The most current version of the Municipality of Anchorage Standards and Specifications (M.A.S.S.) is provided on the Municipality website at www.muni.org/projectmgmt/publications.cfm. Notifications will be sent when updates are made to the document, but each user of M.A.S.S. is responsible to verify that they are using the most current version.

If you notice any errors or omissions, or have suggestions for improvements to M.A.S.S., send an email to massupdate@muni.org. The emails will be used to develop future updates to M.A.S.S.
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SECTION 10.01 DEFINITIONS

In these Contract Documents, the following words or expressions shall have the meaning given below:

- **AAC** Alaska Administrative Code (x AAC y where x=Title, y=Chapter)
- **AASHTO** American Association of State Highway and Transportation Officials
- **ACI** American Concrete Institute
- **ACS** Alaska Communications Systems
- **ADEC** State of Alaska, Department of Environmental Conservation
- **ADOT/PF** State of Alaska, Department of Transportation and Public Facilities
- **AMC** Anchorage Municipal Code (AMC x.y where x=Title, y=Chapter)
- **ANSI** American National Standards Institute
- **API** American Petroleum Institute
- **APWA** American Public Works Association
- **AS** Alaska Statute (AS x.y where x=Title, y=Chapter)
- **ASA** American Standard Association
- **ASTM** American Society for Testing and Materials
- **ATM** Alaska Traffic Manual
- **AWS** American Welding Society
- **AWWA** American Water Works Association
- **AWWU** Anchorage Water and Wastewater Utility
- **ENSTAR** Enstar Natural Gas Company
- **EPA** Environmental Protection Agency
- **GCI** General Communications Incorporated
- **M.A.S.S.** Municipality of Anchorage - Standard Specifications
- **ML&P** Municipal Light and Power
- **MUTCD** Manual of Uniform Traffic Control Devices
- **NEC** National Electrical Code
- **NEMA** National Electrical Manufacturer's Association
- **NESC** National Electrical Safety Code
- **NFS** Non-Frost Susceptible per Division 20 – Standard Construction Specifications for Earthwork
- **OSHA** Occupational Safety and Health Act or Administration

**Addendum (Addenda)** - Written or graphic communications issued prior to the execution of the Contract which modify or interpret the Bidding Documents and become part of the Contract Documents upon execution of the Contract.
Additional Work - Work not specifically provided for in the Contract as awarded, but which is consistent with the original scope of Work and a price for similar work is provided in the Contract.

Assembly - The Municipal Assembly of the Municipality of Anchorage.

Bid Proposal - The written proposal of the Bidder, on the form furnished, for the Work contemplated.

Bidder - Any individual, firm, partnership, corporation, or combination thereof formally submitting a Bid for the Work contemplated, and acting directly or through an authorized representative.

Bidding Documents - The Invitation to Bid, Special Provisions, Specifications, Forms, Schedules, Bidder's Checklist, proposed Contract Documents, and all Addenda.

Bid Guarantee - The security furnished by the Bidder as a guarantee to enter into a Contract for the Work contemplated if the Bidder is awarded the Contract.

Change Order Proposal - A written proposal prepared by the Contractor describing and documenting added costs or time extensions that the Contractor claims have been incurred due to unforeseen Work and other matters not contemplated or adequately provided for in the Contract Documents.

Change Order or Contract Amendment - A written agreement entered into between the Contractor and the Owner to amend the Contract Documents, or to otherwise provide for unforeseen Work and other matters not contemplated by or adequately provided for in the Contract Documents.

Cleanup - The restoration of all surface improvements including contouring; grading; required seeding and landscaping; all signage; removal of construction debris; restoration and cleaning of haul roads; and all other associated Work.

Contract - The four (4) page form agreement entitled "Contract" executed by the Contractor and then by the Owner on behalf of the Municipality. The Contract represents the entire and integrated agreement between the parties and supersedes all prior negotiations, representations, or agreements, either written or oral.

Contract Date - The date on which the Contract is executed by the Owner.

Contract Completion Date - The date specified in the Contract Documents for the full completion of all Work required by the Contract Documents, except as otherwise provided in the Contract.

If a number of calendar days is specified in the Contract Documents for the completion of the Contract, the Contract Completion Date shall be those specified number of days after the effective date of the Notice to Proceed, including authorized time extensions.
**Contract Documents** - The Contract and those documents described in the Contract. The Contract Documents can only be amended by written Change Order. Instructions, clarifications, and directives issued by the Engineer under Section 10.05, Article 5.1 - Authority of the Engineer are not Contract Documents.

**Contract Item** (Bid Item, Pay Item) - A specifically described unit of Work for which a price is provided in the Contract.

**Contractor** - The individual, firm, corporation, partnership, joint venture, limited liability corporation, or limited liability partnership, or authorized agent thereof, executing the Contract and performing the Work under the terms of the Contract Documents.

**Days** -

**Calendar:** Unless otherwise designated in the Special Provisions, days as used in the Contract Documents shall be understood to mean calendar days.

**Working:** A working day is defined as any day on which the Contractor is required to Work by the Contract Documents or any other day not otherwise defined herein as a non-working day.

**Non Working:** A non-working day is defined as Sunday, a recognized holiday, a day on which the Contractor is specifically required by the Special Provisions to suspend construction operations, or a day on which a suspension order is in effect. Recognized holidays shall be: New Year's Day, President’s Day, Martin Luther King Day, Memorial Day, Independence Day, Labor Day, Veterans Day, Thanksgiving Day, and Christmas Day. When any of the above days falls on a Saturday, the preceding Friday shall be counted as a holiday. When any of the above days falls on a Sunday, the following Monday shall be counted as a holiday.

**Deviation Request** – A Contractor initiated request for a revision in the requirements of the Contract Documents.

**Drawings** - The maps, plans, sheets, or other graphic illustrations listed and referred to in the Contract.

**Engineer** - The Engineer identified in the Notice to Proceed as being the authorized agent and/or representative of the Owner.

**Extra Work** - Work not within the original scope of Work but is determined by the Engineer to be essential for the satisfactory completion of the Contract.

**Final Acceptance Date** - The date on which the Work in its entirety has been constructed, inspected, and accepted pursuant to the provisions of Section 10.05, Article 5.26 - Final Inspection.

**Furnish** - Purchase and deliver to the Project.
**Indicated** - Shown on the Drawings, noted on Drawings, specified, or a combination thereof.

**Inspector** - The authorized agent and/or representative of the Engineer or Owner assigned to observe the Work.

**Install** - Set in place and make usable.

**Liquidated Damages** - The amount prescribed herein to be paid to the Owner, or to be deducted from any payments due or to become due the Contractor, for each calendar day's delay in completing the whole or any specified portion of the Work beyond the time allowed in the Contract or as extended by Change Order.

**M.A.S.S. or MASS** - Municipality of Anchorage Standard Specifications. Internal references to M.A.S.S. are in the form of Division, Section, Article, SubArticle.

**Municipality or Anchorage** - Municipality of Anchorage.

**Necessary** - Needed, as reasonably inferred from the Contract Documents, in order to make the Work complete and available for use.

**Notice-to-Proceed** - The written communication, issued by the Owner to the Contractor authorizing him to proceed with the Work, which identifies the Engineer and establishes the time of commencement and date of completion.

**Notice-to-Resume** - The written notice issued by the Engineer which terminates a period of suspension of Work, reinstates the counting of Contract time and requires the Contractor to resume Contract Work.

**Or Equal** - Whenever a material, article, or piece of equipment is identified on the Drawings or in the Project Manual by reference to manufacturers' or vendors' names, trade names, catalog numbers, etc., it is intended merely to establish a standard; and any material, article, or equipment of other manufacturers and vendors which in the opinion of the Engineer will perform in an equal or better manner the duties imposed by the general design shall be considered equally acceptable provided the material, article, or equipment so proposed will not require a change in the related Work.

**Owner** - The Department or Agency of the Municipality of Anchorage identified in the Contract. Owner does not include those Municipal employees, such as the Building Official or Fire Marshal and their staffs, who enforce certain building, health and safety, and fire codes.

**Performance and Payment Bond** - The form of security approved by the Municipality, furnished by the Contractor and his Surety guaranteeing the complete and faithful performance of all the obligations and conditions placed upon the Contractor by the Contract.
**Product Data** - Brochures, illustrations, diagrams, and other information furnished by the Contractor to illustrate a material, product, or system for some portion of the Work.

**Project** - The total construction of which the Work performed under the Contract Documents may be the whole or a part.

**Project Manual** - The bound documentary information organizing the documents into two general categories: (1) those describing the Bidding Documents, and (2) the Contract Documents.

**Provide** - Furnish and install; perform all Work necessary to complete the Work.

**Purchasing Officer** - That person within the Municipality of Anchorage who is vested under Anchorage Municipal Code with all authority pertaining to the procurement of supplies, services, and construction.

**Request For Information (RFI) or Clarification** – A written document prepared by the Contractor to request information or clarification of the Contract Documents. Each RFI shall be numbered consecutively and a log maintained of submittals and responses.

**Samples** - Physical examples which illustrate materials, equipment, or workmanship and establish standards by which the Work or a product will be judged.

**Shop Drawings** - All drawings, diagrams, illustrations, schedules, and other data which are prepared by the Contractor, a Subcontractor, manufacturer, supplier, or distributor which illustrate the equipment, material, or some portion of the Work.

**Special Provisions** - That portion of the Specifications entitled Special Provisions setting forth conditions or requirements unique to the Work.

**Specifications** - The directions, requirements, explanations, terms, and provisions pertaining to the Work.

**Subcontractor** - Any individual, firm, corporation, partnership, joint venture, limited liability corporation, or limited liability partnership, or authorized agent thereof, acting for or on behalf of the Contractor in the performance of a part of the Contract. This does not include those working for hire or suppliers of material or equipment.

**Substantial Completion Date** - The date upon which the Work required by the Contract has been inspected and in the opinion of the Engineer is essentially completed and available for the Owner's beneficial use for the purpose and in the manner intended by the Contract Documents, including satisfactory acceptance of all required testing.

**Surety** - The Company or Association which is bound with and for the Contractor for the acceptable performance of the Contract and for the payment of all obligations arising out of the Contract. Where applying to the Bid Guarantee, it refers to the Company or Association which shall forfeit the sum of the Guarantee when the Bidder fails to execute the Contract after the Bid is accepted by the Municipality.
Technical Specifications - Divisions 20 through 80 of MASS.

**Time and Material Work** - Work performed by the Contractor at the written direction of the Engineer for which no item is provided in the Contract and for which no unit price or lump sum basis can be agreed upon.

**Utility Company** - The entity (person, corporation, company, agency, or other) that furnishes service(s) (including petroleum and petroleum products, electricity, sanitary sewer, communications, water, natural gas, traffic signal control, and storm sewer) utilizing conduit, pipe, wire, cable, or other transmission lines.

**Winter Suspension** - The period of time through which no field work is accomplished due to adverse winter weather conditions as permitted by Section 10.05, Article 5.32 – Winter Suspension.

**Work** - The furnishing of all labor, materials, equipment, and other incidentals necessary or convenient for the successful completion of all the duties and obligations imposed by the Contract.

**Working Titles** – Working titles which may have masculine genders such as “workman” and “flagman” or are pronouns such as “he,” “his,” and “him” are utilized in the Contract Documents for the sake of brevity and are intended to refer to persons of either gender.

**Written Notice** - A written communication delivered in person to the individual or to a member of a firm, or agency, or to an officer of the corporation, or agency for whom it is intended, or sent by mail to the business address stated in the Contract Documents. Communication via electronic facsimile (FAX) or electronic mail shall not be considered proper written notice.
SECTION 10.02  BIDDING REQUIREMENTS AND CONDITIONS

Article 2.1  Examination of Bidding Documents and Site

The Bidder shall examine carefully the site of the proposed Work and the Bidding Documents before submitting a Bid. The submission of a Bid shall be an admission that the Bidder has made such examination and is satisfied as to the conditions to be encountered in performing the Work and as to the requirements and accuracy of the Bidding Documents. The Bidder further declares that the amount(s) bid are for the total Work as contained in the Contract Documents.

The Municipality assumes no responsibility for any understanding or representations concerning conditions made by any of its officers, agents, or employees prior to the execution of the Contract, unless such understanding or representations are expressly stated in the Bidding Documents or Addenda.

When soils boring data is provided by the Bidding Documents, the Bidder shall assume responsibility for any conclusions he may draw from such data. The Bidder shall be responsible for obtaining and analyzing such additional data as he may require and shall be responsible for conclusions drawn from that information.

By submitting a bid, the Contractor declares that he has carefully examined the Contract Documents, that he has full knowledge thereof and that he has investigated the site and satisfied as to the conditions affecting the Work, including, but not limited to those bearing upon transportation, disposal, handling and storage of materials, availability of labor, water, electrical power, roads, and uncertainties of weather, physical conditions at the site including all existing utilities, the conformation and conditions of the ground, the character of equipment and facilities needed preliminary to and during prosecution of the Work. The Contractor further declares that he is satisfied as to the character, quality and quantity of surface and subsurface materials or obstacles to be encountered insofar as this information is reasonably ascertainable from an inspection of the site, including all prior exploratory work, as well as from information presented by the Drawings and Specifications made a part of the Contract. Any failure by the Contractor to acquaint himself with the available information shall not relieve him from responsibility for estimating properly the difficulty or cost of successfully performing the Work.

The Bidder is encouraged to support the Municipality’s Disadvantaged and Woman-Owned Business Enterprises (DBE/WBE) program. The Bidder shall comply with the DBE/WBE Specifications if included in the Contract. The Bidder shall comply with the requirements of the Equal Opportunity Special Provisions as contained in the Bid and resulting Contract.

Article 2.2  Interpretation or Correction of Bidding Documents

Bidders shall notify the Purchasing Officer promptly of any error, omission, or inconsistency that may be discovered during examination of the Bidding Documents and the proposed construction site. Requests from Bidders for interpretation or clarification of the Bidding Documents shall be made in writing to the Purchasing Officer and shall arrive at least seven (7) working days prior to the date for opening Bids. Oral questions may be presented at a pre-bid conference if one is provided for in the Bidding Documents.
Interpretations, corrections, or changes, if any, to the Bidding Documents shall be made by Addendum. Bidders shall not rely upon interpretations, corrections, and changes made in any other manner, including orally at the pre-bid conference. Interpretations, corrections, and changes shall not be binding unless included in an Addendum.

**Article 2.3 Preparation and Submission of Bids**

Bids shall be submitted on the forms furnished and must be manually signed. Bids should be submitted in a sealed envelope addressed as indicated in the Invitation to Bid and on which the Invitation Number is plainly marked.

Bidders must quote on all items, unless specifically allowed to bid on only a portion of the items within the Invitation to Bid and they are warned that failure to do so shall disqualify the Bid. The Bidder may bid an item at no cost and shall insert the words “no cost” in the space provided for any no cost item. When quotations on all items are not required, Bidders should insert the words “no bid” in the space provided for any item where no quotation is made. If erasures or other changes appear on the forms, each such erasure or change must be initialed by the person signing the Bid.

Bids shall specify a unit or lump sum price, typed or written in ink, for each bid item called for. If the bid is submitted in both words and figures and there is a discrepancy between the written words and figures, the written words shall govern. In case of error in the extension of prices, the unit price shall govern. Bids may be rejected if they show any omissions, alteration of the forms, additions not required, conditional or alternate bids not required, qualified bids, or irregularities of any kind.

**Article 2.4 Bid Guarantee**

Each Bid shall be accompanied by a certified check, cashier’s check, or Bid Bond, in the amount of ten percent (10%) of the total amount of the Bid if the total amount of the bid is $100,000 or more, with good and sufficient corporate surety acceptable to the Municipality. If the total bid amount of the bid is less than $100,000, the bid shall be accompanied by a bid guarantee, in the form specified above, in the amount of $2,000. Bid Guarantees for the three (3) low Bidders shall be held until the Contract is executed. All other Bid Guarantees will be returned within seven (7) days of the bid opening. Power-of-Attorney for the person signing the Bid Bond for the Surety must be submitted with the Bid Bond.

**Article 2.5 Disadvantaged and Woman Owned Business Enterprises (DBE/WBE) Requirements**

Each Bid shall be accompanied by those (DBE/WBE) Forms provided by the Purchasing Officer and as required by the bidder’s checklist. The Bidder shall comply with the Municipality of Anchorage Office of Equal Opportunity Disadvantaged Business Enterprise Program Specifications for Municipal Contracts, including submittal requirements for bids, prior to and after award. See the Disadvantaged and Women-Owned Business Enterprises (D/WBE) Specifications Section of the Invitation to Bid for participation goals for each particular project.
SECTION 10.03  AWARD AND EXECUTION OF CONTRACT

Article 3.1  General

The provisions of this Section are intended to be supplemental to, and not to replace AMC Title 7.

Article 3.2  Receipt and Opening of Bids

Bids shall be submitted to the Municipal Purchasing Department prior to the time of opening specified in the Invitation to Bid and the exact date and time of receipt of Bids shall be recorded. Late Bids shall not be considered, but will be held unopened until the time of award and then returned to the Bidder unless other disposition is requested or agreed to by the Bidder. Time of Bid receipt shall be determined by the time stamp of the Municipal Purchasing Department.

Facsimile bids shall not be considered. Modification by facsimile of bids already submitted shall be considered if received in writing by the Purchasing Officer prior to the time of bid opening fixed in the Invitation to Bid. Facsimile modifications shall not reveal the amount of the original or revised bid. Modifications shall state a plus or minus to the affected bid item.

No liability shall attach to the Municipality for the premature opening of or the failure to open a Bid not properly addressed and/or identified.

Bids may be withdrawn in person, by written letter, or by facsimile when such request is received by the Purchasing Officer PRIOR to the time specified for receipt of Bids.

If more than one Bid is offered by any one party, by or in the name of his clerk, partner, or other person, all such Bids shall be rejected. A party who has quoted prices to a Bidder is not thereby disqualified from quoting prices to other Bidders, or from submitting a Bid directly for the Work.

Article 3.3  Bidder Qualifications

All bidders shall hold a valid Alaska Contractor’s license per AS 08.18.

The Purchasing Officer reserves the right to determine whether or not a Bidder is a responsible Contractor. The Purchasing Officer may require the Bidder to submit such information as he may deem necessary to determine a bidder’s responsibility. Failure or refusal on behalf of the Bidder to submit the required information, in whole or in part, may be grounds for the Purchasing Officer to determine the Bidder as non-responsible.

Any determination that a Bidder is non-responsible shall be made by the Purchasing Officer using AMC 7.20.030 as the basis for that determination. Such determination shall be made in writing to the Bidder setting forth the reasons for such determination and the Bidder’s right to request a review of this determination.
A Bidder's representations concerning his qualifications shall be construed as a covenant under the Contract. Should it appear that the Bidder has made a material misrepresentation, the Owner shall have the right to terminate the Contract for the Contractor's breach, and the Owner may then pursue such remedies as provided in the Contract Documents or as provided at law or equity.

If a Contractor has had a contract terminated by the Owner for cause as provided in Section 10.05, Article 5.28 – Termination of Contract by Owner, the Contractor may not be allowed to bid on the Owner's future contracts for a period of two (2) years. This two (2) year period shall commence from the date of the termination of the Contractor by the Owner.

Article 3.4 Action on Bids

The Municipality reserves the right to reject any and all Bids, and to waive any informalities and irregularities in Bidding or award of the Contract.

The Municipality may reject any bid which is unbalanced if it is in the best interest of the Municipality to do so. A bid is unbalanced when, in the opinion of the Purchasing Officer, it allocates a disproportionate share of costs or profit, or both, to the price of one (1) or more items of Work and reduces the share of costs or profit, or both, allocated to the price of another item or items of Work, and if there is a reasonable possibility that the bid will not result in the lowest overall cost of the Work to the Municipality.

Unless otherwise stated in the Bidding Documents, the Contract, if awarded, shall be awarded to the responsible Bidder who submits the lowest responsive Bid. When the Bidding Documents contain a basic bid and additive or deductive alternates, only the total of the basic bid and the alternates to be awarded shall be used to determine the low Bidder.

When the Bidding Documents contain a basic bid and additive alternates, the low Bidder shall be determined by the lowest combination of the basic bid and as many additive alternates as may be selected within the funds available. For evaluation purposes, additive alternates shall be chosen in the order listed in the Bid. The Purchasing Officer may bypass any additive alternate whose selection would cause the Contract to exceed the funds available.

When the Bidding Documents contain deductive alternates, the low Bidder shall be determined by the lowest basic bid. If the lowest basic bid exceeds the funds available, the low Bidder shall be determined by eliminating deductive alternates in the order listed in the Bid until the award can be made within the available funds. The Purchasing Officer may bypass any deductive alternate to maximize the use of available funds.

The Purchasing Officer shall use the list of priorities in the bid schedule to determine the low bidder only. After determining the low bidder, an award may be made on any combination of bid items provided: (1) it is in the best interest of the Municipality; (2) funds are available at the time of award; and (3) the low bidder's price for the combination to be awarded is less than the price offered by any other responsive, responsible bidder.
The amount of the Contract shall be the total sum of the amounts computed from the estimated quantities and unit prices and/or the lump sum awarded by the Purchasing Officer and specified in the Contract.

Unless otherwise specified in the Invitation to Bid, Notice of Award or rejection shall be given within forty-five (45) days of Bid opening. The notice shall be in writing and signed by the Purchasing Officer or authorized designee. A Notice of Award shall constitute an acceptance of the Bid. No other act(s) of the Municipality or its representatives shall constitute an acceptance of a Bid. The acceptance of a Bid shall bind the successful Bidder to execute the Contract.

