored, and infiltrated into the ground.
- Can be used at the surface or at varying depths.
- Infiltration trenches, chambers, ponds, leach field., etc.

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**Infiltration Example**

- Medical Plaza Way, Clarksville, IN
- 75,000 SF Hospital and 40,000 SF Medical Building
- Local regulations required stormwater retention



Photo from Chambermaxx website

- Parking Lot was installed with **690** Contech Chambermaxx units for **33,810** cubic feet (**252,916** gallons) of storage.

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**Can we do this in Anchorage?**

- **Infiltration (Also called Retention)**
  - Russian Jack Spring Park (Chambers)
  - Tacotna Commons (Ponds)
  - Unique Mechanics (Chambers)
  - Radio Shack/True Value (Chambers)
  - Anchorage Fire Station 5 (Leach-field Concept)
  - Providence Extended Care Facility (Pond)
  - UAA Health Science Building (Chambers and Pond)

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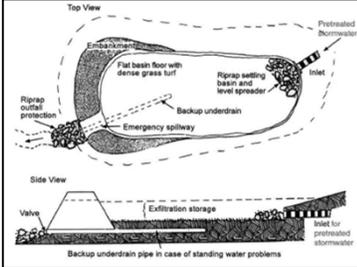
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**Considerations for Anchorage**

**Infiltration - Basin**

- Basin
- Shallow depressions that collect and infiltrate water of a period of several days




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Considerations for Anchorage  
**Infiltration - Basin**



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Considerations for Anchorage  
**Infiltration - Basin**

- When properly designed, can work well in cold climates.
- With ice cover or frozen conditions, water ponds on the surface—no problem.
- Limited use with high groundwater table
- Can work with poor soils, depending on space available and needed drain-down times.
- Plowing and Sanding – Pretreatment is recommended
  - Sediment forebay
  - Grit chamber

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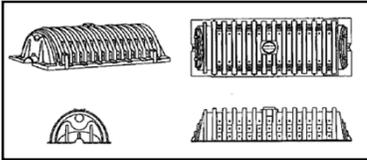
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Considerations for Anchorage  
**Infiltration**

- Chambers
- Store and infiltrate water
- Versatile – can go below pavement or pervious surface
- Can be installed below frost depth
- Usually requires pretreatment for sediment



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Considerations for Anchorage  
**Infiltration**

- Chambers



Infiltration:  
✓ Frozen Ground  
✓ Poorly infiltrating soils  
✓ Plowing and sanding  
X High ground water  
X Contaminated soils

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**Porous Pavements**

- A system that reduces impervious surfaces, thus increasing infiltration and reducing runoff.
- May include modular paving blocks or grids, porous concrete, porous asphalt, cast-in-place concrete grids, and soil enhancement technologies.

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**Porous Asphalt Examples**

- Porous Asphalt Parking - Clark Township, Town Hall parking lot, New Jersey



- Porous Concrete Sidewalk - NE 79<sup>th</sup> Street, Redmond WA



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**Can we do this in Anchorage?**

- **Porous Pavements**
  - Russian Jack Springs Park (Porous Asphalt) – installed in parking bays in the parking lot.
  - Residential Condominium on Spenard (Pavers and Porous Concrete) – installed in three courtyard "common" areas for the Habitat for Humanity Project

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**Considerations for Anchorage Porous Pavements**

- Several Types
  - Porous Asphalt
  - Porous Concrete
  - Porous Pavers
- Fairly new practice for Anchorage
- Porous asphalt – RJSP
  - Performance will be monitored
- Porous Pavers and Porous Concrete
  - Performance was monitored and results are presented in a technical paper by Tamás Deák.

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**Considerations for Anchorage Porous Pavements**

- Requires engineering for the system, subgrade, and placement—don't just "throw it in"
- Consider location, use, and maintenance capabilities
- Works very well with poorly infiltrating soils—spreads out the water
- Impact of plowing and sanding depends on the facility design and frequency of maintenance
  - Habitat project is sanded and not vacuum swept, performing as designed
  - Russian Jack project was layed out to minimize impacts of sanding
- Research different types of systems
- One paver block may be locally manufactured starting this year.