**Article 3.5 Bonds, Insurance, EEO and DBE/WBE Forms**

The successful Bidder shall furnish the Purchasing Officer a Performance and Payment Bond in the full amount of the Contract. The Bond is for the faithful performance of the Contract in all respects including, but not limited to, payment for all materials and labor. All alterations, extensions of time, additional Work and other changes authorized by the Contract Documents may be, at the option of the Owner, made without securing the consent of the Surety or Sureties. Contractor shall provide the Bond with a good and sufficient corporate surety acceptable to the Municipality. A Power-of-Attorney for the person signing the Bond for the Surety must be submitted with the Bond.

The successful Bidder shall furnish the Purchasing Officer a certificate of insurance pursuant to the provisions of Section 10.06, Article 6.9 – Insurance, and shall execute and furnish the Purchasing Officer all the required EEO and DBE/WBE forms.

The Bidder shall exercise positive efforts to comply with the Equal Employment Opportunity and Disadvantaged/Woman Owned Business Enterprise requirement policies of the Municipality. The Bidder shall familiarize himself with the Equal Opportunity Special Provisions and Disadvantaged/Woman Owned Business Enterprise Specifications for Municipal Contracts, including submittal requirements for bids, prior to award and after award.

**Article 3.6 Execution of Contract**

The successful Bidder shall execute the Contract and furnish the required bonding and insurance within five (5) working days after Notice of Award of the Contract is issued.

The Contract shall be considered executed by the successful Bidder when two (2) copies of the Contract, signed by an authorized representative of the Contractor, and the required bond and insurance certificate are received by the Purchasing Officer. Failure or neglect of the Contractor to execute the Contract within the time specified may result in a forfeiture of the Bid Guarantee and award of the Contract to the next lowest Bidder.

The Owner shall execute the Contract within ten (10) working days after execution by the Contractor as set forth above. The date the Contract is executed by the Owner is the Contract Date. The rights and obligations provided for in the Contract shall become effective and binding upon the parties as of the Contract Date.
The Contractor shall be supplied upon request with twelve (12) sets of the Contract Documents exclusive of M.A.S.S. The Contractor may obtain any additional documents from the Engineer by compensating the Owner for the cost of the printing involved.

The Notice-to-Proceed shall be issued within seven (7) working days after the Contract Date unless otherwise specified in the Special Provisions. The effective date of the Notice to Proceed shall be within ten (10) working days of the Contract Date. The Engineer or his authorized representative, the Engineer’s address, and the completion date shall be designated in the Notice-to-Proceed.

**Article 3.7 Contractor’s Warranty**

The Contractor shall warranty all materials and workmanship for one (1) year from the Final Acceptance Date unless otherwise specified in the Special Provisions. This warranty shall require the Contractor to remedy promptly, without cost to the Owner, any and all defects in material and workmanship including any consequential damages resulting from defective materials or workmanship.

All warranty Work shall be subject to the same Contract provisions, including materials, quality of work, authority of the Engineer and inspection, as provided for in the original Work. All warranty Work shall be at the sole expense of the Contractor. All materials and workmanship directly or indirectly involved in repairs or replacements under this Article shall carry an extended warranty of not less than one (1) year from the date of the Engineer’s written acceptance of the repair or replacement Work, or through the warranty period for the original project Work, whichever is longer.

If the defect, in the opinion of the Engineer, is of such nature as to demand immediate repair, the Owner shall have the right to take corrective action and the cost thereof shall be borne by the Contractor.

If the Contract includes Work in different geographic locations, then the Work in each location may be accepted and the warranty period for that location may begin independent of the completion of the Work in the other locations.
SECTION 10.04    SCOPE OF WORK

Article 4.1    Intent of the Contract Documents

The intent of the Contract Documents is to provide for the execution and completion of the Work in its entirety. Except as otherwise specifically provided herein, the Contractor shall furnish all permits, transportation, handling, storage of materials, labor, tools, implements, machinery, supplies, materials, water, heat, utilities, and incidentals, and shall do all things necessary to perform and to complete the Work.

When words that have a common technical or trade meaning are used to describe Work, materials, or equipment, such words shall be interpreted in accordance with that meaning.

Reference to Standard Specifications, manuals, or codes of any technical society, organization, or association, or to the Laws or Regulations of any governmental authority, whether such reference be specific or by implication, shall mean the latest Standard Specification, manual, code or Laws or Regulations in effect at the time of opening of Bids, except as may be otherwise specifically stated.

However, no provisions of any referenced Standard Specification, manual or code (whether or not specifically incorporated by reference in the Contract Documents) shall be effective to change the respective duties and responsibilities of the Owner, the Contractor, or the Engineer nor any of their consultants, agents, or employees from those set forth in the Contract Documents.

With reference to Drawings, the order of precedence is as follows:

1. Figures (numerals) govern over scaled dimensions.
2. Detailed Drawings govern over general Drawings or Standard Details.

Article 4.2    Interpretation of Contract, Specifications, and Drawings

The Contract Documents are intended to be complementary and to describe and provide for a complete description of the entire scope of Work. A requirement occurring in one Section of the Contract Documents is as binding as though occurring in all.

In cases of conflict in the requirements of the Contract Documents such conflict shall be reconciled by the acceptance of the following order of precedence for the various Contract Documents: (1) the Contract; (2) the Bid; (3) Special Provisions; (4) the Technical Specifications; (5) the Drawings; (6) the Standard General Provisions; and (7) specifications incorporated by reference in any of the above.

The apparent silence of the Specifications and Drawings as to any detail or the apparent omission from them of a detailed description concerning any point, shall be regarded as meaning that only the best general practice is to prevail and that only approved material and workmanship of first quality are to be used.
The Contractor shall carefully study and compare the Contract Documents and shall at once report to the Engineer any error, inconsistency or omission he may discover including any requirement which may be contrary to any law, ordinance, rule, regulation, or order of any public authority bearing on the performance of the Work.

The Contractor shall take no advantage of any errors or omissions in the Specifications and Drawings or of any discrepancies in or between same. Work knowingly performed by the Contractor as a result of an error or omission in the Drawings and/or Specifications where such error or omission is not called to the attention of the Engineer shall be at the Contractor's risk and expense.

All Contractor-initiated requests for interpretation or clarification of the Contract Documents shall be accompanied by a completed Request For Information (RFI) form. Each request shall clearly and completely state the basis for lack of clarity in the Contract Documents and shall refer to the applicable Specifications, Drawings and details that give rise to the request. If not provided in the Contract Documents, a copy of the RFI form shall be obtained from the Engineer. Engineer shall respond to the RFI in writing within ten (10) working days. The Engineer's response shall serve to clarify and interpret the existing Contract requirements and shall not be construed as requiring or directing a change in the Work.

Article 4.3  Submittal List

The Contractor shall complete, submit, and/or comply with all requirements as indicated in the Submittal List located in the Bidding Documents. The Contractor is hereby advised the Submittal List is not an all-inclusive document. The Submittal List does not relieve the Contractor from his obligation to comply with all submittals, certifications, or other requirements as specified in M.A.S.S., the Special Provisions, or the Drawings. The Contractor is responsible to determine that all submittals, certifications, and/or requirements are met, whether or not specifically addressed in the Submittal List.

Article 4.4  Estimates of Quantities

The quantities shown in the Bid, whether for a unit price contract or a combination of a lump sum contract and unit price contract, are approximate only and are not to be taken to be either representations or warranties. Since quantities in the Contract Documents are estimates only, actual quantities may increase or decrease without constituting a change in the Work unless the increase or decrease involves a major bid item which varies by more than twenty-five percent (25%) as provided below. The cumulative variations in quantities together with other changes in the Work shall not increase the Contract amount by more than the limits set forth in AMC 7.15.080, without Assembly approval.

Article 4.5  Increased Quantities

The Owner reserves the right to increase the quantity of any bid item. There may be an adjustment of unit prices of major bid items where the actual quantity of the item is increased by more than twenty-five percent (25%) of the estimated quantity. A major bid item is defined as any item where the total bid price of the item exceeds twice the average
bid price of an item. Twice the average bid price of an item is calculated by the following formula:

\[
\frac{T}{I} \times 2
\]

where "T" equals total bid amount of the schedule in which the item appears and "I" equals the number of items in the schedule in which the item appears.

If a major bid item quantity increases by more than twenty-five percent (25%), the unit price for one hundred and twenty-five percent (125%) of the estimated quantity shall remain as set forth in the Bid and the price for additional quantities above the one hundred and twenty-five percent (125%) for said item may be negotiated for a greater or lesser amount upon the demand of either the Owner or the Contractor.

A unit price increase may be allowed only if the Contractor can substantiate to the satisfaction of the Engineer that he incurred increased unit costs in providing the additive quantities over and above that unit cost incurred in providing the estimated quantity of the bid item. A negotiated increase in price may include a maximum of ten percent (10%) for overhead and profit on increased costs.

A negotiated increase in price on any item shall not preclude a claim for increased costs on other items of the Work under Section 10.05, Article 5.21 - Claims for Additional Compensation.

A decrease in unit price for that quantity over one hundred and twenty-five percent (125%) of the estimated quantity shall be allowed only if the Owner establishes that the unit cost for such additional quantities is less than the unit cost for the estimated quantity. For this purpose, the Contractor shall provide job records as required by the Engineer.

**Article 4.6 Decreased Quantities**

The Owner reserves the right to decrease the quantity of any Bid item. There may be an adjustment of unit prices of major Bid items where the actual quantity of the item is decreased by more than twenty-five percent (25%) of the estimated quantity and the aggregate total of all quantity revisions decreases the total Contract amount by more than ten percent (10%). Change orders for extra Work shall not be used for this calculation. A major bid item is defined in Article 4.5 - Increased Quantities.

If a major bid item qualifies for a unit price adjustment, the Contractor shall be allowed, upon proper submittals, an allowance for overhead costs for the decreased quantity. The allowance shall be ten percent (10%) of the amount represented by the difference between the actually installed quantities and seventy-five percent (75%) of the Contract estimated quantities. Payment of this allowance does not preclude a claim for increased costs on other items of the Work under Section 10.05, Article 5.21 - Claims for Additional Compensation.
The provisions of this Article shall not apply to reduced quantities resulting from the termination, or partial termination, of the Contract for cause or for the Owner's convenience.

**Article 4.7 Reference Stakes and Surveying**

Bench Marks and/or reference points have been identified and/or placed initially by the Engineer, and the horizontal and vertical reference locations are indicated in the Drawings. The Contractor shall ensure that all construction surveying Work required is completed in strict conformity with Division 65 - Standard Construction Specifications for Construction Survey.

At various points throughout the Work, Contractor's operations may disturb existing survey monuments, bench marks, or referenced points. If Contractor disturbs these items, Contractor shall replace them at Contractor's expense. Contractor shall replace at Contractor's expense, survey monuments, bench marks, or reference points which, in the judgment of the Engineer, are outside the limits of the Work area and which are disturbed or destroyed by Contractor.

**Article 4.8 Work Incidental to the Contract**

Several items of Work, not covered in the Bid Proposal, are incidental to the cost of the Contract. These items shall include, but are not limited to, the following:

1. Type 1 Storm Water Pollution Plan
2. Utility location and verification.
3. Shoring utility and power poles, and protecting overhead and underground utilities.
5. Providing safe hauling routes for transporting excavation material, classified fill, other construction materials, and site access.
6. Dewatering of roadway excavation and pipe trenches, unless payment is provided elsewhere.
7. Resetting disturbed property corners or monuments.
8. Removal and reinstallation of manholes and catch basins unless identified as a bid item. Removal and replacement of storm drain and subdrain pipes and cleanouts unless identified as a bid item.
10. Furnishing and installing bonding and grounding conductors for electrical installations.
11. Post-construction cleanup.
12. Reimbursement to utilities for associated inspection or relocation.
13. Other items indicated on the Drawings or in these Specifications.
Article 4.9 Disposal Sites

Except as otherwise stated in the Special Provisions, the Contractor shall make his own arrangements, assume all costs, and obtain all necessary permits in connection with disposal sites. Contractor shall furnish a disposal site for trees, brush, out-sized boulders, and other objectionable debris or materials. Contractor shall dispose unusable excavation, unsuitable or surplus material at either the Anchorage Regional Landfill off Hiland Road or at a lawful, permitted disposal site furnished by Contractor and approved by Owner. The Anchorage Regional Landfill shall charge the current disposal fee for unusable excavation or any material containing stumps, brush, or other construction debris or materials. Contractor-furnished disposal sites shall conform to AMC 23.105 Grading, Excavation, and Fill.

Disposal sites shall be located and maintained in such a manner as to prevent a public nuisance.

When the Contractor makes proper arrangements for disposal on private property, he shall obtain written permission from the property owner or owners for such disposal sites and shall furnish the Engineer with a copy of this permission and the appropriate Municipal Permit for grading and/or fill. The written permission shall specifically provide that the property owner shall not hold the Municipality, its employees, agents, or consultants liable for use of or damage to this property. The Contractor shall be held liable for any trespass and/or property damage incurred outside of the disposal site.

Waste Disposal. Prior to construction, the Contractor shall submit a description of his plan for disposing of unsuitable materials and waste resulting from the Work under the Contract. If any material is disposed of in unauthorized areas, the Contractor shall remove the material and restore the area to the condition of the adjacent undisturbed areas at no cost to the Owner.

Article 4.10 Protection of Persons, Property and Environment

The Contractor shall be responsible for initiating, supervising, and maintaining all safety programs and precautions in a manner to prevent damage, injury, or loss to the Work employees, site visitors, the public, the environment, and property. These safety requirements are applicable to the Work whether on-site or off the site for Work under the control, custody, or care of the Contractor. These responsibilities apply to all adjacent sites and their improvements including landscaping, walks, roadways, structures, utilities and drainageways. If the Contractor encounters material on the site that may be reasonably identified as asbestos-containing material (ACM), polychlorinated biphenyl (PCB), or other hazardous materials not requiring abatement as part of the Work, the Contractor shall stop the Work immediately in the affected area and notify the Owner’s Representative and Owner in writing. The Work shall not be resumed in the affected area until a final determination has been made by the Engineer on the status of the material in question.
Until Contract completion, the Contractor shall be solely and continuously responsible, twenty-four (24) hours per day, seven (7) days per week, for the safety measures outlined in this Article, including, but not limited to, the following:

(a) Erecting and maintaining, as required by existing conditions and progress of Work, all safeguards for safety and protection, including barricades, danger signs, traffic control devices, and other warnings against hazards.

(b) Providing reasonable access at all times for emergency units such as the Anchorage Police Department, the Anchorage Fire Department, and the Anchorage Fire Department's Hazardous Materials Response Team.

During periods of suspension of Work, refer to Section 10.05, Article 5.24 – Suspension of Work and Article 5.32 – Winter Suspension, for areas of responsibilities.

Protection of Water Resources. The Contractor shall control the disposal of fuels, oils, bitumens, calcium chloride, acid, or any other harmful materials, both on and off the premises, and shall comply with applicable federal, state, and municipal laws concerning pollution of waterways while performing Work under the Contract. Special measures shall be taken to prevent chemicals, fuels, oils, greases, bituminous materials, and sewage from entering established drainages.

**Article 4.11 Private Property in Right-of-Way**

Unless otherwise specified in the Drawings and/or Specifications, the Engineer shall contact the property owner to remove any fences, trailers, sheds, machinery, or other miscellaneous personal property is located within the right-of-way, and/or utility easements which interferes with construction. If the property owner cannot be contacted or does not move the item(s) of personal property, the Engineer shall notify the Contractor, in writing, to remove the item(s) from the right-of-way to the property owner's lot or as otherwise directed by the Engineer. When removing personal property from the right-of-way, the Contractor shall take care not to damage the items. Any damage to the item(s) shall be repaired or the items replaced in kind by the Contractor at no cost to the Municipality.

Unless otherwise stated in the Drawings and/or Specifications, payment for the removal of personal property from the right-of-way and the setting of these items down on the owner's lot shall be an incidental item and no separate payment shall be made.

Payment for Work such as resetting fences or restoration of personal property items to their original or alternate locations shall be specified in the Drawings and/or Specifications. If payment for such Work is not otherwise specified in the Drawings and/or Specifications, such Work shall be considered incidental to the Contract unless otherwise negotiated with the Engineer.

**Article 4.12 Public Convenience and Access**

The Contractor shall conduct the Work in such a manner as to cause minimum inconvenience to pedestrians and vehicular traffic and to persons conducting commercial
enterprises or residing along the route of Work. Without prior approval of the Engineer, entrances or driveways of all kinds shall not be blocked for more than three (3) hours. Temporary pedestrian bridges, ramps, or culverts shall be provided and maintained at entrances and shall be adequate in width and strength for the service required. All Work involved in providing for construction, maintenance, and use of entrances and driveways is the responsibility of the Contractor and shall not be paid for separately and shall be considered incidental to the lump sum and/or unit prices contained in the Contract Documents.

The Contractor shall comply with this Article at all times. Under no circumstances shall the public be denied access to the adjoining lots unless proper notification to the property owners and/or tenants has been provided and an alternate access has been provided and approved by the Engineer. The Contractor shall provide the property owners and/or tenants written notification no less than forty-eight (48) hours prior to any closure of access.

The Contractor shall, prior to the commencement of Work, submit any written agreements between the Contractor and property owners regarding access and use of private property within the project limits for any purposes associated with this Project. Any such agreements shall indemnify the Municipality from any and all actions that result from activities of the Contractor.

The Contractor shall conduct his Work to insure that there is no disruption of mail service, school bus service as applicable, trash collection, access by emergency vehicles, or any unnecessary disruption of general access to any business or private residence.

If the Contractor’s Work is delayed because of any construction and/or transportation activities of nearby construction, regardless of whether authorized by the Owner, the Contractor shall not be entitled to additional compensation from the Owner. The Contractor may be entitled to an extension of time to the extent that such delay was unavoidable through reasonable efforts on the Contractor’s part. Except as to a possible entitlement to such an extension of time, the Contractor shall hold harmless, defend, and indemnify the Owner from and against any and all claims, damages, losses, and/or expenses, including attorney’s fees, by the Contractor or third-parties, arising directly or indirectly as a result of nearby construction and/or transportation impacts.

**Article 4.13 Traffic Control Plan**

The Contractor shall conduct his operations to minimize interference to vehicular traffic. The Contractor shall submit a Traffic Control Plan (TCP) showing street closures, partial closures, detours, and all pertinent traffic control. The Contractor’s TCP shall be reviewed by the Traffic Engineer and then submitted for approval to Development Services. Emergency units shall be provided vehicular access at all times. Construction operations involving a closure of an arterial street shall not commence until after 9:00 a.m. unless otherwise approved by the Traffic Engineer. Requests for arterial street closures shall be made sufficiently in advance of the planned closure to allow a minimum of seventy-two (72) hour advance notice to the public.
When a street closure or partial closure is approved, the Contractor shall provide and maintain adequate detour routes, either by appropriately signing existing streets or by the construction of temporary roadways. Detour routes using existing streets must be left in a condition at least equal to their condition immediately prior to use as a detour. The Contractor shall be responsible to provide, erect, and maintain barricades, fences, signs, flags, lights, flagmen, and any other devices necessary to insure traffic safety. Placement and design of signs, barricades, and other devices to be furnished and used by the Contractor shall conform with the standards specified in the latest edition of the Manual on Uniform Traffic Control Devices (MUTCD). Traffic signs no longer required shall be promptly removed. Where operations are performed in stages, only those devices necessary to the stage in progress shall be visible. It shall be the Contractor's responsibility to maintain all barricades, signs, and lights throughout the night hours, weekends, holidays, or other periods of inactivity. Should the Contractor fail in this maintenance obligation, the Owner may erect the necessary barricades, signs, and lights, and deduct the cost thereof from payments due the Contractor. Action by the Owner to erect barricades, signs, or lights does not relieve the Contractor's indemnification obligations set forth in Section 10.06, Article 6.10 - Indemnification.

The Municipal Traffic Department shall require a minimum of three (3) days for TCP review time for streets with the following classification:

1. Minor arterials and higher must be submitted at least ten (10) days prior to the scheduled closure (three (3) day review time, seven (7) day advertisement/public notice time).
2. Streets listed as collectors must be submitted no later than six (6) days prior (three (3) day review time, three (3) day advertisement/public notice time).
3. All other streets must be submitted no later than three (3) days prior (three day (3) day review time, no advertisement/public notice required).

The TCP shall conform to the standards in the latest edition of Part VI of the Manual of Uniform Traffic Control Devices (MUTCD), and shall also conform to the requirements in the latest edition and supplements of the Alaska Traffic Manual (ATM) prepared by the Alaska Department of Transportation and Public Facilities. When a conflict exists between M.A.S.S. and the ATM, the requirements of Specifications or any Special Provisions shall govern.

If Work shall be done within a State of Alaska right-of-way as identified in the Special Provisions, the TCP shall also be submitted to the State of Alaska, Department of Transportation and Public Facilities Regional Traffic Engineer for acceptance and approval.

Providing the TCP to the Engineer and the State of Alaska Department of Transportation and Public Facilities Regional Traffic Engineer shall be the responsibility of the Contractor and shall be paid for under the bid item “Traffic Maintenance” and no separate payment shall be made.
When, in the opinion of the Engineer, traffic maintenance is deficient, inadequate, improper, or conditions exist such that public safety would be adversely affected, or public convenience unnecessarily degraded, the Contractor shall be notified in writing by the Engineer. Such notification shall be accompanied by a statement of the corrective action to be taken. If the Contractor fails to promptly comply with such instruction, the Engineer may suspend any or all Work on the project until satisfactory, corrective action is taken. If the Contractor fails to take such prompt action, the Engineer shall order such Work, as deemed necessary to ensure public safety and/or to enhance the public convenience, to be accomplished by an Owner-selected workforce. The cost of this Work shall be deducted from monies otherwise due the Contractor.