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Considerations for Anchorage  
**Porous Pavements**



Habitat for Humanity Project (2008)

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Considerations for Anchorage  
**Porous Pavements**



Photos and project information courtesy of Tamás Deák, kpb architects. "*Cold Climate Performance Evaluation of Permeable Interlocking Concrete Pavement and Porous Concrete Pavement Systems.*"

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Considerations for Anchorage  
**Porous Pavements**



Infiltration:  
✓ Frozen Ground  
✓ **Poorly infiltrating soils**  
-- Plowing and sanding  
X High ground water  
X Contaminated soils

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Considerations for Anchorage

**Other Easy Ideas**

- Slow your water down – increase the site Time of Concentration
  - Helps water soak into the ground
  - Can help reduce the peak flow
  - Example – Direct downspouts to lawn instead of driveway
- Capture water and use it later. (E.g. Rain Barrel)
- Send runoff from pervious areas to impervious areas.

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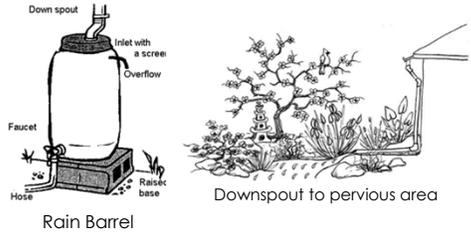
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Considerations for Anchorage

**Other Easy Ideas**



The diagram shows two water management techniques. On the left, a rain barrel is shown with a 'Down spout' at the top, an 'Inlet with a screen' just below it, and an 'Overflow' on the right side. At the bottom left, there is a 'Faucet' and a 'Hose' attached to the barrel. At the bottom right, there is a 'Raiser base'. On the right, a downspout is shown extending from a roofline down to a lawn area, labeled 'Downspout to pervious area'.

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Low Impact Development

**Design Considerations**

- Infiltration (or percolation) rates
  - Look at the soils at facility depth and below
  - Onsite testing: In-place, At depth
  - Test value vs. design value
  - Choose appropriate FS
    - Consider long-term perc rates
    - Depends on facility, it's critical function, the consequences of failure, etc.
    - Involve a geotechnical engineer
  - Field verify design values during construction
  - For poorly draining soils, consider a subdrain and/or overflow.

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Low Impact Development  
**Design Considerations**

- Many site see success using multiple techniques
- Multiple Independent facilities
  - Habitat project – porous pavements + rain gardens
- Treatment "train"
  - UAA Health Science Building– Chambers→Pond→Receiving System
  - RJSP (train)—Porous Asphalt→Chambers

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Combined Techniques

- Maplewood Mall, Minneapolis, MN suburb
  - Porous pavers, Rain gardens, tree trenches, planters,, and more.
  - Project will capture and infiltrate or filter at least 1 inch of runoff



For more info, visit  
[http://www.stormh2o.com/SW/Articles/Green\\_Infrastructure\\_Makes\\_Sense\\_in\\_the\\_Twin\\_Cities\\_19789.aspx](http://www.stormh2o.com/SW/Articles/Green_Infrastructure_Makes_Sense_in_the_Twin_Cities_19789.aspx)

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Low Impact Development  
**Example Work Session: Estimating Runoff and Sizing a Bioretention Facility**

- Simple method for determining flow into your facility
- Direct Determination Method (See handout)

Runoff = Rainfall – Depression storage – Infiltration

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Low Impact Development  
**What about cost?**

- True representation of cost in Anchorage is TBD
- Costs tend to be unpredictable due to uncertainty
- The more LID practices are included on projects, the less the uncertainty
- Contractors will become familiar with the practices

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Low Impact Development  
**What about cost?**

- Uncertainty in RJSP Porous Asphalt
- Unit Prices on Bids for Porous Asphalt had a 154% increase from the low bid to the high bids
- Regular asphalt had a 48% increase from the low bid to the high bid
- Compared to regular asphalt, the percent increase in cost ranged from 8% to 134%.

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Low Impact Development  
**What about cost?**

- What Communities are saying
  - Minneapolis/St. Paul
    - *"In most instances we have found that the green infrastructure costs will be considerably less than grey infrastructure costs in heavily urbanized environments where land costs are high and there is also the cost or impact of shutting down roads and whatnot on the existing owners and tenants"* – Peter MacDonagh, Principal and Cotounder of Kestrel Design Group
    - *"Green Infrastructure can be quite economical as compared to conventional stormwater management BMPs that can consume more land area. Green infrastructure can reduce other stormwater conveyance and storage costs. With the added benefit of providing improved aesthetics and water conservation, some argue that the costs can be less over the long term."* – Cliff Aichinger, Administrator of the Ramsey-Washing Metro Watershed District, MN

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**Low Impact Development**  
**What about cost?**

- Banking on Green – A Look at how green infrastructure can save Municipalities Money and Provide Economic Benefits Community-wide. (April 2012)
- Discusses:
  - What's driving Green Infrastructure – Environment and Infrastructure deficit
  - Capital Costs
  - Long-term costs
  - Direct and Indirect Benefits

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**Green Infrastructure Practices Offer Cost-Effective Solutions**  
*American Society of Landscape Architect's Green Infrastructure Survey*

As part of its efforts to collect information about green infrastructure, EPA asked ASLA to collect case studies on projects that successfully and sustainably manage stormwater. ASLA members responded with 479 case studies from 43 states, the District of Columbia, and Canada. Not only do these projects showcase landscape architecture, they also demonstrate to policymakers the value of promoting green infrastructure policies. Green infrastructure and low-impact development (LID) approaches, which are less costly than traditional grey infrastructure projects, can save communities millions of dollars each year and improve the quality of our nation's water supply.

Project type:		Green Infrastructure type:	
Institutional/Education	21.5%	Retrofit of existing property	50.7%
Open Space/Park	21.3%	New development	30.7%
Other	17.6%	Redevelopment project	18.6%
Transportation Corridor/Streetscape	11.9%		
Commercial	8.6%	Did use of green infrastructure increase costs?	
Single Family Residential	5.5%	Reduced costs	44.1%
Government Complex	4.2%	Did not influence costs	31.4%
Multifamily Residential	3.7%	Increased costs	24.5%
Open Space Garden/Arboretum	2.9%		
Mixed Use	1.8%		
Industrial	1.1%		

**Analysis**

- Over 300 ASLA members and other practitioners responded with 479 case studies from 43 states, the District of Columbia, and Canada.
- 55 percent of the projects were designed to meet a local ordinance.
- 88 percent of local regulators were supportive of the green infrastructure projects submitted.
- 68 percent of the projects received local public funding.

Details about the study and its results are available here: [www.asla.org/stormwater](http://www.asla.org/stormwater)

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**Low Impact Development**  
**Other Considerations**

- Maintenance:
  - Depends on the facility
  - Maintenance frequency and capabilities should be considered during design
    - Involve maintenance personnel
  - Small, Frequent maintenance
    - Mowing
    - Weeding
    - Caring for Vegetation
  - Larger maintenance
    - Structure repairs
    - Eventual rehabilitation
  - Involve maintenance staff in the design process
  - Be practical

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**Low Impact Development  
Other Considerations**

- Construction is KEY
  - Devil is in the details
  - On-site representative needs a thorough understanding of the entire project.
  - Special care is needed for low-bid projects vs. collaborative design-build efforts.

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**Selecting the right LID Practice**

- Picking an LID technique right for your site is key
- Different techniques work better with different constraints
- Don't forget the purpose – small, frequent events.
- See Selection Considerations Handouts
- MOA is expanding their design criteria for LID and other stormwater management facilities to make implementation easier.