Article 4.14 Maintenance and Drainage

The Contractor shall maintain all detour routes, haul routes, streets under construction, and all ditches, water courses, existing drainage patterns, siltation control, gutters, sidewalks, walkways and bike trails affected by the Work until the Final Acceptance Date. This includes, but is not limited to, shaping, grading, and dust control. The Contractor shall maintain existing drainage patterns disturbed or utilized by construction, including re-establishment of drainage ditches, swales and gutter flowlines to their preconstruction condition, grade, and elevation.

When cleaning paved streets, curb and gutters, alleys, and sidewalks, the Contractor shall not flush the streets using only water, but shall use such methods as established by the Director of Health and Human Services for sweeping operations. The Contractor shall prevent any spillage from entering any storm drains.

All streets, drainage ditches, swales, water courses, gutters, sidewalks, walkways and bike trails, used by the Contractor or interrupted by his Work, shall be restored to their pre-existing condition. The Contractor shall construct and maintain any drainage and siltation control necessary to accommodate water caused by his pumping or dewatering operations, and shall contain the water to prevent inconvenience to pedestrian and vehicular traffic.

Contractor shall repair or replace catch basins, storm drain manholes, or storm drains damaged during construction as an incidental item of construction at no additional cost to the Owner.

All costs associated with maintenance of drainage patterns and repair or replacement of drainage ditches, swales, catch basins, storm drains, gutter flowlines, and any other drainage appurtenances shall be incidental to the Contract or to the item under construction, and no separate payment shall be made.

For dust and mud control, the Contractor shall maintain all excavations, embankments, stockpiles, access roads, waste areas, borrow areas, and all other Work areas free from excess dust and mud to avoid causing a hazard or nuisance to others.

The Contractor shall provide water or other dust palliatives and appropriate distribution equipment as required for dust control on their haul roads and Work areas. The Contractor
shall assure that all loose material and debris has been removed from haul vehicles prior to their leaving or entering the site to minimize spills of material on road surfaces.

All existing paved areas and roadways adjacent to the project construction site or used as haul roads, shall be kept clean of dirt, mud, and debris resulting from the Contractor’s Work during the construction period.

**Article 4.15 Temporary Erosion Control and Storm Water Pollution Prevention Plans for Construction**

Contractor shall provide all erosion and sediment control measures and best management practices necessary for the prevention of water pollution, erosion, and/or siltation. These measures are for the protection of streams, lakes, ponds, wetlands and tidal waters in compliance with Anchorage Municipal Code, without regard to the size of the Project.

Contractor shall comply with all the requirements set forth in Alaska Administrative Code, 18 AAC 70, and Anchorage Municipal Code Title 21. The Anchorage Municipal Code provides for a fine to be levied for each day of non-compliance.

Unless an erosion and sediment control plan is specifically identified and included in the Drawings or other Contract Documents, Contractor shall provide a Storm Water Pollution Prevention Plan (SWPPP) describing measures and best management practices to be employed during construction. The SWPPP shall meet the requirements of the Municipality’s Storm Water Treatment Plan Guidance Review Manual and shall be one of three types:

<table>
<thead>
<tr>
<th>Type 1 SWPPP</th>
<th>Projects that disturb less than ten thousand (10,000) square feet but that either disturb more than five hundred (500) square feet or are deeper than four feet (4’) or both</th>
</tr>
</thead>
<tbody>
<tr>
<td>Type 2 SWPPP</td>
<td>Projects that disturb between ten thousand (10,000) square feet to one (1) acre</td>
</tr>
<tr>
<td>Type 3 SWPPP</td>
<td>Projects that disturb one or more acre</td>
</tr>
</tbody>
</table>

Type 1, 2 and 3 SWPPPs are described in Division 20, Section 20.02 – Storm Water Pollution Prevention Plan.

Contractor shall implement the erosion and sediment control measures immediately after Contractor mobilization and before commencing any clearing, grubbing, excavation, pavement removal, or other operations or activities which may result in erosion or siltation.

Erosion and sediment control measures and best management practices include such items as silt fences, sedimentation ponds, intercepting embankments and channels, check dams, rock lining, mulching, jute matting, seeding, sodding, and other erosion control devices and practices as required. In order to prevent erosion or siltation, clearing, grubbing, grading, filling and other operations shall be scheduled and performed such that permanent erosion control measures follow immediately. Permanent erosion and sediment control measures are those Work items specified elsewhere in the Contract Documents.
which are intended to provide permanent erosion control such as paving, seeding and other measures and practices as required.

Erosion and sediment control measures shall remain in place and in good functioning or working condition until Work is complete under the Contract and final stabilization of the permanent erosion and sediment control measures is achieved and accepted by the Engineer. The continued maintenance of these erosion and sediment control items and replacement of damaged items shall be the ongoing responsibility of the Contractor.

The Engineer may suspend Work pursuant to Section 10.05, Article 5.24 – Suspension of Work if the Contractor fails to fully carry out the requirements of the erosion and sediment control plan. After suspension of the Work, the Owner may perform or contract the performance of the erosion and sediment control measures and deduct those costs from the Contractor's progress payments.

Payment for a Type 1 SWPPP is incidental to the Contract and no separate payment shall be made. Pay items for Type 2 and Type 3 SWPPPs are found in Division 20, Section 20.02 – Storm Water Pollution Prevention Plan.

**Article 4.16 Temporary Utilities**

The Contractor shall provide and pay all costs for temporary utilities including gas, water, sanitary sewer, telephone, and electricity necessary to perform the Work. The Contractor shall pay for these costs during periods of suspensions of Work. The Owner does not represent that utility service is available to the site.

The Contractor shall provide temporary heat, including fuel and power, as required to protect materials and Work from the elements. The Contractor shall provide and maintain temporary toilets and shall furnish drinking water for all those connected with the Work.

**Article 4.17 Utilities**

Locations of utilities shown on the Drawings are not exact. Above-ground utilities have been field located. Below-ground utilities are shown as depicted on record documents prepared by others and these documents are not necessarily As-Builts; therefore, the depicted locations may not be exact or complete and the Contractor is cautioned to approach his Work accordingly. The Owner shall not be held liable for damages to utilities incurred during construction, including lost time and/or associated costs, due to deficiencies or omissions on the Drawings or these Specifications. At least forty-eight (48) hours prior to commencing Work, the Contractor shall contact all local utility companies to obtain underground utility locates. The Contractor shall exert due care to prevent damage to utilities. Should a utility be damaged, the Contractor shall immediately notify the utility company and shall have the damage repaired at no cost to the Owner. It is expressly understood that the utility has the right to do Work or have its contractor do Work in connection with making repairs to the utility lines damaged by the Contractor. If any utility company determines that a utility has to be temporarily raised, lowered, moved, guyed, shore, braced, or otherwise protected during construction, it shall be done at the expense of the Contractor and to the satisfaction of the utility company.
The Contractor shall be responsible for maintaining all utility service connections whether marked on the Drawings or not. In addition, the Contractor shall repair or replace all utility service connections (at his own cost) that are damaged by his actions.

At a sufficient distance, prior to encountering a known obstacle or tie-in to an existing conduit, pipe or manhole, the Contractor shall expose and verify the exact location of the obstacle, pipe, or manhole so that proper alignment and/or grade may be determined before the pipe sections are laid in the trench and backfilled. The Contractor shall notify the Engineer of the results of this verification, prior to commencement of the Work affected by results of verification, so that any modification to the Drawings or supplementary instructions may be supplied to the Contractor. The Contractor shall allow the Engineer one complete working day to review the verification results and provide any design modifications or supplementary instructions necessary. No additional payment shall be made to the Contractor for this Work.

The cost incurred for removal and alignment of backfilled pipe sections due to improper verification methods shall be borne by the Contractor.

Unless otherwise specified in the Special Provisions, it is the intent of the Contract Documents that utilities shall not be relocated to facilitate construction. If the Engineer determines that an existing utility must be relocated because it is in direct conflict with the facility being constructed, the existing utility shall be relocated by the Utility Company at no charge to the Contractor. In the event a water or sewer service is relocated, it shall be installed in compliance with the minimum separation distances set forth in 18 AAC 80.

The Contractor shall be responsible for coordinating the Work with any Work of a Utility Company and shall not interfere with the initial installation, relocation, reconstruction, or replacement of any utility including the making of necessary service connections by the utility company. If the Work of the Contractor is delayed because of any acts or omissions of the utility company, the Contractor shall not be entitled to additional compensation from the Owner but may be entitled to an extension of time.

The Contractor is required by Municipal ordinance to request locates from the Locate Call Center (278-3121) a minimum of forty-eight (48) hours prior to any excavation.

No buried utility shall be covered until its owner has inspected and accepted it.

Certain utility companies may have facilities within the project limits and some or all of those utility companies may be relocating their facilities and installing crossings within the project limits. The Contractor shall have no right to proceed first with the Work under the Contract in advance of any utility company, except as set forth in the Special Provisions.

A. Trash

Contractor shall coordinate trash pick-up for local residents affected by the construction Work with the solid waste utility.

B. Water and Wastewater
All existing key boxes, cleanouts, manholes, etc. shall be located and exposed by the Contractor and carefully protected during the course of the Work. The Contractor, in conjunction with the Engineer, shall check all utilities prior to the start of the construction and record their condition. All manholes, catch basins, cleanouts, etc. shall be checked for damage resulting from the Contractor's operation prior to final acceptance by the Owner. The Contractor is responsible for restoring all existing utilities to preexisting conditions, and shall coordinate with the affected utility in having any necessary repairs completed.

Contractor shall schedule a walk-through of the project with AWWU Engineering Division and check the existing condition prior to street reconstruction of sanitary sewer manholes, cleanouts and services, and water mainline valves, keyboxes, services, and hydrants. If adjustments are made to AWWU water or sanitary sewer facilities, the Contractor shall schedule and complete a final acceptance walk-through inspection of said facilities with AWWU prior to scheduling this project’s prefinal inspection, provide a Record Drawing, and list any new facility components.

The Contractor shall schedule and be responsible for obtaining all required AWWU permits and shall notify AWWU of any change in status or condition of existing water and/or wastewater infrastructure. In the event water service may be interrupted during the course of construction, the Contractor shall adhere to the requirements of AWWU Policy and Procedures, No. 15-4, Customer Notification for Scheduled or Emergency Water Service Outages.

C. Gas

ENSTAR Natural Gas Company’s natural gas pipelines shall have continuous support during excavation and backfill in accordance with ENSTAR’s Safety Requirements for Excavation Adjacent to Natural Gas Pipeline. Contractor shall exercise extreme caution during excavation and backfill operations to prevent damage to the pipe and pipe coating. Contractor shall place at least twelve inches (12") of classified fill and backfill and compact below exposed gas pipelines, in accordance with Division 20, Section 20.21 - Classified Fill and Backfill.

The Contractor shall call ENSTAR to perform a cathodic protection area sweep after all underground Work (including electrical tie-in work) has been completed and before surface restoration or paving begins. The Contractor shall notify ENSTAR two (2) working days before the sweep is to be performed. The Contractor shall allow time for repairs in the event that underground contacts are detected.

Contractor shall furnish ENSTAR a copy of the construction schedule for Work within the vicinity of gas mains, and shall accommodate ENSTAR’s authorized inspectors per Section 10.05, Article 5.14 – Inspection.

D. Electrical and Telecommunications

The Contractor shall accomplish all Work in close proximity to electrical or telecommunications facilities in conformance with clearance requirements in
accordance with the latest edition of the National Electrical Safety Code (NESC) and the established codes and guidelines of the affected utilities as well as applicable federal and state laws and regulations.

If excavation is required within five feet (5') of utility poles, pad-mounted equipment or concrete ductwork, shoring is required. Prior to shoring, the Contractor shall provide the serving-utility with the proposed method in writing for utility approval. Contractor shall support and protect exposed cables as prescribed by the respective electric and communications utilities. If Contractor damages an electric or communications cable, installed in accordance with an approved municipal Right-of-Way permit, cable shall be repaired by or replaced by the respective utility at Contractor's expense.

Hand digging is required within two feet (2') of a buried electrical or telecommunications cable. Once the utility’s underground cable is exposed, it must be supported every two feet (2'), if it is not lying on the bottom of the trench. Any Work on the electric utility’s facilities or infrastructure must be done by a Journeyman Power Lineman, with a State Certificate of Fitness. If an electrical or telecommunications cable is encountered during excavation, the cable shall be inspected for damage by the respective utility’s operations personnel. If the cable condition appears acceptable, the cable shall be buried by the Contractor and sand bedded per the utility's specifications. The cable shall be tested by the utility to ensure its integrity. Should the cable be damaged due to the excavation, the utility will install a new cable at the Contractor’s expense.

If Contractor requests the electric or telecommunications utility to do any shoring of that utility’s infrastructure, Contractor shall coordinate three (3) working days in advance of shoring with that utility’s operations department.

For overhead electrical power lines, Alaska State Law requires at least ten feet (10’) clearance between energized conductors and construction equipment and structures. Before working under or near electrical power lines, Contractor shall contact that utility’s operations division so they will be aware of the situation.

Any landscaping in areas above buried electrical or telecommunications cables should be limited to bedding plants, small shrubs, or grass. No concrete walls or structures can be built over underground cables.

Contractor shall coordinate any changes in grade and/or earthwork fill/cut areas with electric and telecommunications utilities so adequate infrastructure depth of bury may be maintained.

Any relocation or repair of electrical or telecommunications facilities must be done by the respective utility’s personnel. All relocation Work performed by the electric and/or telecommunications utility for the Contractor shall be at the Contractor’s expense.
All necessary arrangements to be made with the Utility for compliance with AS 18.60.680 shall be arranged by the Contractor in advance of the project start date. All costs of compliance with these requirements, including charges by the Utilities, shall be borne by the Contractor.

E. Traffic Signal Control and Communications

The Contractor shall follow the requirements set forth in Division 80 – Standard Construction Specifications for Traffic Signals and Illumination.

Prior to start of Work that is in close proximity to the following utilities, Contractor shall coordinate with each utility as required by the respective utility and provide a minimum of forty-eight (48) hours notice.

- Alaska Communication Systems (ACS)
- Anchorage Water & Wastewater Utility (AWWU)
- AT&T Alascom
- Chugach Electric Association (CEA)
- ENSTAR Natural Gas
- GCI Cable
- Municipal Light & Power (ML&P)
- Municipal Street and Storm Drain Maintenance
- Municipal Traffic Signals Section
- Solid Waste Services (SWS)

Contractor shall provide the Engineer a minimum of forty-eight (48) hours notice of all utility walk-through inspections and area sweeps. Contractor shall conduct the utility walk-through inspection in the presence of the Engineer, unless the Engineer directs otherwise. Contractor shall furnish a copy of all utility inspection reports to the Engineer prior to commencing construction activities, and provide copies of post-construction inspection reports prior to applying for final payment.

The requirements of this Section shall be considered incidental to the Contract and no separate payment shall be made. Further, the Contractor shall hold harmless, defend, and indemnify the Owner from and against any and all claims, damages, losses, and expenses, including attorney fees, by the Contractor or third-parties arising directly or indirectly out of any conflict between the Work under the Contract and any claim, interference, or delay for whatever reasons.

**Article 4.18 Utility Connections**

Whenever the Contract Documents require permanent connections to be made to utility lines, the Contractor shall, unless otherwise specified in the Special Provisions, be responsible for making the connection to the utility line, or have the Utility Company make
the connection, at the point(s) indicated on the Drawings. The Contractor shall be responsible for making all necessary applications to the Utility Company, for paying all fees and for performing any Work associated with making the connections which is not performed by the Utility Company. The Contractor is not responsible for bringing utility lines to the point of connection. The Contractor shall pay all costs for utility service prior to the Substantial Completion Date.

Article 4.19 Record Documents

Contractor shall maintain Record Documents on the job site consisting of a complete set of drawings, survey line and grade books, and all Contract Documents. Contractor shall record (on Record Documents) and keep current on a daily basis all changes in location (both vertical and horizontal), material, equipment, and all changes in the Work. Contractor shall record all horizontal and vertical locations of all utilities encountered, in conformance with the requirements of Division 65, Section 65.02, Article 2.14 – As-built Surveys and Record Drawings. Contractor shall identify design dimensions, elevations, and grades with no change as being accurate by noting “ASB” adjacent to the design value. At all times Contractor shall make the Record Documents available to the Engineer.

The Contractor shall provide horizontal and vertical locations of all water and sanitary sewer service connections at the property line or lease lot line, including swing ties and offsets to property or lease lot corners. The Record Documents shall include two or more swing ties from prominent, permanent features to show the location of each installed water service connection. Contractor shall provide swing ties as close to perpendicular to each other as possible, and when property or lease lot corners are in, use them as swing tie references.

Contractor shall ensure all additions and corrections are neat, clean, and legible. If additional plan sheets are required, Contractor shall prepare them on reproducible mylar or like material and size as the original Drawings. Contractor shall replace Drawings damaged or lost by Contractor at his expense and to the satisfaction of the Engineer.

The Engineer shall review all Record Documents for completeness and conformance to the standards stated above. Contractor shall make all corrections, changes, additions, and deletions required to conform to the standards.

The Engineer may periodically review the status of the Record Documents during the course of the Work. Failure of Contractor to keep the Record Documents current and in the required condition shall be considered cause for additional withholding from the progress payments as provided in Section 10.07, Article 7.5 - Progress Payments.

Contractor shall deliver approved final Record Documents, bearing certification by Contractor that the Record Documents are a complete and accurate representation of the project as constructed, to the Engineer within thirty (30) days after Substantial Completion or prior to Final Acceptance of the project, whichever is earlier.
Certification for Record Drawings shall be affixed to the final reproducible Drawings and shall include the following unqualified statement, which the Contractor must sign and date as a condition to Final Acceptance of the project.

“This shall serve to certify that these Record Drawings are a true and accurate representation of the project as constructed.”

A similarly prepared and affixed signed statement shall be included on other required Record Documents.

The development, preparation and presentation of all Record Documents is incidental to other Work and no separate payment shall be made.

Article 4.20 Operating and Maintenance Manuals

The Contractor shall provide to the Engineer prior to the pre-final inspection four (4) sets of Operating and Maintenance Manuals for all items of material and equipment as required by the Technical Specifications. In addition to the requirements in the Technical Specifications and Special Provisions, the Manuals shall contain an Index, by Specification Section; a key plan which graphically locates items of equipment; a list of manufacturers, suppliers and distributors with addresses and telephone numbers; and a list of local representatives with addresses and telephone numbers.

Article 4.21 Ownership of Contract Documents

Owner-furnished Drawings, Specifications, and copies thereof are the Owner’s property. Contractor shall not use them on other projects and/or Work and shall return them to the Owner on request at the completion of the Work. Reuse of these materials without specific written authorization or adaptation by the Owner is prohibited and shall be at the risk of the user and without liability or legal expense to the Owner or their consultants. The Contractor shall hold harmless the Owner, Engineer, and Design Engineer and their consultants from any liability arising out of reuse of Drawings and/or Specifications supplied to the Contractor under the Contract.
SECTION 10.05 CONTROL OF WORK

Article 5.1 Authority of the Engineer

The Engineer shall be the Owner's representative and shall observe the Work in progress on behalf of the Owner and shall be identified at the time the Notice to Proceed is issued. The Engineer shall not be responsible for construction means, methods, techniques, sequences, or procedures, or for safety precautions and programs in connection with the Work. Visits and observations made by the Engineer shall not relieve the Contractor of his obligation to conduct comprehensive inspections of the Work and to furnish materials and perform acceptable Work, and to provide adequate safety precautions, in conformance with the intent of the Contract. The Work shall not be considered completed until a Certificate of Completion is issued by the Engineer. The Contractor shall at all times carry out and fulfill the written instructions and written directions of the Engineer regarding the Contract Documents.

The Engineer shall have the authority to order changes in the Work requiring an adjustment in the Contract amount and/or time. The Contractor shall perform such changes in the Work in accordance with supplemental Drawings and instructions as the Engineer may issue. Any single change in the Work, or cumulative changes in the Work, which will cause the total value of the Contract to exceed the limits stated in AMC 7.15.080 requires Assembly approval.

The Engineer shall in all cases make determinations on any and all questions which may arise concerning the quality, quantity, and acceptability of materials furnished, the Work performed, the rate of progress of the Work, the interpretation of Contract Documents, and the merit of all Contractor claims for additional compensation or time extension submitted under Article 5.21 – Claims for Additional Compensation.

If the Contractor determines that instructions, clarifications, or directions issued by the Engineer constitute a change in the requirements of the Contract Documents, the Contractor may make a claim as provided under Article 5.21 - Claims for Additional Compensation.