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**Low Impact Development  
Lots of Options!**

- There are MANY ways to incorporate LID
- Remember, think small. Little efforts can add up to a large success.

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Low Impact Development  
Resources

- Low Impact Development Center  
[www.lowimpactdevelopment.org](http://www.lowimpactdevelopment.org)
- Stormwater Magazine  
<http://www.stormh2o.com/SW/SWhome.aspx>
- Minnesota BMP Manual  
<http://www.pca.state.mn.us/index.php/water/water-types-and-programs/stormwater/stormwater-management/minnesotas-stormwater-manual.html>
- Upcoming MOA revised stormwater design criteria manual

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Instructor Contact Information

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AWR Engineering, LLC  
[jdusel@awr-eng.com](mailto:jdusel@awr-eng.com)  
[www.awr-eng.com](http://www.awr-eng.com)

Thanks for coming!

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## Bioretention Sizing Exercise

We are going to design a bioretention area in just a few simple steps. In Part 1, we will calculate the runoff that will be collected in the bioretention area. In Part 2, we will size the bioretention area.

### Part 1 – Calculate Runoff using the Direct Determination Method.

#### Site Information

Total Site Area =	1 Acre
Roof top Area =	0.2 Acre
Parking Lot and Driveway Area =	0.6 Acre
Lawn Area =	0.2 Acre
Design Rainfall Event =	0.5 inches in 24 hours
Soil Type =	Type B (Mostly sandy)
Design Soil Moisture Condition =	Partially Dried Out (At least 48-hours with no prior precipitation)

#### ***Runoff = Rainfall – Depression Storage – Infiltration Loss***

(See the attached sheet on the Direct Determination Method for depression storage values, estimates of maximum and minimum infiltration rates, and decay constant. Infiltration rates can also be based on field testing.)

$$\text{Roof Runoff} = 0.52'' - 0.1'' - 0'' = 0.42''$$

$$\text{Pavement Runoff (Parking lot and driveway)} = \underline{\hspace{10cm}}$$

$$\text{Lawn Runoff} = 0.52'' - 0.25'' - \text{Lawn Infiltration (f)}$$

$$\text{Lawn Infiltration Rate} = f_{min} + (f_{max} - f_{min}) * e^{-k\Delta t}$$

From the attached handout, for a sandy, Type B soil:

$$f_{min} = \underline{\hspace{2cm}} \frac{\text{in}}{\text{hour}} \text{ and } f_{max} = \underline{\hspace{2cm}} \frac{\text{in}}{\text{hour}} \text{ and } k \cong \underline{\hspace{2cm}} \text{ hr}^{-1}$$

This equation is describing how fast the soil infiltration rate will change from maximum to minimum as the soils becomes saturated. For the sake of time in this example, you can choose  $\Delta t = 24$  hours, or the whole storm. This basically assumes the minimum infiltration rate for the entire storm, and will result in less infiltration than if you looked at a smaller  $\Delta t$ , like 0.5 hours. (In the case of a smaller  $\Delta t$ , you would look at the infiltration loss every 30 minutes and sum them all up when you're finished.)

$$\text{Lawn Infiltration Rate} = \underline{\hspace{2cm}} \text{ inches/hour}$$

$$\text{Then, total Lawn Infiltration} = 0.15 \text{ in/hour} * 24 \text{ hours} = \underline{\hspace{2cm}} \text{ inches}$$

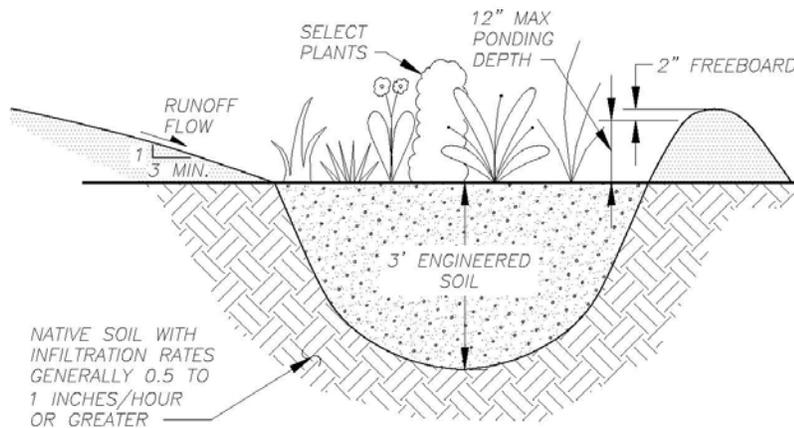
Lawn Runoff =  $0.5'' - 0.25'' - 3.6'' =$  \_\_\_\_\_ inches. (Note: if you get a negative number, then runoff is zero)

Total Site Runoff (in inches per acre)

$$= \frac{(\text{Roof Runoff} * \text{Roof Area}) + (\text{Pavement Runoff} * \text{Pavement Area}) + (\text{Lawn Runoff} * \text{Lawn Area})}{\text{Site Area}}$$

Total Site Runoff in inches per unit area = \_\_\_\_\_ inches/acre.

**Part 2 – Size the bioretention area footprint.**



Sizing equation for a standard bioretention facility:

$$A_r = \left( \frac{TIV * d_{es}}{I * (P_d + d_{es}) * t} \right)$$

Where:

$A_r$  = Minimum bioretention area Footprint (square feet)

$TIV$  = Target Infiltration Volume (cubic feet)

$P_d$  = Depth of Poned Water (ft)

$d_{es}$  = Depth of Engineered Soil (ft)

$I$  = Infiltration rate of the native soil at the bottom of the bioretention area (ft/hour)

$t$  = Facility drain-down time (hours) (48 hours maximum)

To get TIV, convert the inches of runoff per acre from Part 1 to a volume in cubic feet.

$$TIV = 0.336 \text{ inches/acre} * 1 \text{ acre} * 43,560 \text{ ft}^2/\text{acre} * 1 \text{ foot}/12 \text{ inches} = \text{_____} \text{ ft}^3$$

The infiltration rate in the sizing equation should be the *design* infiltration rate, based on testing and an appropriate factor of safety. For this example, let's assume that value is the same as the minimum infiltration rate used in Part 1 (0.15 inches/hour = 0.0125 ft/hour).



## Bioretention Sizing Exercise - *Solution*

We are going to design a bioretention area in just a few simple steps. In Part 1, we will calculate the runoff that will be collected in the bioretention area. In Part 2, we will size the bioretention area.

### Part 1 – Calculate Runoff using the Direct Determination Method.

#### Site Information

Total Site Area =	1 Acre
Roof top Area =	0.2 Acre
Parking Lot and Driveway Area =	0.6 Acre
Lawn Area =	0.2 Acre
Design Rainfall Event =	0.5 inches in 24 hours
Soil Type =	Type B (Mostly sandy)
Design Soil Moisture Condition =	Partially Dried Out (At least 48-hours with no prior precipitation)

$$\text{Runoff} = \text{Rainfall} - \text{Depression Storage} - \text{Infiltration Loss}$$

(See the attached sheet on the Direct Determination Method for depression storage values, estimates of maximum and minimum infiltration rates, and decay constant. Infiltration rates can also be based on field testing.)

$$\text{Roof Runoff} = 0.52'' - 0.1'' = 0.42''$$

$$\text{Pavement Runoff (Parking lot and driveway)} = 0.52'' - 0.1'' = 0.42''$$

$$\text{Lawn Runoff} = 0.52'' - 0.25'' - \text{Lawn Infiltration (f)}$$

$$\text{Lawn Infiltration} = f_{min} + (f_{max} - f_{min}) * e^{-k\Delta t}$$

From the attached handout, for a sandy, Type B soil:

$$f_{min} = 0.15 \frac{\text{in}}{\text{hour}} \quad \text{and} \quad f_{max} = 5 \frac{\text{in}}{\text{hour}} \quad \text{and} \quad k \cong 2 \text{ hr}^{-1}$$