Article 5.2 Prosecution of the Work

The Work shall not commence until a written Notice-to-Proceed has been received by the Contractor. The Work shall commence within ten (10) days after the effective date specified in the Notice to Proceed and shall be prosecuted vigorously and continuously. No mobilization to the site or field construction activity shall occur until after the required Construction Progress Schedule has been initially submitted in the form and detail required; however, the counting of Contract time shall continue. In this instance, no claim for delay or compensation of any kind shall be considered meritorious and shall not be paid or otherwise recognized. No Requests for Payment shall be accepted until after the schedule has been submitted in the form and detail required.
Article 5.3 Construction Progress Schedule and Schedule of Values

Within ten (10) days after the effective date of the Notice to Proceed and prior to commencement of the Work, the Contractor shall submit to the Engineer a Construction Progress Schedule in the form of a time-scaled bar chart, the elements of which shall be the Divisions and Sections of the Project Manual, weather and ground condition restraints, and Work suspensions and other significant influences on the Contract amount and/or the time for completion of the Work. The Contractor shall include other significant features of the Work such as the submittal schedule, permit acquisition plan, material procurement milestones, plant and equipment procurement dates, and shipping schedules. Any Work plan requirements including project phasing shall also be included and identified in the Construction Progress Schedule. The bar chart shall include a graph representing the monthly percent of Work to be completed. The bar chart shall be revised and resubmitted as required by the Engineer, when conditions cause changes to the construction schedule, or on a monthly basis, whichever is sooner.

When required by the Engineer, the Contractor shall also deliver, at the same time the Construction Progress Schedule is delivered and in a form satisfactory to the Engineer, a Schedule of Values for Contract Payments for those lump sum items designated by the Engineer. When payment for the Work is based primarily on unit prices identified in the Bid Schedule, no Schedule of Values shall be required. However, when the Bid Schedule includes a mixture of unit prices and lump sum prices, and the lump sum prices represent a significant portion of the total Contract amount, then the Engineer reserves the right to require a Schedule of Values for specified lump sum items. The Contractor shall submit monthly partial Payment Estimates based on the Schedule of Values if they have been required. All Schedules of Values and Payment Estimates shall, as a minimum, be organized to be consistent with the Divisions and Sections of the Project Manual. The Engineer may require submission of revised construction schedules demonstrating the manner in which the Contractor will achieve the necessary rate of progress, all without additional cost to the Owner. Partial Payment Estimates may be appropriately reduced if the Engineer determines that the Contractor has failed to supply the Owner with the requested or necessary information.

In addition to the time-scaled bar chart described above, and when specified in the Special Provisions, the Contractor shall develop and submit to the Engineer for approval a time-scaled Critical Path Method (CPM) schedule. Both a hard copy and electronic copy of the CPM schedule shall be submitted. The Contractor shall revise and resubmit the CPM schedule to reflect any alteration in the sequence of scheduled activities or of the critical path with each partial payment request and at such other times as the Engineer may require, and at any time the Contractor determines that the critical path is altered by changes or other circumstances. The Contractor shall submit the initial CPM schedule to the Engineer no later than twenty-one (21) days from the effective date of the Notice to Proceed and at least monthly thereafter.

The Contractor shall organize the scheduled activities to be consistent with those Specifications, Divisions and Sections, required for the Work. Each Division and Section of the Specifications and each item in the Schedule of Values shall be represented by one or
more scheduled activities. In addition, one or more scheduled activities shall be used to represent the submittal schedule, permit acquisition, materials procurement, plant and equipment procurement, shipping, and all other significant elements of the Work. The Contractor shall include activities that address weather and ground condition restraints, critical dates, holidays, periods of Work suspension, and all other restraints (i.e., all events that are critical or will become critical to the schedule).

All schedules, whether they are bar chart schedules or CPM schedules, shall include enough detail to adequately describe all important activities necessary to complete the Work. Unless otherwise agreed to by the Engineer, no single activity in any schedule shall be allowed to represent more than fifty thousand dollars ($50,000) of the total scope of the Work. The initial schedule and monthly schedule updates shall be considered payable activities and appropriate payment amounts shall be included in the Schedule of Values or the Bid Schedule. Failure to provide adequate schedules shall result in non-payment in accordance with the amounts established in the Schedule of Values or the Bid Schedule.

Article 5.4 Non-Working Hours, Holidays, Saturdays, and Sundays

The Contractor shall give the Engineer forty-eight (48) hours advance notice of his intention to work overtime, Saturdays, nights, Sundays or holidays, or anytime outside the usual working hours. In no case shall the Contractor do any such Work without first notifying the Engineer to allow arrangements for proper inspection. Contractor shall not be reimbursed for Work performed in violation of this Article unless the Work is in direct response to an Emergency and approved after-the-fact by the Engineer.

The Contractor shall reimburse the Owner labor, overtime, and all other costs for inspection Work performed on Sundays, recognized holidays, and for any Work in excess of a standard work day (excluding meal times) except when the Work is required by a permit issued by an agency after the Contract has been executed, or unless the Work period is designated by the Owner.

Article 5.5 Shop Drawings

The Contractor shall submit to the Engineer for review six (6) copies of those Shop Drawings required by the Contract Documents within a reasonable time and in such sequence as to cause no delay in the Work or in the work of the Owner or any separate contractor. A properly completed Submittal Transmittal form shall accompany all submittals. If a substitution is being requested, a Substitution Request form shall be submitted with the appropriate backup documentation.

The Shop Drawings submitted by the Contractor shall bear his specific written and signed certification that he has verified: (1) that the Work shown is in conformance with the Contract Documents; (2) that he has determined and verified quantities, dimensions, field measurements, and related field construction criteria; and (3) that he has checked and coordinated the submittal with the requirements of the Work. The Contractor shall indicate on the Shop Drawing submittal any deviation from the requirements of the Contract Documents.
All Shop Drawings shall be clear and legible. Any Drawings submitted which appear to be carelessly prepared, erroneous, or unchecked shall be returned to the Contractor for further action, and resubmittal.

Within a reasonable time, the Engineer shall review and approve or take other appropriate action on the submittals, but only for conformance with the design concept of the Work and with the information given in the Contract Documents. The Engineer's approval of a specific item shall not indicate approval of an assembly of which the item is a component.

The Engineer shall state the reasons for rejection and/or resubmittal requirements if applicable.

Revisions on re-submittals other than those requested by the Engineer on previous submittals shall be specifically noted by the Contractor.

Upon approval of the Shop Drawings by the Engineer, two (2) copies shall be returned to the Contractor. If the Contractor requires more than two (2) copies, the Contractor shall submit such additional copies.

The Engineer's approval of Shop Drawings does not relieve the Contractor of responsibility for any deviation from the Contract Documents unless the Contractor has submitted and received written approval of the Deviation Request. Errors and omissions that may occur in the Shop Drawings are the responsibility of the Contractor. The Contractor is not relieved of this responsibility by the Engineer's approval of the Shop Drawings.

When Shop Drawings are required on a portion of the Work, the Contractor shall not commence that portion of Work or any item relying on said portion of Work until such Shop Drawings have been given written approval by the Engineer.

The Contractor shall keep one copy of all Contract Documents, including modifications, and one copy of approved Shop Drawings in good order and available to the Engineer or his representative at the construction site.

**Article 5.6 Product Data**

The Contractor shall submit for approval six (6) copies of complete Product Data for those items for which submittals are required by the Contract Documents including, but not limited to, specific performance data, material description, rating, capacity, working pressure, material gage or thickness, brand name, catalog number, and operating and maintenance data. Submittals shall be submitted within a reasonable time and in such sequence as to not cause a delay in the Work, in the Work of the Owner, or any separate Contractor. A properly completed Submittal Transmittal form shall accompany all submittals. If a substitution is being requested, a Substitution Request form shall be submitted with the appropriate backup documentation.

Within a reasonable time, the Engineer shall review and approve or take other action on the submittals. Approval by the Engineer is required before any of the equipment is ordered.
Product Data for equipment approved by the Engineer shall not in any case supersede the Contract Documents. The approval by the Engineer shall not relieve the Contractor from responsibility to correct deviations from Drawings or Specifications, unless he has notified the Engineer in writing of such deviations at the time of submission and secured the Engineer’s written approval. The Contractor shall not be relieved from responsibility to correct errors of any sort in the items submitted. The Contractor shall check and approve the item described by the Product Data with the Contract Documents for deviations and errors prior to submittal to the Engineer for approval. The Contractor shall ensure that items to be furnished fit the space available as shown in the Contract Documents.

Upon approval of the equipment by the Engineer, the Contractor shall furnish four (4) copies of Product Data of all equipment or components together with operating and maintenance instructions.

**Article 5.7 Materials**

All materials and equipment furnished under the Contract shall be new, unless otherwise specified, and shall be of good quality, free from defects, and shall conform to the requirements of the Contract Documents. Substitute materials shall not be used unless approved through the Substitution Request process by the Engineer in writing prior to installation. When required by the Engineer, the Contractor shall furnish satisfactory evidence as to the kind and quality of materials and equipment.

In order to establish standards of quality, the Specifications may refer to certain products by name and catalog number. This procedure is not to be construed as eliminating from competition other products of equal or better quality by other manufacturers. The words "approved equal" shall be considered following all such listings regardless of whether or not they so appear, UNLESS the listing(s) specifically state “No Substitutions.” In such event, no substitutions shall be accepted.

The Contractor shall furnish the Engineer the complete list of proposed substitutions within ten (10) calendar days of the effective date of the Notice-to-Proceed (or such time as may be approved in writing by the Engineer), together with complete engineering and catalog data in sufficient time prior to their use to give the Engineer adequate time for review. A properly completed Submittal Transmittal form shall accompany all submittals. If a substitution is being requested, a Substitution Request form shall be submitted with the appropriate backup documentation. Failure on the part of the Contractor to obtain the necessary approval prior to ordering or using such alternate material or equipment shall not relieve the Contractor of furnishing acceptable material or equipment as required by the Contract Documents.

The Contractor shall abide by the Engineer's decision when proposed substitute materials or items of equipment are judged to be unacceptable and shall furnish the specified material or item of equipment in such case. The Engineer shall approve or disapprove proposed substitutions in writing within a reasonable time. No substitute materials shall be used unless approved in writing.
Materials shall be stored in such a manner as to insure the preservation of their quality and fitness for use. When considered necessary to protect materials against cold, dampness, or to keep them clean and free from dust, dirt, or other detrimental matter, suitable sheds, platforms, and covers shall be provided. The Contractor shall provide easy access to stored materials for inspection whenever requested by the Engineer.

Manufactured articles, material, and equipment shall be applied, installed, connected, erected, used, cleaned, and conditioned as directed by the Manufacturer. In the event of conflict between the manufacturer's directions and the Contract Documents, the higher standard requirements shall govern.

**Article 5.8 Testing of Materials**

All tests shall be made in accordance with methods as described and designated in the Contract Documents. When tests of materials are required on site, such tests shall be provided by and at the expense of the Owner unless otherwise specified in the Special Provisions. All factory testing, mill testing and other off site testing shall be as specified or required to conform with codes and industry standards and provided by and at the expense of the Contractor.

The Contractor shall provide such labor and facilities as may be required for collecting and forwarding Samples necessary for testing and shall hold the materials represented by the Samples until tests have been made and the materials found equal to the requirements of the Specifications. The Contractor in all cases shall furnish the required Samples without charge.

In the absence of any definite Specification, it shall be understood that such materials and tests shall meet the Specifications and requirements of the American Society for Testing and Materials (ASTM) or the American Association of State Highway Transportation Officials (AASHTO).

Wherever a particular specification of a Society for Testing and Materials is referred to by number, it shall be understood that such reference shall include all amendments and additions thereto adopted by such organizations prior to the award of the Contract.

Repetitive testing of materials in constant use may be required periodically by the Engineer. Required retesting shall be accomplished at the expense of the Contractor when materials have previously been tested and have not met the requirements of the Contract Documents.

**Article 5.9 Contractor's Authorized Representatives and Employees**

The Contractor shall within five (5) days after the Notice to Proceed but no later than the Preconstruction Conference, name the Superintendent, the Safety Supervisor required by Section 10.06, Article 6.8 – Safety, and file with the Engineer a list of all persons who are authorized to sign documents on behalf of the Contractor to fully bind the firm. The name and twenty-four (24) hour phone numbers of two persons that may legally act on behalf of the Contractor in case of emergency at any time shall also be provided.
The Superintendent is the Contractor’s representative at the site and has authority to act on Contractor’s behalf. All communications given to the Superintendent are as binding as if given to Contractor. A qualified Superintendent is one who is completely familiar with the requirements of the Contract Documents, has experience and ability to direct all Work at the site, is able to speak and communicate effectively using English, and is present at the job site, or readily available at all times while Work is in progress.

The Contractor shall employ only qualified journeymen, mechanics, tradesmen, and installers who are thoroughly skilled and experienced in their respective trades or specialties. When apprentices and helpers are employed, they shall be under the supervision of qualified journeymen mechanics and tradesmen at all times.

The Contractor shall at all times enforce strict discipline and good order among his employees and Subcontractors and shall not employ on the Work any unfit person or anyone not skilled in the task assigned to him. The Engineer may require, at no additional cost to the Owner, the Contractor to remove from the Work any employee or Subcontractor that the Engineer deems incompetent, careless, or otherwise objectionable.

**Article 5.10 Subcontracting**

If any part of the Work to be done under the Contract is subcontracted, the subcontracting shall be done in accordance with the following provisions:

1. All Subcontractors proposed for the Work shall be acceptable to the Owner.

2. Within ten (10) days after the effective date of the Notice-to-Proceed, and prior to commencement of the Work, the Contractor shall provide the Engineer in writing a list of Subcontractors together with a summary of the extent and character of the Work to be done by each Subcontractor. If for sufficient reason, at any time during the progress of the Work, the Engineer determines that any Subcontractor is incompetent or undesirable, he will notify the Contractor accordingly and immediate steps shall be taken by the Contractor for cancellation of such subcontract and new Subcontractor, acceptable to the Owner, shall be provided at no additional cost to the Owner. Subletting by Subcontractors shall be subject to the above.

3. The Contractor shall be fully responsible to the Owner for the acts, errors, and/or omissions of his Subcontractors and of persons either directly or indirectly employed by them. Nothing contained in the Contract Documents shall create any contractual relation between any Subcontractor and the Municipality.

4. The subcontracting of any of the Work to be done shall in no way relieve the Contractor of any part of his obligations under the Contract.
Article 5.11 Right of the Municipality to Do Work

Contractor expressly understands that the Municipality has the right to do Work and may award other Contracts in connection with the Work under the Contract or nearby projects. Contractor shall conduct his operations so as to interfere as little as possible with other contractors or subcontractors on or near the Work.

Article 5.12 Safeguarding of Excavations

The Contractor shall provide such safeguards and protections around and in the vicinity of all excavations as may be necessary to prevent damage to property or injury to persons.

The Contractor shall properly barricade all trench excavations with appropriate signs and warning lights placed to prevent inadvertent entry by vehicular or pedestrian traffic.

The Contractor shall backfill all trench excavations to the top of the trench at the end of each working day, except, at Contractor's option, a "bell-hole" may be left open if properly barricaded and adequate signage and warning lights are placed to prevent inadvertent entry by vehicular or pedestrian traffic.

If ground water or surface water results in standing water in the remaining excavation, the Contractor shall provide continuous pumping to maintain the excavation in a dewatered condition.

The Contractor shall maintain all roadways in a drivable condition for normal vehicular and transport operations, including emergency vehicles, at the end of each day's operation.

These requirements shall in no way relieve the Contractor of the obligation to restore private property to its preconstruction condition.

Article 5.13 Duties of Inspectors

Inspectors shall be authorized to inspect all Work and Materials. Such inspection may extend to all or any part of the Work and to the preparation, fabrication, or manufacture of the materials to be used. Inspectors are not authorized to alter or waive the provisions of the Contract. Inspectors are not authorized to issue instructions contrary to the Contract Documents or to act as foreman for the Contractor.

Inspectors shall immediately inform the Contractor of any deficiency known to exist in the Work and any laboratory test results.

As the Engineer's authorized representative, the inspector may reject damaged or other unsuitable materials and direct their replacement in accordance with the Contract Documents. With prior approval of the Engineer, the inspector may issue temporary Work Suspension Orders due to (1) weather conditions; (2) the Contractor's refusal to carry out the orders or directives of the Engineer or his Authorized Representative; or (3) the Contractor's refusal to perform in accordance with the Contract Documents.
The Contractor's responsibility for Work performed under the Contract shall in no way be relieved because of the presence or absence of an inspector. Work shall not be considered acceptable because of the presence of an inspector.

**Article 5.14 Inspection**

The Engineer or his representative shall be allowed access to all parts of the Work at all times and shall be furnished with every reasonable facility for ascertaining whether or not the Work is being performed in accordance with the requirements and intent of the Contract Documents. Upon the request of the Engineer, the Contractor shall, at any time before Final Acceptance of the Work, remove or uncover such portions of the finished Work as may be directed. After examination, the Contractor shall restore said portions of the Work to the standard required by the Contract Documents. Should the Work thus exposed or examined prove acceptable, the uncovering or removing, the replacing of the coverage or the restoration of the parts removed shall be paid for as extra Work.

Should the Work so exposed or examined prove unacceptable, the uncovering or removing, replacing of the covering and the restoration of the parts removed, shall be at the Contractor's expense.

**Article 5.15 Work Limits, Easements, and Rights-of-Way**

The Owner shall provide work limits, rights-of-way, and easements for the Work. Information regarding the width and status of the easements is shown on the Drawings. The Contractor shall comply with all Special Conditions, provisions, stipulations, and restrictions thereof. The Contractor shall confine his operations to the designated Work areas, rights-of-way and easements and shall observe all restrictions. Prior to the start of construction under the Contract, the Contractor shall ensure that all permits necessary for the construction of the project, including right of entry for driveway reconstruction, have been obtained and shall ensure that they are available on the job site at all times.

The Contractor shall be responsible for any trespass upon adjacent property or injury thereto resulting from or in connection with his operations. The Contractor shall be liable for any claims that may be made on account of trespass and shall provide a written statement from the property owner of full restoration or satisfactory resolution prior to Final Acceptance of the Work. The Contractor shall not have the right to remove materials from a right-of-way, easement, or Work area unless otherwise provided in the Contract Documents.

Should the Contractor desire to go outside designated Work areas, rights-of-way or easements, he shall provide the Engineer with written permission from the property owner before entering such property. The written permission shall specifically provide that the property owner shall not hold the Municipality, its employees, agents or consultants liable for use of or damage to this property.
Article 5.16 Responsibility for Damages

The Contractor shall be responsible for all damages to property, injury to persons, and loss, expense, inconvenience, and delay that may be caused by or that may result from any act, omission, or neglect of the Contractor, his Subcontractors, or his employees in the performance of the Work.

It is specifically understood between the parties executing the Contract that the Contract Documents do not make anyone a third party beneficiary, nor does the Contract authorize anyone not a party to maintain a lawsuit for personal injuries or property damage.

Article 5.17 Repair of Damages Caused by Contractor

All damage and injury to property that is caused by or that results from the carrying out of the Work, or from any act, omission, or neglect of the Contractor, his Subcontractors, or his employees, shall promptly be remedied by the Contractor either by the repairing, rebuilding, or replacing of the property damaged or in some other manner satisfactory to the owner of such property. In case of failure on the part of the Contractor to promptly and satisfactorily remedy such damage or injury, the Municipality may proceed to repair, rebuild, or replace such property as required and the cost thereof shall be deducted from any monies due or which may become due the Contractor.

In applying the above provisions, the repairing, rebuilding, or replacing of damaged property shall be understood to include the providing of any temporary facilities that may be needed to maintain normal service until the required repairing, rebuilding, or replacing is accomplished.

This provision also applies to all areas used by the Contractor for staging of the construction and shall include restoring those properties to their original condition to the satisfaction of the Engineer.

Article 5.18 Changed Conditions

The Contractor shall notify the Engineer in writing, no later than two (2) working days and before such conditions are disturbed, of: (1) subsurface conditions at the site differing materially from those indicated in the Contract, or (2) unknown physical conditions at the site, of an unusual nature, differing materially from those ordinarily encountered and generally recognized as inherent in Work of the character provided for in the Contract. The Engineer shall promptly investigate the allegations. If the Engineer finds that such conditions do materially alter the Contract requirements so as to cause an increase or decrease in the Contractor’s cost of, or the time required for, performance of the Contract, the Engineer may request that the Contractor submit the documentation required under Article 5.21 - Claims for Additional Compensation.

No claim of the Contractor under this clause shall be allowed unless the Contractor has given written notice as required. However, the time prescribed in this Article may be extended for good cause by the Engineer.
Article 5.19 Unauthorized and Defective Work

Any unauthorized or defective Work found to exist during construction shall be immediately remedied by the Contractor. If the Contractor fails to correct unauthorized or defective Work within three (3) days of receipt of written notice, the Owner may correct such deficiencies and deduct the cost thereof from any payment due the Contractor without prejudice to any other remedy including the use of Article 5.28 - Termination of Contract by Owner.

Article 5.20 Changes in the Work

The Engineer shall have the authority to order changes in the Work which may or may not require an adjustment in the Contract amount and/or time. Such changes in the Work shall be performed in accordance with any supplemental Drawings and instructions as the Engineer may issue. Any single change in the Work, or cumulative changes in the Work, which will cause the total value of the Contract to exceed the limits stated in AMC 7.15.080 requires Assembly approval.

The Owner shall pay for additions to the Work or take credit for reductions to the Work using one of the four methods described below and as specified in Section 10.07, Article 7.4 – Change Order Compensation Adjustments:

1. Negotiated unit or lump sum prices;
2. Contract unit or lump sum prices (if they have been included as a part of the Contract);
3. Time and Material prices (when the Engineer determines that Contract prices or negotiated prices do not apply); or
4. No cost changes (when the Engineer determines that a change is necessary which does not affect the price or time for the Work).

Prior to the Engineer authorizing payment for changed Work, the Contractor shall furnish a Change Order Proposal that is itemized as required by the Engineer for both additions and deletions to the Work.

All Contractor-initiated requests for deviation from the requirements of the Contract Documents shall be accomplished by the submittal of a completed Deviation Request Form and a Contractor’s Change Order Proposal. Unless otherwise specified in the Engineer’s approval of a Deviation Request, and formalized by an executed Change Order, any and all increased costs or delays resulting directly or indirectly from the deviation shall be borne solely by the Contractor.

The Contractor’s Change Order Proposal shall be in sufficient detail to permit an analysis of all materials, labor, equipment, subcontracts, insurance, bonds, overhead costs and profit and shall cover all Work involved to accomplish the modification whether deleted,
added or changed. Any amount claimed for subcontracts shall be supported by a similar price breakdown. The Contractor agrees that it will incorporate the provisions of this Article into all agreements with lower tier subcontractors.

Any compensation paid in conjunction with the terms of a Change Order shall constitute total compensation due the Contractor for the Work or alteration defined in the Change Order. By signing the Change Order, the Contractor acknowledges that the stipulated compensation includes payment and Contract time adjustments for the Work or alteration plus all payment for the interruption of schedules, extended overhead, delay or any other impact claim or consequential effects created by the Work or alteration defined in the Change Order, and by such signing specifically waives any reservation or claim for additional compensation in respect to the subject of the Change Order.

If the Contractor’s Change Order Proposal includes a request for a time extension, a justification thereof shall also be furnished. The Change Order Proposal together with the price breakdown and time extension justification shall be furnished by such date as may be specified by the Engineer.

The Engineer may, subject to the written approval of the Purchasing Officer, execute a Unilateral Change Order within the limitations set forth in AMC 7.15.080 if:

1. the Engineer determines that adjustment in compensation or time is due either the Contractor or the Owner;
2. negotiations fail to reach mutual agreement on the magnitude of adjustment; and
3. the revised Work is such that, in the opinion of the Engineer, it could not be reasonably conducted at a time or in a manner that would permit determining its value on a Time and Material basis under Section 10.07, Article 7.4, SubArticle B – Time and Material Changes.

A Unilateral Change Order shall have the same force and effect as a mutually executed Change Order, except that the Contractor shall not have waived his rights to pursue additional compensation as a separate claim. A Unilateral Change Order shall incorporate such Contract adjustment as the Engineer may determine, which may not necessarily be the last offer made during negotiations. The incorporation of the Work by Unilateral Change Order shall in no way diminish the Contractor’s responsibility to complete the revised Work in an efficient, timely manner as otherwise required by the Contract Documents.

Article 5.21 Claims for Additional Compensation

Except where restricted in the Contract Documents, the Contractor may make a claim for additional compensation when he believes that he has incurred additional costs due to the acts, errors, or omissions of the Owner or by reason of changed conditions. If the Contractor becomes aware of any act or occurrence which may form the basis of a claim, the Contractor shall make every effort to mitigate the extent of any amounts claimed for additional compensation and shall immediately notify the Engineer in writing of the potential
for the claim, providing sufficient information to outline the basis of the claim. If the matter is not resolved within seven (7) days, the Contractor shall, within the next fourteen (14) days, submit written notice of the facts which may form the basis of the claim.

Thereafter, the Contractor shall submit the claim in writing to the Engineer within sixty (60) days of the submission of the written notice of the facts unless the Engineer agrees in writing to an extension of time for good cause shown. The Engineer may grant up to a sixty (60) day extension only upon the written request of the Contractor in which all reasons for the request are stated. The Contractor agrees that unless these written notices are provided, the Contractor shall have no entitlement to compensation for the acts, errors, or omissions of the Owner, the Engineer or any other Contractor employed by the Owner. The Contractor shall in all cases continue performance of the Contract.

The written claim presented by the Contractor shall be complete and shall specifically include the following:

1. the facts and circumstances surrounding the claim and the Contract provisions under which the claim is made;
2. the Contractor's assertion as to the original requirements of the Contract Documents and the basis for that assertion or position, citing all pertinent Specifications, Details, Drawing notes or other Contract provisions;
3. a clear certification that the Contractor's Bid Costs were in fact based on the stated original interpretation;
4. the Contractor’s assertion as to the revised requirements of the Contract Documents, citing all pertinent Contract provisions, or lack thereof, and other records on which that assertion or position is based;
5. a narrative description of the increase in the Scope-of-Work resulting from the revision in the requirements;
6. the Pay Items and quantities affected by the alleged change;
7. references to previous notices of pending claim; and
8. the specific relief requested, including both time extension and additional cost compensation and the basis on which both were calculated.

In the case of cost compensation, such basis for specific relief shall include (1) the labor classifications, rates and additional time; (2) the equipment descriptions, rates and additional time; (3) material descriptions, unit prices and quantities; and (4) appropriate supporting documentation as to materials, unit prices, labor rates and equipment rates. The permitted rates and allowances shall be as provided under Article 5.20 - Changes in the Work.

Claims presented that do not include the above information or otherwise considered to be incomplete will be returned to the Contractor without review by the Engineer. Within sixty (60) days after receipt of the Contractor’s properly submitted and complete claim, the Engineer shall render a Final Determination as to the merit of the claim and, if any are justified, the amount of additional compensation and time due. Any change in the Contract
sum or allowable time resulting from such claim shall be authorized only by the execution of a proper Change Order.

**Article 5.22 Time for Completion of Work**

The Owner shall indicate in the Special Provisions either a time period for completion of the Work or a completion date. Time is of the essence in the Contract. Therefore, the Work to be performed under the Contract shall be completed in its entirety within the time period specified or before the completion date.

The Contractor shall furnish such manpower, materials, facilities, and equipment and shall work the required hours, including authorized night shifts, overtime operations, and Saturdays, Sundays, and holidays as may be necessary to insure the completion of the Work within the time specified.

Failure of the Contractor to comply with the requirements of this Article may be considered grounds for termination under the provisions of Article 5.28 - Termination of Contractor by Owner.

**Article 5.23 Delays and Extension of Time**

If the Contractor is delayed, beyond his control and without fault or negligence on his part, at any time in the progress of the Work by any act or neglect of the Owner or by changes ordered in the Work, or by labor disputes, fire, unusual delay in transportation, adverse weather conditions not reasonably anticipated, unavailability of materials for which orders were placed timely, or by unavoidable casualties, then the time period for completion or the completion date may be extended by Change Order, for such reasonable time as the Engineer may determine, without invalidating any of the provisions of the Contract and without the consent of the Surety.

Any claim for extension of time shall be made in accordance with the procedures set forth in Article 5.21 - Claims for Additional Compensation. In the case of a continuing delay, only one claim is necessary. The Contractor shall provide an estimate of the probable impact of such delay on the progress of the Work.

No extension of time or changes to bid unit prices shall be granted due to the Contractor’s failure to properly plan for and deliver bid items that require a long lead-time. The timely delivery of schedule-critical items is crucial and the Contractor shall include in his bid unit price the cost (if any) of expedited delivery to assure that construction can be completed within the time of completion specified in the Contract.

**Article 5.24 Suspension of Work**

By executing a Contract, the Contractor agrees that the Owner has the undisputed right to suspend the Work and that this right is a material condition of the Contract. The Contractor shall immediately suspend the Work as directed in a written order from the Engineer or Owner. Failure of the Contractor to immediately suspend the Work as directed shall constitute a material and immediate breach of the Contract by the Contractor. The Owner
may terminate the Contract for default without providing the ten (10) day notice specified in Article 5.28 – Termination of Contract by Owner, should the Contractor fail, refuse or otherwise not immediately suspend the Work as directed.

The Work may be suspended in whole or in part by a written order of the Engineer for the convenience of the Owner. The Contractor shall take every precaution to prevent any damage or unreasonable deterioration of the Work during the time it is suspended. Suspension of the Work by the Engineer for the convenience of the Owner may furnish grounds for a claim by the Contractor for additional compensation and/or a time extension, in which case the Contractor, when making a claim, shall comply with the provisions of Article 5.21 - Claims for Additional Compensation.

Upon the failure of the Contractor to carry out the orders of the Engineer or to perform in accordance with the Contract Documents, the Engineer may suspend the Work for such period as may be necessary. Time lost by reason of such suspension, or replacement of improper Work or material, shall not furnish any grounds to the Contractor for claiming additional compensation and/or an extension of time and shall not release the Contractor from any liability for damages or for failure to complete the Work within the time prescribed.

In the event that a suspension of Work is ordered in writing by the Engineer due to adverse weather or unforeseen conditions, and, in the opinion of the Engineer, the Contractor has prosecuted the Work with due diligence prior to the time of suspension, the Contractor may be due an extension of time, but not additional compensation.

Where the Contract provides for a time period for completion and the Work is suspended for the convenience of the Owner or adverse weather or unforeseen conditions and the Contractor has prosecuted the Work with due diligence, the time period and liquidated damages provision of the Contract shall be suspended until a Notice to Resume Work is issued by the Engineer.

**Article 5.25 Final Trimming of Work**

The Contractor shall be responsible for all repair to the Work as necessary to overcome deterioration or damage that may occur prior to Final Inspection. The Contractor at all times shall keep the premises free from accumulation of waste materials, rubbish and debris. The Contractor shall grade all existing driveways on, and which have been affected by the project within the rights-of-way or easements as directed by the Engineer. At the completion of the Work, all waste materials, rubbish, debris and temporary structures from and about the Project as well as all his tools, construction equipment, machinery and surplus materials shall have been removed from the Project area. The Work shall be in a neatly trimmed and well-finished condition throughout at the time of Final Inspection. This Work shall be considered incidental to the Contract unless there is a specific Contract item for this Work.

Any time during the Work that cleanup, in the opinion of the Engineer, is not keeping pace with the rest of the Work, the Contractor shall, at the direction of the Engineer, suspend all operations on the major items of Work until the premises are cleaned up to the satisfaction
of the Owner. Any additional expense involved shall be the sole responsibility of the Contractor and the Owner shall not be held liable for this additional expense.

All contours, roadway surfaces, drainage courses, street name signs, traffic control signs, mailboxes, newspaper boxes, property corner markers, survey markers, survey monuments, utility markers, existing vegetation, shrubbery, lawns, trees, fences, rockeries, landscaping, sidewalks, driveways, and other improvements, removed to facilitate or damaged by the Contractor's operations shall be fully restored to original condition at their original location by the Contractor unless otherwise required by the Contract Documents or directed by the Engineer. Items damaged by the Contractor during removal, storage, or restoration shall be repaired or replaced in kind by the Contractor. Repairing or replacing damaged items shall be considered incidental to the Contract and no separate payment shall be made.

Article 5.26 Final Inspection

When the Contractor, through his own comprehensive inspection, has concluded that all Work is completed, all code compliance inspections have been performed, and all other Contract requirements have been fulfilled; he shall notify the Engineer in writing of completion and request a pre-final inspection of the Project. The pre-final inspection shall be performed in the presence of a representative of the Owner, the Engineer, and the Contractor. The Contractor shall make available copies of all required code compliance inspection reports at this inspection. All deficiencies indicated by the pre-final inspection shall be listed and promptly furnished to the Contractor for remedial action. When all listed deficiencies have been corrected, the Contractor shall notify the Engineer and a Final Inspection will be performed. When the Final Inspection verifies correction of the listed deficiencies, the Engineer shall issue a Certificate of Completion.

When the Final Inspection reveals uncorrected listed deficiencies, the above outlined procedure shall be repeated and the cost of reinspection shall be deducted from any money due the Contractor. This cost shall include, but is not limited to, salaries, administrative, and transportation costs.

Article 5.27 Liquidated Damages

The Owner may withhold from any progress payment the sum per day identified in the Special Provisions as Liquidated Damages for each and every calendar day that the Substantial Completion Date is delayed beyond the Contract Completion Date. After substantial completion, the Owner may withhold out of any progress payment the sum per day identified in the Special Provisions as Liquidated Damages for each and every calendar day that the Final Acceptance Date is delayed beyond the Contract Completion Date. If no money is due Contractor, the Owner shall have the right to recover said sums from Contractor, the Surety, or both.

The Contractor acknowledges that the daily amount of the Liquidated Damages provision is not a penalty, but rather is a reimbursement for damages that the Owner will sustain by reason of delayed completion. The Contractor further acknowledges that the daily amount
of Liquidated Damages is a reasonable alternative to the complex calculations that would otherwise be necessary to determine such damages.

Permitting the Contractor to continue and finish the Work or any part of it after the time fixed for its completion, or after the date to which the time for completion may have been extended, shall in no way operate as a waiver on the part of the Owner of any of its rights, including liquidated damages, under the Contract.

**Article 5.28 Termination of Contract by Owner**

If the Contractor should be adjudged bankrupt, or if he should make a general assignment for the benefit of his creditors, or if a receiver should be appointed on account of his insolvency, or if he should persistently or repeatedly refuse or fail to supply enough properly skilled workmen or proper materials for the efficient prosecution of the Work, or persistently disregard laws, ordinances, or the instructions of the Engineer, or otherwise substantially violate any provisions of the Contract, the Owner may, without prejudice to any other right or remedy and after giving the Contractor and his Surety ten (10) days concurrent written notice, terminate the Contract and take possession of the premises and of all materials, tools and appliances thereon. Notwithstanding the foregoing, the Owner may immediately terminate the Contract for default without providing a ten (10) day notice if the Contractor fails, refuses or otherwise does not comply with a written order by the Engineer that may involve issues of safety or a Suspension of Work issued under Article 5.24 – Suspension of Work. When the Contractor and Surety are notified of the termination of the Contract, the Owner may demand that the Surety fulfill its obligations under the Performance and Payment Bond. Should the Surety fail to perform its obligations under the Bond after demand of the Owner, the Owner may finish the Work by whatever method that the Owner determines expedient. The Contractor and his Surety shall be responsible for compensating the Owner for all excess costs, including applicable liquidated damages and all reprocurement costs, incurred in accomplishment of the Work.

In the event the Owner terminates the Contract, the Owner does not waive any other right or remedy under the Contract or any other right or remedy available at law or equity. The Contractor may not be allowed to bid on any Owner's contracts for a period of two years following the date of this termination by the Owner.

In the case of termination before completion for any cause whatsoever, the Contractor, if notified to do so by the Owner, shall promptly remove equipment and supplies from the premises of the Owner. Failure to do so shall authorize the Owner to remove such equipment and supplies from the premises at the expense of the Contractor.

**Article 5.29 Termination of Work for Owners Convenience**

At any time during the term of the Contract, the Owner may terminate the Work, in whole or in part, for any reason that the Engineer shall determine to be in the best interest of the Owner. Any such termination shall be accomplished by delivery of a Notice of Termination to the Contractor, specifying (1) that the termination is for the convenience of the Owner; (2) the extent to which performance of the Work under the Contract is terminated; and (3) the date upon which such termination becomes effective.
Except as otherwise directed by the Owner, after receipt of a Notice of Termination, the Contractor shall:

1. Stop Work under the Contract on the date and to the extent specified in the Notice of Termination;

2. Place no further orders or subcontracts for materials, services, or facilities except as may be necessary for completion of such portion of the Work under the Contract as is not terminated;

3. Terminate all orders and subcontracts to the extent that they relate to the performance of Work terminated by the Notice of Termination;

4. Settle all outstanding liabilities and all claims arising out of such termination of orders and subcontracts, the cost of which would be reimbursable, in whole, or in part, in accordance with the provisions of the Contract;

5. Submit to the Engineer a list, certified as to quantity and quality, of any or all items of termination inventory exclusive of items the disposition of which had been directed or authorized by the Engineer;

6. Transfer to the Engineer the completed or partially completed plans, drawings, information, and other property which, if the Contract had been completed, would be required to be furnished to the Owner;

7. Take such action as may be necessary, or as the Engineer may direct, for the protection and preservation of the property related to the Contract which is in the possession of the Contractor and in which the Owner has or may acquire any interest.

The Contractor shall proceed immediately with the performance of the above obligations notwithstanding any delay in determining or adjusting the amount of any item of reimbursable cost under this clause.

When the Owner orders Termination of Work effective on a certain date, all completed units of Work within each pay item as of that date shall be paid for at the Contract unit bid price. Payment for materials included in the material inventory described in #5 above shall be paid at actual cost delivered to the project or storage site, including transportation charges. Allowable total markup on the actual cost shall not exceed fifteen percent (15%).

After receipt of a Notice of Termination, the Contractor shall submit to the Engineer his claim for alleged additional damages or costs not covered above or elsewhere in these Specifications as provided in Article 5.21 - Claims for Additional Compensation. In no event, however, shall loss of anticipated profits be considered as part of any claim and/or settlement.
**Article 5.30 Use of Completed or Uncompleted Portions**

The Owner shall have the right to take possession of and use any completed or partially completed portions of the Work, prior to the date specified for completion, and such action and use shall not be considered an acceptance of that Work. If such use by the Owner causes additional expense to the Contractor and/or delay in the Work, the Contractor may be entitled to additional compensation and/or an extension of time. Claims for additional compensation or a time extension shall follow the procedures set forth in Article 5.21 - Claims for Additional Compensation. The Owner shall be responsible for routine maintenance or damages caused by its use of such portions of the Work.

**Article 5.31 Winter Suspension**

Unless otherwise specified in the Special Provisions, the Engineer may suspend the Work when adverse winter weather conditions make it impractical to secure the desired results. Where the Work is suspended for adverse winter weather conditions, the Contractor shall not be entitled to additional compensation.

Winter Suspensions generally occur on October 15 of each construction season. Although this date may vary from year to year, the Contractor should plan his Work in anticipation of a Winter Suspension occurring at or about this time each year. The Contractor shall schedule and sequence all operations such that the condition of the Work meets Suitable Conditions for Winter Maintenance, as described in this Article under SubArticle C below, to provide for routine maintenance by the Owner during the Winter Suspension period.

If the Work area is judged by the Owner to not meet Suitable Conditions to provide for routine maintenance during the Winter Suspension period, the Contractor shall be responsible for all costs necessary to establish Suitable Conditions for Winter Maintenance, including costs incurred by the Owner to prepare the site to meet Suitable Conditions and/or the payment of Excess Maintenance Costs as described in SubArticle B below.

A. Coordination with the Owner

The Contractor shall meet with the Engineer no later than September 15th to outline the Work to be completed before Winter Suspension. At the meeting, the Contractor shall provide a written Winter Suspension Plan describing the Work to be completed prior to the Winter Suspension period, including an updated progress schedule, clear definitions of the Work underway and the proposed condition of each element of the Work at the time of the anticipated Winter Suspension. The Winter Suspension Plan shall be prepared to achieve Suitable Conditions for Winter Maintenance, as described in SubArticle C below.

Before Winter Suspension, the Contractor shall, at his own expense, do all Work necessary to establish Suitable Conditions for Winter Maintenance in accordance with the Contractor’s Winter Suspension Plan. The Contractor shall then schedule a field review for acceptance by the Engineer. Within two (2) days following the field review, the Engineer shall prepare a punch list of deficiencies the Contractor is
required to correct prior to acceptance of Suitable Conditions for Winter Maintenance. The Contractor shall correct all items on the punch list to the satisfaction of the Engineer by the date shown on the punch list. During this period, the Contractor may continue to perform Work, as long as it is performed in such a manner that it does not compromise the Contractor’s ability to achieve Suitable Conditions for Winter Maintenance.

If the Contractor meets all the coordination requirements described herein, the Engineer shall issue a Notice of Acceptance for Winter Maintenance by the Owner. If the Contractor fails to satisfy all the coordination requirements described herein, the Owner may impose any and all Remedies for Failure to Comply, as described in SubArticle B below.

If unusual weather, scheduling constraints, or other unforeseen conditions make it difficult to initiate or continue the Work in distinct areas within the project limits, the Contractor may be entitled to a time extension to the Contract Completion Date if:

1. The Contractor notifies the Owner, prior to September 15, of the issues affecting the Contractor’s ability to perform the Work; or
2. The Contractor establishes that initiating or continuing the Work will result in conditions not suitable for winter maintenance by the Owner; or
3. The Contractor specifically identifies those distinct areas where the Work cannot be continued or initiated because it will result in conditions not suitable for winter maintenance by the Owner.

If the Owner agrees that a time extension to the Contract Completion Date is warranted, the Owner shall issue a Change Order acknowledging the revised Contract Completion Date. However, the Contractor shall not be entitled to additional compensation as a result of the issuance of such a time extension.

B. Remedies for Failure to Comply

The Engineer may apply any, some, or all of the remedies identified in this SubArticle if (1) the Contractor has not presented a written Winter Suspension Plan prior to September 20th; (2) at any time, in the opinion of the Engineer, the Contractor does not appear to be preparing the Work for Winter Suspension in a reasonable manner; (3) the Contractor fails to correct punch list items for Winter Suspension; or (4) for any reason the Work is found to be unsuitable for maintenance by the Owner during the Winter Suspension period.

Remedies the Engineer may utilize include the following:

1. Direct the Contractor to complete the Work required to meet Suitable Conditions for Winter Maintenance at the Contractor’s expense.
2. Using the forces of the Owner or a separate contractor, complete the Work required to meet Suitable Conditions for Winter Maintenance. Costs incurred by the Owner due to the Contractor’s failure to obtain a Notice of Acceptance for Winter Maintenance from the Owner shall be borne by the Contractor, including but not necessarily limited to Work required of the Owner’s forces, the cost of separate contractors retained by the Owner, and/or any claims made against the Owner by the abutting property owners or the public.