This equation is describing how fast the soil infiltration rate will change from maximum to minimum as the soils becomes saturated. For the sake of time in this example, you can choose  $\Delta t = 24$  hours, or the whole storm. This basically assumes the minimum infiltration rate for the entire storm, and will result in less infiltration than if you looked at a smaller  $\Delta t$ , like 0.5 hours. (In the case of a smaller  $\Delta t$ , you would look at the infiltration loss every 30 minutes and sum them all up when you're finished.)

$$\text{Then, Lawn Infiltration} = 0.15 \text{ in/hour} * 24 \text{ hours} = \underline{3.6 \text{ inches}}$$

$$\text{Lawn Runoff} = 0.5'' - 0.25'' - 3.6'' = -3.35 \text{ inches (or no runoff)} \rightarrow 0''$$

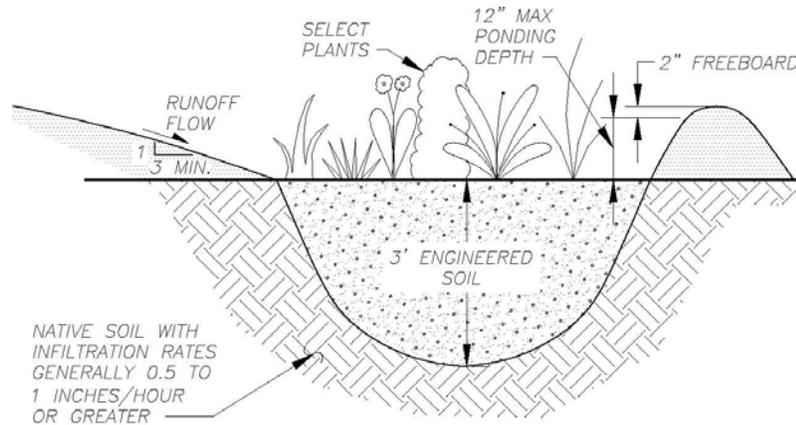
Total Site Runoff (in inches per acre)

$$= \frac{(\text{Roof Runoff} * \text{Roof Area}) + (\text{Pavement Runoff} * \text{Pavement Area}) + (\text{Lawn Runoff} * \text{Lawn Area})}{\text{Site Area}}$$

Total Site Runoff in inches per unit area =  $(0.42 * 0.2) + (0.42 * 0.5) + (0 * 0.2) / 1$

$$= \underline{0.336 \text{ inches/acre}}$$

**Part 2 – Size the bioretention area footprint.**



Sizing equation for a standard bioretention facility:

$$A_r = \left( \frac{TIV * d_{es}}{I * (P_d + d_{es}) * t} \right)$$

Where:

$A_r$  = Minimum bioretention area Footprint (square feet)

$TIV$  = Target Infiltration Volume (cubic feet)

$P_d$  = Depth of Poned Water (ft)

$d_{es}$  = Depth of Engineered Soil (ft)

$I$  = Infiltration rate of the native soil at the bottom of the bioretention area (ft/hour)

$t$  = Facility drain-down time (hours) (48 hours maximum)

To get TIV, convert the inches of runoff per acre from Part 1 to a volume in cubic feet.

$$TIV = 0.336 \text{ inches/acre} * 1 \text{ acre} * 43,560 \text{ ft}^2/\text{acre} * 1 \text{ foot}/12 \text{ inches} = \underline{1,219.7 \text{ ft}^3}$$

The infiltration rate in the sizing equation should be the design infiltration rate, based on testing and an appropriate factor of safety. For this example, let's assume that value is the same as the minimum infiltration rate used in Part 1 (0.15 inches/hour = 0.0125 ft/hour).