3. If the Contractor fails to prepare the Work to meet Suitable Conditions for Winter Maintenance, and the Owner is unable to correct the Contractor’s Work to achieve Suitable Conditions for Winter Maintenance, the Excess Maintenance Costs incurred by the Owner above and beyond those costs reasonably necessary to maintain the road had it been prepared to meet Suitable Conditions for Winter Maintenance shall be borne by the Contractor. Excess Maintenance Costs shall be determined by the Owner and submitted to the Contractor at the end of the winter maintenance period.

If the Engineer determines it is in the best interest of the Owner, the abutting property owners, or the public, he may also apply any of these remedies to specific elements or distinct areas of the Work while applying other remedies to other elements or distinct areas of the Work. If, in the opinion of the Engineer, it is in the best interest of the Municipality to delete portions of the Suitable Conditions for Winter Maintenance, the Engineer may issue written notice to the Contractor.

C. Suitable Conditions for Winter Maintenance

Suitable Conditions for Winter Maintenance shall be determined by the Engineer and shall include the following:

1. Travel ways that are to be paved in their final condition as a part of the Contract shall be paved as follows:

   a. Final pavement as shown on the Drawings, or

   b. The bottom layer of pavement shown on the Drawings; placed according to the Contract Documents at the design thickness of the layer. (In the event the layer is less than one and one-half inches (1.5”) thick, the Engineer may require the thickness be increased.); or

   c. Temporary AC Pavement (Class E), typically twenty-four feet (24’) wide, two inches (2”) in thickness, along the project centerline in those areas designated by the Engineer. If Temporary AC Pavement is designated by the Engineer, the Contractor shall be paid for the installation by Change Order at seventy-five percent (75%) of the Contract unit rate for AC Pavement.
2. Drainage ways that are to be paved with curb and gutter, valley gutter, paved shoulders or paved swales in their final condition as a part of the Contract shall be paved as follows:
   
a. Final curb & gutter, valley gutter or pavement as shown on the Drawings; or

b. Temporary AC Pavement (Class E), two inches (2") in thickness, in those areas designated by the Engineer. If Temporary AC Pavement is designated by the Engineer, the Contractor shall be paid for the installation by change order at seventy-five percent (75%) of the Contract unit rate for AC Pavement.

For temporary drainage facilities to be deemed suitable, all collection points included in the project design shall be functional. Where Best Management Practices are in place for Erosion and Sediment Control, those features shall be made suitable for the winter to the satisfaction of the Engineer.

3. All obstacles to snow clearing, snow storage, and snow loading and hauling shall be removed or diminished to the satisfaction of the Engineer. The space required for snow clearing, storage, loading and hauling shall be as determined by the Engineer.

4. Illumination, traffic signals, and signage shall be in proper working order.

5. All existing roads affected by the Work shall be returned to full operation.

6. A minimum of one half inch (0.5") of cover must be provided for all utilities below the surface of the travel way during the Winter Suspension period. If subsequent adjustments to the utilities become necessary when the travel ways are completed at a later date, the cost of these subsequent adjustments shall be considered incidental to the Contract.

7. Temporary or permanent backfill must be installed behind all curbs and medians to eliminate tripping hazards during the Winter Suspension period.

Installation of Temporary AC Pavement shall not be a basis for any time extension or additional costs, other than the actual cost to install the Temporary AC Pavement as described above.

D. Owner and Contractor Responsibilities During the Winter Maintenance Period

The Owner shall perform the routine winter maintenance operations specified below during the Winter Suspension Period. Routine winter maintenance shall include and is limited to the following:

1. Maintaining the traveled way and/or detour surface.
2. Maintaining drainage facilities except final cleaning of storm drains.

3. Maintaining access to abutting properties.

The Contractor shall remain responsible for all other elements of the Work, including those described by Section 10.04, Article 4.15 – Temporary Erosion Control and Storm Water Pollution Prevention Plans for Construction, throughout the Winter Suspension period.

During the Winter Suspension period, the Contractor shall continue to be responsible for the protection of the Work and shall repair all damage at the Contractor’s expense except where the damage is caused by the Owner’s maintenance forces. When the Work is resumed, the Contractor agrees to accept the traveled way and drainage system as the Owner has maintained it and no claim shall be made because of its condition or the manner in which the Owner performed the maintenance.

**Article 5.32 Pre-Construction Conference**

Prior to the start of Work and within five (5) working days after delivery of the executed Contract by the Owner to the Contractor, the Owner shall hold a Pre-Construction Conference to (1) review the Contractor’s schedules and Drawings; (2) establish procedures for handling shop drawings and other submissions; (3) establish procedures for submitting and processing applications for payment; and (4) address any other general housekeeping issues as necessary. Contractor and his Superintendent and Subcontractors shall attend the conference to meet with the Owner or his Representative, Engineer, and Inspector.

**Article 5.33 Use of Explosives**

Unless specifically authorized in the Special Provisions, the use of explosives is prohibited.
SECTION 10.06 LEGAL RELATIONS AND RESPONSIBILITIES

Article 6.1 Laws to be Observed

The Contract shall be governed by the laws of the State of Alaska. The Contractor at all times shall observe and comply with all Federal, State and local laws, ordinances, and regulations in any manner affecting the conduct of the Work and all such orders or decrees existing or which may be enacted or promulgated by legislative bodies, boards, tribunals or Courts having any jurisdiction or authority over the Work. The Contractor shall defend, indemnify, and hold harmless the Municipality and the officers, employees, and agents of the Owner, including the Engineer, against any claim or liability arising from or based on the violation of any such laws, ordinances, regulations, orders, or decrees, whether such violations be by the Contractor, his Subcontractor(s), or his employees.

Article 6.2 Notice to Contractors

Any written notice to the Contractor by the Owner shall be served on said Contractor or his representative either personally or by mailing to the address given in the Contract. Owner shall not use electronic facsimile transmissions to serve notice to Contractor.

Article 6.3 Notice by Contractors

Any notice to the Owner by the Contractor shall be made in writing delivered to the Engineer or his representative in person or mailed to the office of the Engineer at the address given in the official Notice to Proceed. Contractor shall not use electronic facsimile transmissions to serve notice to the Owner.

Article 6.4 Successors and Assigns

The Contractor binds himself, his partners, successors, assigns, and legal representatives to the Owner with respect to all covenants, conditions, and obligations contained in the Contract Documents.

Article 6.5 Assignments

The Contractor shall not assign the whole or any part of the Contract or any monies due or to become due the Contractor without written consent of the Owner. If the Contractor assigns all or any part of any monies due or to become due him, the instrument of assignment shall state that the right of the assignee in and to any monies due or to become due to the Contractor shall be subject to prior claims of all persons, firms, and corporations who performed Work or supplied materials under the Contract.

Article 6.6 Permits

All permits or licenses which are required by any federal, state or local governmental agency or any public utility and not provided by Owner shall be obtained and paid for by the Contractor when such permits or licenses are necessary for the prosecution of the
Work. The Contractor shall be responsible for all stipulations of these permits and shall be responsible for all costs associated with these permits and their stipulations.

The Contractor shall give all notices and comply with all laws, ordinances, rules and regulations bearing on the conduct of the Work as specified herein. The Contractor shall also be responsible for requesting all code compliance inspections.

The Owner shall obtain the required permits and authorizations for Work within the State of Alaska Railroad rights-of-way and permits from the U.S. Corps of Engineers, U.S. Fish and Wildlife Service, and State Department of Fish and Game. Prior to the start of Work within the scope of such permits, the Contractor shall obtain the necessary approvals and permits relating to the method, plan and exact schedule of construction for any Work within such rights-of-way, creeks and wetlands.

Failure on the part of the Contractor to comply with any of the stipulations of any of the applicable Owner- or Contractor-acquired permits shall be sufficient cause for the Owner to suspend that Work. Suspension of Work based on the foregoing shall not be subject to Section 10.05, Articles 5.21 – Claims for Additional Compensation or 5.24 – Suspension of Work for a basis of a claim by the Contractor.

The payment of basic and special fees, established under AMC 24.30.100, Permit Fees For Permanent Uses of Public Places (street use ordinance), and which are applicable to the project, shall not be the responsibility of the Contractor. These fees shall not be considered a bid item, nor shall they be considered incidental to any bid item.

The Contractor shall obtain permits and fulfill all other requirements of M.A.S.S., the Municipal Code, the Building Official, and the Municipal Engineer.

The Contractor shall obtain and pay for all permits, deposits and connection fees for tapping any required water and/or wastewater service connection for new and disrupted service connections.

Where sanitary sewer is available, the Contractor may apply for a dewatering permit from AWWU for dewatering discharge to the sanitary sewer. Contractor shall comply with Division 20, Section 20.12 – Dewatering for dewatering activities.

**Article 6.7 Copyrights and Patents**

The Contractor shall defend, indemnify and hold harmless the Municipality, its officers, employees, and agents of the Owner, including the Engineer from any and all claims, suits, or actions brought for the infringement of any copyright or patent claimed to be infringed by any material, devices, drawings, method, or process to be incorporated in the Work and/or required to be used in connection with the Work, including all attorney's fees and costs.

**Article 6.8 Safety**

The Contractor shall be solely and completely responsible for conditions of the jobsite, including safety of all persons (including employees, Owner's representatives, and the
public) and property during performance of the Work. This requirement shall apply continuously twenty-four (24) hours per day, seven (7) days per week and shall not be limited to normal working hours. Safety provisions shall conform to U.S. Department of Labor (OSHA), the State Occupational Safety and Health Act, and all other applicable Federal, State, County, and local laws, ordinances, codes, the requirements set forth below, and any regulations that may be detailed in other parts of the Contract Documents. Where any of the aforementioned safety provisions, laws, ordinances, and/or Contract Document requirements is in conflict, the more stringent requirement shall be followed. The Contractor's failure to thoroughly familiarize himself with the aforementioned safety provisions shall not relieve him from compliance with the obligations and penalties set forth herein.

The Contractor shall develop and maintain for the duration of the Contract, a safety program that will effectively incorporate and implement all required safety provisions. The Contractor shall appoint an employee who is qualified and authorized to supervise and enforce compliance with the safety program and shall notify the Engineer of the name and contact phone number for this person prior to commencement of the Work.

The duty of the Engineer to conduct construction review of the Work does not include review and/or approval of the adequacy of the Contractor's safety supervisor, the safety program, or any safety measures taken in, on, or near the construction site.

If death or serious injuries or serious damages are caused, the accident shall be reported immediately by telephone or messenger to both the Engineer and the Owner. In addition, the Contractor must promptly report in writing to the Engineer all accidents whatsoever arising out of, or in connection with, the performance of the Work whether on, or adjacent to, the site, giving full details and statements of witnesses.

If a claim is made by anyone against the Contractor, including his employees and agents, or any Subcontractor on account of any accident, the Contractor shall promptly report the facts in writing to the Engineer, giving full details of the claim.

Failure to comply with the Occupational Safety and Health rules and regulations, notwithstanding any other provision of the Contract, is sufficient cause for termination under the provisions of the Contract.

### Article 6.9 Insurance

As a prerequisite to execution of the Contract, the Contractor shall obtain all insurance required under this Article. The Contractor shall maintain this insurance until the Final Acceptance Date. The Contractor shall file with the Purchasing Officer as verification of insurance a certificate of insurance on the forms furnished or otherwise approved form, showing the type and amounts of insurance, the policy number, and expiration date. The certificate must be signed by an authorized representative of the insurance company. Each certificate of insurance shall state that the insurance company will provide not less than thirty (30) days written notice to the Engineer and the Risk Manager of the Municipality of any material change, cancellation, or non-renewal of the insurance policies. All General Liability and Automobile Liability insurance policies required under this Article...
shall name the Municipality as an additional insured for the purposes of this Project and shall contain a waiver of subrogation against the Municipality.

The Contractor shall provide the following types of insurance:

<table>
<thead>
<tr>
<th>Insurance Type</th>
<th>Minimum Limits</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>Workers' Compensation</strong></td>
<td></td>
</tr>
<tr>
<td>$500,000 Employer's Liability and Worker's Compensation as required by Alaska State Workers Compensation Statutes.</td>
<td>$500,000</td>
</tr>
<tr>
<td><strong>Commercial General Liability</strong></td>
<td></td>
</tr>
<tr>
<td>Bodily Injury and Property Damage Liability</td>
<td>$1,000,000 Combined Limit Each Occurrence</td>
</tr>
<tr>
<td>Premises Operations including explosion,</td>
<td>and $2,000,000 Aggregate</td>
</tr>
<tr>
<td>collapse and underground;</td>
<td></td>
</tr>
<tr>
<td>Products and Complete Operations;</td>
<td></td>
</tr>
<tr>
<td>Broad Form Property Damage;</td>
<td></td>
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<tr>
<td>Blanket Contractual;</td>
<td></td>
</tr>
<tr>
<td>Personal Injury</td>
<td></td>
</tr>
<tr>
<td>Owner's/Contractor's Protection</td>
<td></td>
</tr>
<tr>
<td><strong>Commercial Automobile Liability</strong></td>
<td></td>
</tr>
<tr>
<td>Bodily Injury and Property Damage,</td>
<td>$1,000,000 Combined Limit per Accident</td>
</tr>
<tr>
<td>including all owned, hired and non-owned</td>
<td></td>
</tr>
<tr>
<td>automobiles</td>
<td></td>
</tr>
</tbody>
</table>

When specified in the Special Provisions the Contractor shall provide the following additional coverages:

<table>
<thead>
<tr>
<th>Insurance Type</th>
<th>Minimum Limits</th>
</tr>
</thead>
<tbody>
<tr>
<td>Federal Longshoremen and Harbor Workers</td>
<td>Statutory</td>
</tr>
<tr>
<td>Compensation Act</td>
<td></td>
</tr>
<tr>
<td>Federal Maritime Liability Law (Jones Act)</td>
<td>$1,000,000 Total Contract Amount</td>
</tr>
<tr>
<td>Builder's Risk</td>
<td></td>
</tr>
</tbody>
</table>

NOTICE TO “OUT OF STATE” CONTRACTORS

A Certificate of Insurance for Alaska Worker's Compensation, or an “other states” endorsement on Contractor’s home state Worker’s Compensation policy, is required prior to execution of a Contract or commencement of any Contract performance, if any in-state visits or Work is required or anticipated.

**Article 6.10 Indemnification**

To the fullest extent permitted by law, the Contractor shall indemnify, defend, and hold harmless the Municipality and the Engineer and their agents and employees from and
against all claims, damages, losses and expenses including attorneys' fees arising out of or resulting from the performance of the Work, provided that any such claim, damage, loss or expense is (1) attributable to bodily injury, sickness, disease, death or personal injury, or to injury to or destruction of tangible property including the loss of use resulting therefrom, and (2) caused in whole or in part by any negligent act or omission of the Contractor, any Subcontractor, anyone directly or indirectly employed by any of them or anyone for whose acts any of them may be liable, regardless of whether or not it is caused in part by a party indemnified hereunder.

In any and all claims against the Municipality or the Engineer or their agents or employees by any employee of the Contractor, any Subcontractor, anyone directly or indirectly employed by any of them, or anyone for whose acts any of them may be liable, the indemnification obligation under this Article shall not be limited in any way by any limitation on the amount or type of damages, compensation or benefits payable by or for the Contractor or any Subcontractor under Worker's Compensation acts, disability benefit acts, or other employee benefit acts.

**Article 6.11 Claims by Workmen, Suppliers, and Subcontractors**

In the event the Contractor or any Subcontractor fails, neglects, or refuses to make prompt and full payment for labor, services, materials, supplies, or provisions furnished by any person in connection with the Work, the Owner may withhold the amount due from the Contractor's progress payments provided that an affidavit of claim on the form furnished is filed with the Engineer. The withholding by the Owner does not relieve the Contractor or his Surety from their obligations with respect to the payment of such claims. Sums withheld from progress payments shall be disbursed pursuant to Section 10.07 Article 7.6 - Payment of Claimants.

**Article 6.12 Certified Payroll**

The Contractor shall file with the Alaska Department of Labor, Wage and Hour Administration, Labor Standards and Safety Division, a certified payroll, as required and at such frequency as required by the State of Alaska.

**Article 6.13 Lawsuits**

If a lawsuit is filed by the Contractor or his Surety against the Municipality or by the Municipality against the Contractor or his Surety, the suit shall be commenced in the Third Judicial District in Anchorage, Alaska.

If one of the questions at issue is the satisfactory performance of the Work by the Contractor, and should the appropriate Court decide that the Work of the Contractor was unsatisfactory, then the Contractor or his Surety shall reimburse the Owner for all legal and all other expenses incurred by the Owner because of the lawsuit as may be allowed and set by the Court. Further, it is agreed that the Owner may deduct such costs from any sum or sums then due or that may become due the Contractor under the Contract.
If any clause or condition of the Contract is held as a matter of law to be unenforceable or unconscionable, the remainder of the Contract shall be enforceable without such clause.

**Article 6.14 Preference to Local Labor**

Where applicable, the Contractor shall comply with the provisions of AS 36.10 requiring employment preference for Alaska residents.

**Article 6.15 State of Alaska Prevailing Wage Scale**

Where applicable, the Contractor shall comply with AS 36.5 for the payment of prevailing wages to their employees.

If the Contract contains State of Alaska wage rates and a Federal Wage Decision, the Contractor and all Subcontractors shall comply with both wage decisions. The Contractor and all Subcontractors shall be responsible for paying the higher pay rate between the state and federal wage decisions. Additionally, the Contractor and all Subcontractors shall be responsible for providing certified payrolls to both the Engineer and the State of Alaska, Department of Labor, Wage and Hour Division on a weekly basis utilizing the appropriate agency’s form(s).

**Article 6.16 Nondiscrimination**

The Contractor shall not discriminate against any employee or applicant for employment on any basis prohibited by law. The Contractor shall take affirmative action to ensure that applicants are employed and that employees are treated during employment without regard to any attribute protected by law. Such action shall include, without limitation, employment, upgrading, demotion or transfer, recruitment or recruiting advertising, lay-off or termination, rates of pay or other forms of compensation, and selection for training including apprenticeship. The Contractor agrees to post in conspicuous places, available to employees and applicants for employment, notices setting forth their rights with regard to employment discrimination.

The Contractor shall state, in all solicitations or advertisements for employees for the Work, that all qualified applicants will receive consideration for employment without regard to any attribute protected by law.

The Contractor shall comply with any and all reporting requirements which the Anchorage Office of Equal Employment Opportunity Contract Compliance may establish by regulation.

The Contractor shall include the provisions of this Article in every subcontract or purchase order under the Contract, so as to be binding upon every such Subcontractor or vendor of the Contractor under the Contract.

**Article 6.17 Rights and Remedies**

The duties and obligations of the Contractor imposed by the Contract Documents and the rights and remedies of the Owner available thereunder shall be in addition to and not a
limitation of any duties, obligations, rights, and remedies otherwise imposed or available by law.

The failure of the Owner or the Engineer to insist in any one or more instances upon the strict performance of any one or more of the provisions of the Contract, or to exercise any right herein contained or provided by law, shall not be construed as a waiver or relinquishment of the performance of such provision or right(s) or of the right to subsequently demand such strict performance or exercise such right(s), and the rights shall continue unchanged and remain in full force and effect.

**Article 6.18  Payment of Taxes**

As a condition of performance of the Contract, the Contractor shall pay all municipal taxes incurred by the Contractor. Satisfactory performance of this paragraph is a condition precedent to payment by the Municipality under the Contract.
SECTION 10.07 MEASUREMENT AND PAYMENT

Article 7.1 Method of Measurement

All Work completed under the Contract shall be measured by the Engineer according to United States standard measures, unless otherwise stated in the Bidding Documents.

When any vehicle delivers to the project classified fill or backfill of any kind, bedding material, leveling course, pavement materials, or any other material measured by weight, the driver of the vehicle shall give to the inspector a legible "original" computer-generated or machine-printed weight ticket with the following information:

1. Vehicle identification number.
2. License number & associated trailer license number(s).
3. Tare weight of the vehicle(s).
4. Gross weight of the loaded vehicle(s) as registered on the scale.
5. Maximum allowable vehicle weight (MAVW) or legal gross weight of the vehicle(s) as permitted by AMC 9.46.090 or AMC 9.46.100.
6. Sequential ticket number, date, time of weight, pay item in words, and project location.
7. Bid Schedule and Number of Item.
8. Pit location and name of scale operator.

The Owner shall not be required to pay for that portion of the load in excess of the legal gross weight.

Vehicle(s) shall be tared a minimum of once daily by the scale operator. The Engineer may request additional tares to be done at any time the scale is operational. The Engineer may also require that he be present when tares are done.

If the Contractor is not providing fill materials from a commercially established material source in the Municipality, and if the Contractor currently does not have at the other approved material source a computer-generated or machine printed weight ticket system, the Contractor shall furnish competent scale operators to weigh all materials measured and paid for on a weight basis. The scale operator(s) shall operate the scale(s) and keep records as directed by the Engineer, including the information as listed in the above seven (7) items. In addition, the scale operator shall keep a scale diary on a project by project basis. The scale diary shall be presented to the Engineer, on a daily basis, certifying that entries in the diary are true and correct for the specific project. The Owner shall make no direct payment to the Contractor for furnishing scale operator(s), equipment, and expendables required, the costs thereof being considered an incidental Contractor obligation. The accuracy of all scales, both private and commercial, is the responsibility of the Contractor. The Contractor shall maintain scales according to the specifications, tolerances and regulations for commercial weighing and measuring devices contained in the National Bureau of Standards, Handbook 44, as adopted by AS 45.75.050(d).
Article 7.2 Scope of Payment

The Contractor shall accept the compensation as set forth in the Contract Documents in full payment for the Work. The Contractor shall do all things necessary to perform and to complete the Work according to the Contract Documents, including but not limited to furnishing all labor, tools, implements, machinery, supplies, materials, water, heat, utilities, transportation, and permits necessary to perform the Work. The Contractor shall be responsible for all loss, damage, or liability arising from the nature of the Work or from the action of the elements or from any unforeseen difficulties which may be encountered. Work paid for under one item shall not be paid for under another item.

The Contract price shall constitute full compensation for furnishing all plant, labor, equipment and materials, and performing all operations required to complete the Work as specified and as shown on the Drawings or otherwise directed. Notwithstanding the omission or mention of any incident or incidental Work, the Contract price and payment shall also constitute full compensation for all Work incident or incidental to completion of the items, unless such Work is otherwise specifically mentioned for separate payment under another bid item. In the event any Work is required by the Contract Documents, but is not identified as being directly incident or incidental to the completion of any Contract item, the Contract price or prices for all enumerated items shall also constitute full compensation of such Work.

In this Section, the terms "construct, furnish, install, erect, place, and prepare," shall be construed to mean that the bid item(s) is/are complete, in place, and approved by the Engineer.

Article 7.3 Advances on Materials

The Contractor may request advance payment for materials to be incorporated in the Work, provided such materials are delivered and stored at the site, or if approved by the Engineer, at another site within the Municipality. The Contractor shall be solely responsible for the protection of these materials. Only the Contractor's costs of materials (including freight) as verified by invoices, shall be considered for such advance payments by the Owner.

No payment shall be made on any single class of material valued at less than $5,000. No advance payment shall be made for fuels, supplies, forms, lumber, falsework, or other materials, or on temporary structures of any kind which will not become an integral part of the finished construction. Retainage shall not be withheld for advance payment for materials to be incorporated in the Work. However, once the materials have been incorporated in the Work and payment is requested the retainage in Article 7.5 - Progress Payments, shall apply.

The Contractor shall make available to the Engineer evidence of payment for the materials for which he is requesting advances, insurance to assure replacement if lost, stolen or damaged, and other information the Engineer may request.
Article 7.4 Change Order Compensation Adjustments

Each Change Order Proposal shall include a clear summary of the Contract requirements; the reason for the requested change; a description of the change and whether additional time and/or other compensation is requested or credit offered to the Owner. Unless formalized by an executed Change Order, any and all increased costs or delays resulting directly or indirectly from an unapproved Change Order Proposal shall be borne solely by the Contractor.

Any compensation paid in conjunction with the terms of a Change Order shall constitute total compensation due the Contractor for the Work or alteration defined in the Change Order. By signing the Change Order, the Contractor acknowledges that the stipulated compensation includes payment for the Work or alteration plus all payment for the interruption of schedules, extended overhead, delay or any other impact claim or consequential effects and, by such signing, specifically waives any reservation or claim for additional compensation with respect to the subject of the Change Order.

A. Negotiated Changes

When extra Work is ordered by the Engineer to be performed on a negotiated unit or lump sum basis, the Contractor shall be required to submit a properly itemized Change Order Proposal covering all the additional Work and/or Work to be deleted. The proposal shall be itemized for the various components of Work and segregated by labor, material, and equipment costs in a format satisfactory to the Engineer. Each proposal shall include similar itemized costs for all subcontractors, regardless of tier. The labor, material and equipment components of each proposal shall include the following:

1. Labor

   Labor costs shall include the direct hourly cost of labor stated on the certified payroll for each labor classification plus other direct labor costs including, but not limited to, FICA, Workers' Compensation, ESC, and public liability and property damage insurance when premiums are based on a percentage of payroll. The labor costs shall include only those direct labor hours required to perform the changed Work for workers and working foremen. Supervision above the level of working foremen (such as general foremen, superintendents, and project managers, etc.) shall not be included in labor costs and shall be considered to be included in the Overhead and Profit Markup as described hereafter in this Article.

2. Materials

   Costs for materials and supplies, including freight, shall be based on the net actual cost of the material and supplies required to perform the changed Work, as verified by appropriate vendor and third party invoices. Material costs shall reflect cost reductions available to the Contractor due
to trade discounts, volume rebates, and price reductions for prompt payments, if applicable. Material costs must be itemized to display the unit price for each specific item incorporated into the Work.

3. Equipment Rentals

Time for both owned and rented equipment shall be estimated to the nearest one-quarter hour for purposes of computing compensation to the Contractor for equipment utilized under these rates.

The equipment rates for both owned and rented equipment as determined below shall be full compensation for providing the required equipment and no additional compensation shall be made for other costs such as, but not limited to, fuels, lubricants, replacement parts or maintenance. Cost of repairs, both major and minor, as well as charges for mechanic's time utilized in servicing equipment to ready it for use prior to moving to the project and similar charges shall not be allowed.

When it is necessary to obtain equipment from sources beyond the project limits at the request of Owner exclusively for changed Work, the actual cost of transferring the equipment to the site of the Work and return shall be allowed as an additional item of expense. Where the move is made by common carrier, the move-in allowance shall be limited to the amount of the freight bill or invoice. If the Contractor hauls the equipment with his own forces, the allowance shall be limited to the rental rate for the hauling unit plus operator wages. Move-in allowance shall not be made for equipment brought to the project for changed Work which is subsequently retained on the project and utilized for completion of Contract items.

a. Contractor Owned Equipment (over $500)

For any Contractor-owned machinery or special equipment, the Contractor shall include costs for the rental rates in the current edition and appropriate volume of the "Rental Rate Blue Book For Construction Equipment," (hereinafter referred to as the "Blue Book"), published by Dataquest, Inc. These rates do not apply to equipment or tools valued at less than five hundred dollars ($500). Hourly rental rates shall be determined as follows:

1. The established hourly rental rate shall be equal to the monthly rate for the basic equipment plus the monthly rate for applicable attachments as set forth in the "Blue Book", necessary to perform the Work, both divided by 176, all multiplied by the area adjustment factor, plus the estimated hourly operating costs listed in the Blue Book.
2. The area adjustment factors shall be applied to those sections in the "Blue Book" containing an area adjustment map.

3. The "Equipment Life" adjustment factor sections shall not apply.

For Contractor-owned equipment not listed in the “Blue Book”, the Contractor shall receive a rental rate as agreed in writing between the parties before the changed Work is begun. If agreement cannot be reached, the Engineer reserves the right to establish a rate based on similar equipment shown in the “Blue Book” or based on prevailing commercial rates in the area.

b. Rented Equipment (over $500)

Costs for equipment brought to the Work site and rented or leased specifically for Work required under this Section shall be included at the actual rental rate and supported by invoices from the equipment vendor. Rental rates for equipment shall be consistent with prevailing rates for similar equipment in the area.

Costs for rented equipment previously on the site and utilized specifically for changed Work shall be included at the actual rental rate and supported by invoices from the equipment vendor, provided the hourly rate for this equipment shall not be greater than the hourly rate paid for that same equipment for other Work in the Contract.

c. Equipment less than $500

Equipment, tools, and other specialty items valued at less than $500 are included in Profit and Overhead and no separate payment shall be made.

4. Allowances for Profit and Overhead

Contractor Change Order Proposals for the performance of changed Work shall include all direct costs for labor and materials as described above. The Engineer shall review the proposals for reasonableness and adequate detail in order to reach agreement with the Contractor before including allowances as described below:

a. In addition to the direct costs of labor, materials and equipment incurred by the Contractor, the Contractor shall be entitled to an allowance for profit and overhead. This allowance shall not exceed twenty percent (20%) of the total direct cost of labor and materials.
b. If Work is performed by a subcontractor, the subcontractor actually performing the Work shall be entitled to those allowances for profit and overhead listed above, and each subsequent higher tiered subcontractor or Contractor shall be allowed up to an additional ten percent (10%) markup on the subcontractor's direct costs, up to a maximum of two tiers of subcontractors.

The allowance made in accordance with the terms outlined above shall be complete reimbursement and compensation for all indirect costs associated with changed Work including, but not limited to, job office overhead, home office overhead, project management, superintendents, general foremen, estimating, engineering, detailing, legal, accounting, shop drawings, costs of small tools and small equipment, bond cost, insurance premiums, profits, delay impacts on the rest of the Work and losses of all kinds and other items of cost not specifically designated. No other reimbursement, compensation or payment shall be made for changed Work.

Any allowance made by the Contractor to a Subcontractor, other than specified herein, shall be at the expense of the Contractor.

B. Time and Material Changes

When extra Work is ordered by the Engineer to be performed on a time and materials basis, the Contractor shall be required to perform the extra Work at the actual direct cost for labor, materials and equipment plus allowances for profit and overhead as described under the Negotiated Changes clause of this Article. The amount of the allowance for profit and overhead for Time and Materials changes shall not exceed fifteen percent (15%). In order for payment to occur, the Contractor must document all direct costs in a manner acceptable to the Engineer. The Contractor shall provide daily time sheets with the names of all Contractors employees working on the changed Work, the number of hours each employee works on the changed Work, and a description of the Work performed. In addition, the Contractor shall provide daily records of all equipment used to perform the changed Work showing the number of hours each piece of equipment was used, a description of the Work performed, and the name of the equipment operator. All materials incorporated into the changed Work shall be documented with itemized invoices from vendors and suppliers.
C. Unit Price Changes

When extra Work is ordered by the Engineer to be performed on a unit price basis, payment shall be made for both added quantities and deductive quantities in accordance with those unit prices that have been incorporated into the Contract Documents, unless the Engineer determines there is an alternate method. For changed Work authorized by the Engineer, the Contractor shall submit a Change Order Proposal itemizing the quantities of each item of Work for which there is an applicable unit price. The applicable unit prices shall be applied to the net differences of all quantities of the same item. These unit prices shall be considered to cover all direct and indirect costs of furnishing and installing the item, including all profit and overhead. No additional markup for overhead and profit shall be allowed on unit priced items except where the actual quantity used exceeds one hundred and twenty-five percent (125%) of the estimated quantity. For additional unit price Work performed by subcontractors, each subsequent higher tiered subcontractor or Contractor shall be allowed up to an additional five percent (5%) markup on the subcontractor’s direct costs (not including profit and overhead), up to a maximum of two tiers of subcontractors.

D. No Cost Changes

The Engineer shall have authority to order changes in the Work that, at his sole discretion, do not require an adjustment in the Contract amount or an extension of time and are not inconsistent with the intent of the Contract Documents. Such changes shall be effected by written order and shall be binding on the Owner and the Contractor. The Contractor shall carry out such written orders promptly.

If the Contractor claims that such written instructions or orders involve extra costs or an extension of time, it shall make his claim by following the procedures set forth in Article 5.21 - Claims for Additional Compensation. The Contractor shall proceed with the Work as directed by the Engineer while his claim is being evaluated and shall not delay the Work while waiting for a decision.

Article 7.5 Progress Payments

The Contractor shall submit to the Engineer an Application for Payment, on the forms furnished, supported by such data as the Engineer may require substantiating the Contractor's right to payment for Work done during the preceding calendar month. The Engineer shall, within eight (8) days after receipt of the Application for Payment, either approve a Partial Payment Estimate and present it to the Contractor for signature or notify the Contractor in writing his reasons for withholding approval. Approved Partial Payment Estimates shall be received by the Owner within two (2) days after execution by the Contractor. The Owner shall process Partial Payment Estimates and make payment to the Contractor within fifteen (15) days of receipt of the Partial Payment Estimate. If the Owner fails to make payment to the Contractor within thirty (30) days of receipt of the Application for Payment (twenty-one (21) days if the project is funded with State of Alaska grants), the Contractor may, upon seven (7) days written notice to the Owner and Engineer, suspend
the Work. The Contractor shall take every precaution to prevent any damage or unreasonable deterioration of the Work during the time it is suspended. The Engineer may require a schedule of values, or cost breakdown for any lump sum payment Contract item.

For projects where a Performance and Payment Bond is required under Section 10.03, Article 3.5 – Bonds, Insurance, EEO and DBE/WBE Forms, progress payments at one hundred percent (100%) of the estimated value of the Work accomplished, less all previous payments or for authorized withholdings as specified below, shall be made to the Contractor and no deductions shall be made, except as provided under the withholding provisions of this Article.

No interest shall accrue and no interest shall be paid on sums which are withheld as provided for hereinafter.

**Withholding:** The Engineer may withhold funds from a progress payment for any of the following reasons:

1. Defective Work;
2. Claims made directly against the Municipality alleging an act or omission on the part of the Contractor, his employees, his agents, or Subcontractors in connection with the Work;
3. Damage to the Municipality;
4. Reimbursements for Work done by the Owner because of any failure by the Contractor or Subcontractor to carry out the Work in accordance with the Contract Documents;
5. Uncompleted incidental Work, not earning direct payment, including but not limited to testing, cleanup, updating of progress schedules, preparation of Record Documents and Operations and Maintenance Manuals.
6. Liquidated damages;
7. Claims by Subcontractors, suppliers, laborers, or the Alaska Department of Labor;
8. If the Contractor or his/her Subcontractor fails to file reports with the Municipality of Anchorage Office of Equal Opportunity as required by AMCR 7.50.004, AMCR 7.50.005, and by 7.60.004 in a timely manner, monies shall be withheld in the amount of ten percent (10%) of the amount due the Contractor until such time as the reports have been properly filed.

The amount of any withholding for items one (1) through five (5) above shall be the reasonable value of the Work or remedy to be accomplished as estimated by the Engineer, without regard to bid amount or cost to the Contractor. The amount of withholding for items
six (6) through eight (8) shall be in accordance with the claimed amount or the applicable Contract provisions.

Progress payments shall not be construed as an acceptance or approval of any part of the Work covered thereby and they shall in no manner relieve the Contractor of responsibility for correcting defective workmanship or material.

The estimates upon which progress payments are based are not represented to be accurate estimates, and all quantities shown therein are subject to correction on any subsequent pay estimate. If the Contractor uses such estimates as a basis for making payment to Subcontractors, Contractor does so at his own risk and Contractor shall bear all loss that may result. All quantities shall be subject to review by the Engineer prior to approval for payment.

The making of any progress payment under the Contract, either before or after the date set for completion of the Work, shall not operate to invalidate any of the provisions of the Contract or to release the Surety.

In addition to certified payroll submittals to the Alaska Department of Labor required under Section 10.06, Article 6.12 – Certified Payroll, Contractor shall attach certified payrolls to each request for payment or partial payment. Certified payrolls attached to Contractor’s request for payment or partial payment shall cover all pay periods in Contractor’s request for payment or partial payment.

Article 7.6 Payment of Claimants

By submitting a request for Final Payment, the Contractor acknowledges and certifies that all actual or potential claims or issues have been either resolved or withdrawn and that there are no such claims or issues outstanding.

Any claim received by the Engineer against the Contractor or Subcontractors from any materialmen, laborer, supplier, Subcontractor, or the Alaska Department of Labor will be forwarded to the Contractor by certified mail as soon as practical following receipt by the Engineer. Within twenty-one (21) days after the Contractor’s receipt of the said notice, the Contractor shall notify the Engineer in writing by Certified Mail that the said claim is contested or provide proof that the claim has been satisfied. If the Contractor contests the claim, the Contractor shall describe in detail how the Subcontractor was paid or why the Subcontractor should not be paid and furnish to Owner the completed three-point statement form described hereafter. If the Contractor does not respond during the time allotted above, this lack of notice shall constitute consent by the Contractor to have the Owner pay the claim from the earnings of the Contractor. The Owner shall not be responsible to the Contractor if the Contractor subsequently contests the validity of the claim.

Sums withheld pursuant to disputed claims shall not be paid to the claimant except where compelled by legal authority. Such sums may be paid to the Contractor upon the filing of a three-point statement by the Contractor and his Surety on the form furnished by the Engineer stating that: (1) the Contractor contests the validity of the claim; (2) that the
Surety acknowledges responsibility for the payment of the claim in the event it is valid and;
(3) that the Contractor and the Surety specifically agree to hold the Municipality harmless
for making payment to the Contractor of the sums withheld.

In the event that the Contractor revokes consent to pay a claimant as provided herein and
refuses to execute the three-point statement form referenced above, the Municipality may
institute an interpleader action in Superior Court, Third Judicial District, and all Court costs
and attorney's fees incurred by the Municipality shall be paid by the Contractor or the
Surety. Claimants are not intended beneficiaries of this Article and shall have no recourse
against the Municipality for any failure to pay claims from sums withheld from the
Contractor.

Article 7.7 Final Payment

Upon completion of the Work and issuance of a certificate of completion by the Engineer,
the Contractor shall submit a request for Final Payment to the Engineer. No Final Payment
shall be made until the Contractor has filed with the Engineer, prior to acceptance of the
Work, a notarized Certificate of Compliance in the form substantially as follows:

I (we) hereby certify that all Work has been performed and materials supplied in
accordance with the Contract Documents for the above Work, that not less than the
prevailing rates of wages as required by the State Statute have been paid to
labors, workmen, and mechanics, that all payroll taxes have been paid, and that
all claims for material and labor and other services performed in connection with
these Contract Documents have been satisfied.

There shall be deducted from the final payment any sums withheld pursuant to Article 7.6 -
Payment of Claimants.

Article 7.8 Correction of Work after Final Payment

Neither the final payment nor any progress payment shall relieve the Contractor of his
responsibility for paying all costs resulting from defects in materials or workmanship
supplied under the terms of the Contract, and for correction of those defects, for a period of
one year following the Final Acceptance Date. The Owner shall give notice of observed
defects within a reasonable time. The Contractor shall initiate corrective action within five
(5) days after written notification from the Owner or the Owner shall make other provisions
to complete the Work and all costs shall be paid by the Contractor.
SECTION 10.08 FORMS

Article 8.1 Current Forms

The following forms are provided as reference. Electronic versions of the form are available at the Municipality of Anchorage website or from the Engineer.

Article 8.2 Submittal Transmittal

The Submittal Transmittal form is provided on page 71.

Article 8.3 Request for Information Form

The Request for Information form is provided on page 72.

Article 8.4 Substitution Request Form

The Substitution Request form is provided on page 73.

Article 8.5 Deviation Request Form

The Deviation Request form is provided on page 74.
**SUBMITTAL TRANSMITTAL**

**PROJECT**

**CONTRACTOR**

**ORIGINATOR**

**DATE SUBMITTED**

**DRAWING NO.**

**TO:** Municipality of Anchorage  
Project Management and Engineering  
4700 Elmore Road  
Anchorage, AK 99507

**ATTENTION:**

**REVIEW ACTION**

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<td>(Provide Itemized List of Contents in this Submittal)</td>
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Complete either (a) or (b), following:

- (a) We have verified that the material or equipment contained in this submittal meets all the requirements specified or shown (no exceptions), and the submittal is required by the Contract Documents.
- (b) We have verified that the material or equipment contained in this submittal meets all the requirements specified or shown, except for the following deviations (list deviations; attach a separate sheet as necessary), and the submittal is required by the Contract Documents.

**CONTRACTOR**

(Signature)

**ENGINEER**

(Signature)

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Corrections or Comments made relative to submittals during this review do not relieve the Contractor from compliance with the requirements of the Drawings and Specifications. This submittal is only for review of general conformance with the design concept of the Project and general compliance with the information given in the Contract Documents. The Contractor is responsible for confirming and correlating all quantities and dimensions; selecting fabrication processes and techniques of construction; coordinating his work with that of other trades; and performing his work in a safe and satisfactory manner.

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Standard Construction Specifications  
Division 10 – General Provisions  
11/08
REQUEST FOR INFORMATION (RFI)

PROJECT
CONTRACTOR
ORIGINATOR
DATE SUBMITTED
DRAWING NO.
TO:
Municipality of Anchorage
Project Management and Engineering
4700 Elmore Road
Anchorage, AK 99507
ATTENTION:

REQUEST NO. RFI- 
CONTRACT NO. 
PM&E NO. 
SPEC. SECTION 
SHEET ___ OF ___

DESCRIPTION OF REQUEST FOR INFORMATION

CONTRACTOR (Signature) DATE
RESPONSE REQUESTED

RESPONSE TO REQUEST FOR INFORMATION

DIRECTION

☐ Requested Information/Clarification Provided.
☐ Submit a Change Order Proposal and all associated justification/documentation.
☐ 

RESPONSE BY (Signature) DATE

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Division 10 – General Provisions
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11/08
SUBSTITUTION REQUEST

PROJECT

CONTRACTOR

ORIGINATOR

DATE SUBMITTED

DRAWING NO.

TO:
Municipality of Anchorage
Project Management and Engineering
4700 Elmore Road
Anchorage, AK 99507

SPECIFIED ITEM

SPECIFIED ITEM

SUBSTITUTE NO.

SR-

CONTRACT NO.

PM&E NO.

SPEC. SECTION

SHEET ___ OF ___

ATTENTION:

SECTION | PAGE | PARAGRAPH | DESCRIPTION
--- | --- | --- | ---

PROPOSED SUBSTITUTION

JUSTIFICATION

Attached data includes product description, Specifications, Drawings, photographs and performance and test adequate for evaluation of the request. Applicable portions of the data are clearly identified.

The undersigned states that the following paragraphs, unless modified in attachments, are correct:

1. The proposed substitution does not affect dimensions shown on Drawings and will not require any change in the Contract Documents.

2. The undersigned will pay without reimbursement for construction costs and changes to the design including engineering and detailing caused by the requested substitution which is estimated to be $.

3. The proposed substitution will have no adverse affect on other contractors, the construction schedule (specifically the date of substantial completion), or specified warranty requirements.