$$A_r = \frac{1,219.7 \text{ ft}^3 * 3 \text{ ft}}{0.0125 \frac{\text{ft}}{\text{hour}} * (1 \text{ ft} + 3 \text{ ft}) * 48 \text{ hours}}$$

$$\underline{A_r = 1,524.6 \text{ ft}^2}$$

*The bioretention facility can take many different shapes, based on the site configuration. For example, a long linear facility would be approximately 15' x 100'.*

When designing the facility footprint, it is also important to consider the sediment loading of the contributing area. To improve the life of bioretention facilities that are accepting water from parking lots and roadways, the general rule of thumb is to make the facility footprint at least 5% of the contributing impervious area. So, let's compare:

Our contributing impervious area is 0.8 acres or 34,848 square feet. Five percent of that is **1,742.4 ft<sup>2</sup>**. *So, we might consider increasing the size slightly to improve long-term performance.*

## LID Techniques for Common Site Constraints

LID Technique	Common Local Site Constraints					
	Poorly Infiltrating Soils <sup>1</sup>	High Ground Water	Contaminated Soils or Runoff	Bedrock	Frozen Conditions with a bypass or overflow route <sup>2</sup>	Frozen Conditions <sup>3</sup>
<b>Bioretention</b>	✓	✓	✓	✓	✓	
Pervious Pavement	✓				✓	✓
Filter Strips	✓	✓	✓	✓	✓	
Infiltration Basin	✓				✓	✓
Chamber Systems	✓					✓
Vegetative Swale	✓	✓	✓	✓	✓	

- ✓ Works well
- ✓ Works well under specific design conditions

## Stormwater Controls Recommended Selection Considerations

Stormwater Control Technique	Type of Runoff Control		Implementation Considerations		
	Rate Control	Volume Reduction	Accepts Hotspot Runoff	Separation from Groundwater <sup>8</sup> (feet)	Separation from Drinking Water Lines (feet)
Bioretention Facilities	Moderate	Moderate	Yes <sup>3</sup>	29	0
Soakaway Pits	Moderate	High	No <sup>4</sup>	4	25
Infiltration Basins	Moderate	High	No <sup>4</sup>	4	10
Infiltration Trenches	Moderate	High	No <sup>4</sup>	4	25
Vegetative Swales	Moderate	Moderate	No <sup>5</sup>	4	0
Pervious Pavement	Moderate	High	No	2	25
Chamber Systems	High	High	No <sup>4</sup>	4	25
Wet Ponds	High	Low	Yes <sup>6</sup>	N/A <sup>6</sup>	25
Dry Ponds	High	Low <sup>1</sup>	Yes <sup>6</sup>	N/A <sup>6</sup>	10
Oversized Pipes	High	Low	Yes <sup>7</sup>	N/A	10
Sedimentation Basins	High	Low	No	4	10
Filter Strips	Low - Moderate <sup>2</sup>	Low – Moderate <sup>2</sup>	Yes	4	0
Oil and Grit Separators	Low	Low	Yes	N/A	10
Constructed Wetlands	Moderate - High	Moderate	Varies <sup>5</sup>	N/A	25
Rainwater Harvesting	Moderate	Moderate	No	Varies <sup>9</sup>	10

(1) May provide some volume reduction depending on permeability of native soil.

(2) Increased performance when level spreaders are incorporated into the design.

(3) Appropriate plants and other vegetation must be selected for excepted pollutant load.

(4) Yes, if runoff pretreatment is provided.

(5) Yes, under specific conditions.

(6) If hotspot runoff is anticipated, required separation from groundwater is 4 feet.

(7) Hotspot runoff still requires treatment.

(8) Minimum separation distance between the seasonal high groundwater table elevation and the bottom of structural stormwater controls.

(9) Modifications are available for locations with high groundwater.