4. Maintenance and service parts will be locally available for the proposed substitution.

5. The incorporation or use of the substitution in connection with the work is not subject to payment of any license fee or royalty.

The undersigned further states that the function, appearance, and quality of the Proposed Substitution are equivalent or superior to the Specified Item.

Submitted by CONTRACTOR

Reviewed by ENGINEER

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Received by

( name/company )

Date received

Date forwarded

Comments

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Division 10 – General Provisions

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DEVIAITION REQUEST

PROJECT ___________________________ REQUEST NO. DR-_____________________
CONTRACTOR ______________________ CONTRACT NO. _______________________
ORIGINATOR ________________________ PM&E NO. _________________________
DATE SUBMITTED ______________ DRAWING NO. __________ SPEC. SECTION _______ SHEET ___ OF ___
TO: Municipality of Anchorage
    Project Management and Engineering
    4700 Elmore Road
    Anchorage, AK 99507

ATTENTION: _______________________

DESCRIPTION OF DEVIATION REQUEST
A. Original Contract Requirements:

B. Reason for Deviation Request:

C. Proposed Deviation:

D. Any Changes in Contract Time or Cost: ☐ NO ☐ YES

CONTRACTOR __________________________ DATE __________
(Signature) RESPONSE REQUESTED

RESPONSE TO DEVIATION REQUEST

DIRECTION
☐ Approved ☐ Approved as Noted
☐ Disapproved

RESPONSE BY __________________________ DATE __________
(Signature)

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SECTION 20.01 GENERAL

Article 1.1 Scope of Work

The Work covered by this Division consists of providing all plant, labor, equipment, supplies, material, transportation, handling, and storage, and performing all operations pertaining to the: 1) construction of subbase for parking lots, streets, alleys, curbs, gutters, sidewalks and bike trails, 2) construction for all trench excavation, backfill, bedding, and foundation material for utility installation; and 3) excavation and backfill for building structures and retaining walls.

Article 1.2 Definitions

A. Backfill

Material placed in an excavated area.

B. Bedding

Ground or support in which pipe is laid.

C. Borrow

Material used as fill and/or backfill which is obtained from a source other than required excavation.

D. Compaction

Tamping by hand or machine to achieve required density in soils.

E. Disposal Site

Any area where waste, unsuitable, unusable or surplus material from construction is placed. Contractor provided disposal sites are delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites.

F. Excavation

Area or material removed to provide a suitable base for improvement.
G. Fill

Fill is considered the material placed above the original or natural ground line.

H. Leveling Course

Leveling course is compacted material placed above the subbase and below the finishing surface of the improvement.

I. Non-Frost-Susceptible Material

Non-organic soil containing less than three percent (3%) by weight of grains smaller than .02 mm obtained from minus three inches (-3") material.

J. Service Connection

Any connection from a main line utility or storm drain to a property line for the purpose of providing service to an individual property.

K. Subbase

The subbase is compacted material placed above the subgrade and below the leveling course.

L. Subgrade or Bottom Excavation

The subgrade is material below the bottom of excavation and upon which the subbase material is placed.

M. Trench

Any excavation for a utility or drainage system.

N. Unsuitable or Unusable Material

Unsuitable or unusable material may consist of any material which is, in the opinion of the Engineer, inadequate for use in the proposed construction.

Article 1.3 Applicable Standards

The latest revision of the following standards of the American Society for Testing and Materials (ASTM) and the American Association of State Highway Transportation Officials (AASHTO) are hereby made a part of these specifications:

- ASTM C-29 Test for Unit Weight of Aggregate
- ASTM C-117 Test for Materials Finer than No. 200 Sieve in Aggregates by Washing
- ASTM C-131 Test for Resistance to Abrasion of Small Size Coarse Aggregate by Use of the Los Angeles Machine
Article 1.4 Equipment

All equipment, tools, and machines used in the performance of the Work covered by these Specifications shall be subject to the approval of the Engineer and shall comply with all applicable safety requirements. All equipment used on the project shall be adequately maintained and shall be the proper equipment for the Work being accomplished so as to produce the result required by the Contract Documents.

Article 1.5 Compaction Standards

The required density of fill and backfill shall meet the requirements as outlined in Section 20.21 - Classified Fill and Backfill. In areas outside of road rights-of-way, the density shall be as required by the Contract Documents or as directed by the Engineer.

Where compaction density is specified, the maximum density shall be determined in accordance with the current requirements of AASHTO Standard Method T-180-D.

The Diameter of the test mold in AASHTO T-180 Method D limits the size of particles which may be included in the test to that passing the three-quarter inch (3/4") sieve. In those instances where the particles are retained on the three-quarter inch (3/4") sieve, a correction must be applied to the standard laboratory density prior to calculating the percent compaction. To expedite field result the plus three-quarter inch (3/4") material may be sieved wet and the weight computed as a percent of the total weight of the material from the hole. The corrected laboratory density shall be computed in each instance by the formula:

\[
\text{Corrected Laboratory Density} = \frac{\text{Field Density}}{1 - \frac{\text{Plus 3/4"}}{\text{Total Weight}}}
\]
Corrected Lab Density = \( \frac{62.4}{A} + \frac{62.4(B)}{r D} \)

Where:

- \( A \) = Percent by weight of original material retained on the 3/4-inch sieve, expressed as a decimal.
- \( B \) = Percent by weight of original material passing the 3/4-inch sieve, expressed as a decimal.
- \( C \) = Specific gravity of +3/4-inch material (apparent specific gravity) as determined by AASHTO T-85.
- \( D \) = Uncorrected laboratory density (minimum 3/4-inch material).
- \( r \) = Coefficient with value depending on \( A \), as follows:

  For \( A = 0.18 \) or less, \( r = 1.00 \)
  For \( A = 0.19 \) or more, \( r = 1.036 - 0.2A \)

Backfill under traffic and building structures and trench backfill in the public rights-of-way from six inches (6") over the top of the pipe to the surface shall be compacted to ninety-five percent (95%) of maximum density, unless otherwise noted and approved by the Engineer.

The backfill material shall be placed in horizontal lifts not exceeding twelve inches (12") in thickness and compacted. Any excavations improperly filled shall be reopened to the depth required for proper compaction, then refilled and compacted at the Contractor's expense. The use of water in excess of the quantity required to obtain specified density (optimum moisture content) to settle or compact the backfill will not be permitted.

**Article 1.6 Subsurface Investigation**

Information pertaining to subsurface exploration, borings, test pit locations, and other preliminary investigation may appear in the Bidding Documents or be available at selected locations for review by the Bidder. This information was acquired for design purposes only and is not considered adequate for construction.

The soils classifications and geotechnical designations recorded are informational only and represent only those subsurface conditions on the particular date, at the specific location, as indicated on each soils log and on the plans. The ground water levels indicated on the test hole logs and shown on the Drawings were recorded at the time the test holes were performed. These water levels may vary seasonally and are shown for design and informational purposes only. Contractor shall assume responsibility for any conclusions that may be drawn from such information and the conclusions shall not be considered just cause for a claim for additional compensation or contract time extension.
should obtain and analyze such additional information as the Contractor may feel necessary and shall be responsible for any conclusions drawn from that information.

The Owner does not warrant the correctness of the soils investigation or of any interpretation, deduction, or conclusion given in the report relative to subsurface conditions. The Bidder shall make his own deductions and conclusions as to the nature of the materials to be excavated, the difficulties of making and maintaining the required excavations, the difficulties which may arise from subsurface conditions, and of doing any other Work affected by the subsurface conditions, and shall accept full responsibility therefore.

**Article 1.7 Weather Limitations**

Unless otherwise authorized by the Engineer, fill and backfill material, base course, and leveling course shall not be placed when the atmospheric temperature is below thirty-five degrees Fahrenheit (35°F). When the temperature falls below thirty-five degrees Fahrenheit (35°F), it shall be the responsibility of the Contractor to protect all areas of completed Work against any detrimental effects. Any areas of Work not completed in accordance with the Contract Documents that are damaged by weather shall be reconditioned, reshaped, and recompacted by the Contractor in conformance with the requirements of the Contract Document without additional cost to the Owner.

**Article 1.8 Underground Utilities**

The Contractor shall continuously support underground utilities during backfill placement and compaction. During backfill placement and compaction, the Contractor shall place geotextile fabric with a minimum twelve inch (12”) separation from underground utilities, unless directed otherwise by the Engineer.

**Article 1.9 Contaminated Material**

Unless otherwise noted in the Contract Documents, the Owner is not aware of any contaminated material within the project limits. If such material is encountered, Contractor shall notify the Engineer immediately for direction. Unless the contamination was caused by Contractor’s operation, discovery of contaminated material will be treated as a changed condition per Division 10, Section 10.05, Article 5.18 – Changed Conditions.
SECTION 20.02  STORM WATER POLLUTION PREVENTION PLAN

Article 2.1  General

The Work described in this Section shall consist of providing all labor, equipment, materials, and services to prepare, implement, and maintain a Storm Water Pollution Prevention Plan (SWPPP) for projects that may adversely impact receiving waters or waters of the United States. The type of plan required depends on the area disturbed by the project including the construction site and off-site activities which include, but may not be limited to, material sites, waste disposal sites, borrow and fill sites, and equipment and material storage areas.

For Projects that impact an area greater than five hundred (500) square feet or are deeper than four feet (4’), and less than ten thousand (10,000) square feet, a Type 1 SWPPP is required and the cost of the SWPPP is considered incidental to the Contract and no separate payment shall be made. A Type 2 SWPPP is required for Projects that disturb a project area between ten thousand (10,000) square feet and one (1) acre. A Type 3 SWPPP is required for all Projects that disturb one or more acres of land.

Article 2.2  Definitions and Web References

A.  Area of Land Disturbance

The calculation of the acres of land (soil) that will be disturbed by any construction activity including clearing, grading, and excavating. Material and waste disposal sites must either have their own eNOI and SWPPP, or their disturbed land area must be included in the Project. Pavement area is not included if aggregate underlying the pavement is not removed.

B.  BMPs (Best Management Practices)

BMPs are schedules of activities, prohibition of practices, maintenance procedures and other management practices. They include temporary or permanent structural and non-structural devices. BMPs, when used alone or in combination, minimize erosion and contain sediment within the project site, and prevent discharge of pollutants to water bodies and wetlands. Pollutant is defined in 40 CFR 122.2 (a partial listing of this definition includes solid waste, garbage, chemical wastes, dredged spoil, rock, sand).

C.  CGP (Construction General Permit)

The Storm Water Construction General Permit for Discharges from Large and Small Construction Activities, issued by the Environmental Protection Agency (EPA) under the National Pollutant Discharge Elimination System (NPDES).
D. **eNOI**
   
   Electronic Notice of Intent to begin construction activities under the Construction General Permit.

E. **eNOT**
   
   Electronic Notice of Termination to end coverage under the Construction General Permit.

F. **Final Stabilization**
   
   Final stabilization occurs when soil disturbing activities at the site have been completed and the following two criteria are met: (a) establish a uniform and evenly distributed perennial vegetative cover with a density of seventy percent (70%) of the native background vegetative cover for the area, on unpaved areas that are not covered by non-erodible permanent stabilization, and (b) equivalent non-erodible permanent stabilization measures have been constructed (such as riprap, gabions, geotextiles, and crushed aggregate base course) where vegetative cover is not required.

G. **HMCP (Hazardous Material Control Plan)**
   
   The Contractor's detailed plan for prevention of pollution that stems from the storage, use, containment, cleanup, and disposal of hazardous material, including oil products related to construction activities and equipment.

H. **SPCC Plan (Spill Prevention, Control and Countermeasure)**
   
   The Contractor’s detailed plan for oil spill prevention and control measures that meets the requirements of 40 CFR 112.

I. **SWPPP (Storm Water Pollution Prevention Plan)**
   
   The Contractor’s detailed plan and record of activities, to prevent pollution and minimize erosion, and to contain sediment before it leaves the project site or enters waterways or wetlands.

   1. **SWPPP Amendment.** A document that adds to, deletes from, or changes the SWPPP.
   
   2. **SWPPP Manager.** The Contractor’s representative in the field who supervises implementation of the SWPPP.
   
   3. **SWPPP Preparer.** The person who prepares the initial SWPPP.
   
   4. **SWPPP Template.** A template provided by the EPA for a uniform SWPPP format.
5. **Type 1 SWPPP.** A project-specific SWPPP developed by the Contractor in accordance with the Municipality’s Storm Water Treatment Plan Review Guidance (SWTPRG) Manual that illustrates measures to minimize erosion and control sediment on a project. A completed and signed checklist, available in the SWTPRG Manual, is the basis for a Type 1 SWPPP.

6. **Type 2 SWPPP.** A SWPPP developed by the Contractor in accordance with the Municipality’s Storm Water Treatment Plan Review Guidance Manual and based on the CGP SWPPP requirements. It is similar to a Type 3 SWPPP except that endangered species need not be documented and an eNOI is not required.

7. **Type 3 SWPPP.** A SWPPP developed by the Contractor in accordance with the EPA’s CGP and fully meets the requirements of that permit.

J. **Temporary Stabilization**

Temporary stabilization measures are BMPs that protect disturbed land, material stockpiles, material sources, and waste disposal sites; until the next land disturbance, grading, material movement, or final stabilization occurs.

K. **Internet References**

The following websites about erosion, sediment and pollution control are referenced in this document or are considered good resources for information:

1. The EPA’s “Developing your SWPPP, A Guide for Construction Sites,” which includes a SWPPP template, is at www.epa.gov/npdes/swpppguide


3. The International Erosion Control Association website is at www.ieca.org

4. The Construction Industry Compliance Assistance Center website is at www.cicacenter.org

5. The Alaska SWPPP Guide, including template and inspection form is at www.dot.state.ak.us/stwddes/dcpspubs/otherpubs.shtml

6. The EPA Construction General Permit, is at www.epa.gov/npdes/stormwater/cgp

7. The EPA eNOI Form 3510-9 is at www.epa.gov/npdes/stormwater/enoi

8. The status of EPA eNOI is at www.epa.gov/npdes/noisearch

Article 2.3 Plan and Permit Submittals

A. Hazardous Material Control Plan (HMCP) and Spill Prevention, Control and Countermeasure Plan (SPCC)

Submit two signed copies of the HMCP to the Engineer for approval. Submit one signed copy of the SPCC Plan (if required under Section 20.02, Article 2.5, SubArticle C - Hazardous Material Control Plan (HMCP) Requirements) to the Engineer. Deliver these documents to the Engineer no less than fourteen (14) calendar days before the Preconstruction Conference.

The Engineer will review the HMCP submittals within fourteen (14) calendar days. HMCP Submittals will be returned to the Contractor, and marked as either requiring modification or as approved by the Engineer. The Engineer will keep the SPCC Plan as a record document, and reserves the right to review it and require modifications.

B. All Storm Water Pollution Prevention Plans

Submit two signed copies of your SWPPP to the Engineer for approval. Deliver these documents to the Engineer no less than fourteen (14) calendar days before the preconstruction conference.

The Engineer will review the SWPPP submittals within fourteen (14) calendar days. Submittals will be returned to the Contractor, and marked as either requiring modification or as approved by the Engineer.

C. Type 3 Storm Water Pollution Prevention Plan Requirements

Sign and certify the approved SWPPP according to the Construction General Permit requirements, Appendix G, before submitting your eNOI. The SWPPP must also be signed and certified by the Engineer.

Submit your eNOI to EPA and submit a copy of your eNOI to the Engineer at the same time. The Engineer will submit the Municipality’s eNOI to the EPA. Allow adequate time to receive coverage for State and Federal processing of both eNOIs, before beginning construction activities.

D. Additional Type 3 SWPPP submittal requirements for projects that disturb five acres or more of land.

Submit a copy of the Engineer-approved, signed and certified SWPPP, and copies of your eNOI and the Municipality’s eNOI, with the required permit fee to the Alaska Department of Environmental Conservation (ADEC) Storm Water Coordinator. Transmit a copy of the ADEC acceptance letter to the Engineer.
E. Basis of Work.

The active status eNOIs, Engineer-approved SWPPP, Engineer-approved HMCP, submitted SPCC Plan (when required), and EPA Construction General Permit are the basis of the Work required for the Project’s erosion, sediment, and pollution control.

F. Ending Permit Coverage for Type 3 SWPPPs.

Within thirty (30) days of when the Project is stabilized as determined by the Engineer, submit your eNOT to EPA and send a copy of the eNOT to the Engineer. Upon receipt of your eNOT, the Municipality will submit the Municipality’s eNOT to the EPA, and will terminate the Construction General Permit coverage.

Article 2.4 Personnel Qualifications and Authority

A. The SWPPP Preparer must meet the following qualifications:
   • a current certification as a Certified Erosion and Sediment Control Lead (CESCL), or as a Certified Professional in Erosion and Sediment Control (CPESC)
   • two years experience in erosion and sediment control

B. SWPPP Manager:

1. The SWPPP Manager must meet the following qualifications:
   a. current certification as a Certified Erosion and Sediment Control Lead (CESCL), or as a Certified Inspector of Sediment and Erosion control (CISEC), or as a Certified Professional in Erosion and Sediment Control (CPESC)
   b. two years experience in earthwork construction
   c. a duly authorized representative, as defined in the Construction General Permit, Appendix G
   d. knowledgeable in the principles and practices of erosion and sediment controls
   e. in possession of the skills to assess conditions at the construction site that could impact storm water quality, and to assess the effectiveness of any pollution, sediment and erosion control measures selected

2. Responsibilities:

   The SWPPP Manager must be knowledgeable in the requirements of this Section, the SWPPP, the Best Management Practices, and, for Type 2 and
Type 3 SWPPPs, the Construction General Permit. The SWPPP Manager shall be responsible for and oversee the installation, maintenance and removal of erosion and pollution control BMPs, as detailed in the SWPPP or as directed by the Engineer.

The primary job of the SWPPP Manager shall be Work related to this Section. The SWPPP manager shall be on-site whenever land is being disturbed, moved, stockpiled, or disposed and shall inspect BMPs and update the SWPPP.

The SWPPP Manager must have the Contractor’s authority to order immediate corrective action.

Article 2.5 Control and Prevention Plan Contents

A. Type 1 Storm Water Pollution Prevention Plan (SWPPP) Requirements

Use a SWPPP Preparer to visit the Project site and then develop a project specific Storm Water Pollution Prevention Plan, using the checklist in the Municipality's Storm Water Treatment Plan Review Guidance Manual.

Within the SWPPP include copies of:

• The HMCP
• A reference to SPCC Plan location (if required)
• Contractor signed certification of SWPPP

B. Type 2 and 3 SWPPP Requirements

Follow the format of the SWPPP template presented in the EPA’s Developing your SWPPP, A Guide for Construction Sites. Incorporate into the SWPPP the requirements of the project permits and the Contractor’s progress schedule, equipment, and preferred BMPs. The Project site includes the construction site, material sites, waste disposal sites, haul roads, and other affected areas whether public or private.

The SWPPP must address preventing pollution, minimizing erosion, and containing sediment before it leaves the project site, or enters waterways or wetlands. Identify specific areas where pollution or erosion may occur, and describe BMPs including site-specific controls and procedures. Establish a record of land disturbance, and the installation, maintenance, and removal of BMPs. Describe temporary and permanent stabilization measures.

The SWPPP must address the activities of subcontractors, and of utility companies performing Work in the Project area. The SWPPP must describe the roles and responsibilities of the Contractor, subcontractors, utility companies and the Municipality with regard to implementation of the SWPPP.
All material and waste disposal sites must be included in the Project SWPPP, except when they have their own eNOI and SWPPP. Commercial material sites and waste sites that sell to both public and private purchasers usually have their own eNOI and SWPPP.

Specify the line of authority and designate a SWPPP Manager for implementing SWPPP compliance. Designate one representative for each subcontractor who performs land disturbing activities, or who installs and maintains erosion and sediment control measures.

Once construction begins, the SWPPP must be updated with records of land disturbance and erosion and sediment control activity.

Within the SWPPP include copies of:

- The HMCP
- A reference to SPCC Plan location (if required)
- Inspection reports, record updates and amendments required during construction
- Contractor signed certification of SWPPP

In addition, within a Type 3 SWPPP include:

- Contractor’s eNOI
- Municipality’s eNOI
- eNOIs submitted by other on-site operators
- EPA acknowledgement of receipt of each eNOI
- Contractor delegation of signature authority
- Municipality’s delegation of signature authority
- Municipality’s signed certification of SWPPP

C. Hazardous Material Control Plan (HMCP) Requirements

Prepare a HMCP for prevention of pollution that stems from the storage, use, containment, cleanup, and disposal of hazardous material, including oil products related to construction activities and equipment. (See 40 CFR 117 and 302 for listing of hazardous materials.) Collate Material Safety Data Sheets in one location and reference location in HMCP.

List the types and quantities of equipment and cleanup materials available on site. Include a list and location map of cleanup materials, at each different work site and readily available off site (main site, material site, batch plant, storage yard, explosives dump, equipment or fueling yard, etc).
Specify the line of authority and designate a field representative for spill response, and one representative for each subcontractor.

List and give the location of hazardous materials, including office materials, to be used or stored on site, and estimated quantities. Store hazardous materials in covered storage areas.

Detail methods of disposing of waste petroleum products and other hazardous materials generated by the project.

Identify the locations where storage, fueling and maintenance activities will take place, describe the maintenance activities, and list controls to prevent the accidental spillage of oil, petroleum products and other hazardous materials.

Detail procedures for containment and cleanup of hazardous substances. Detail a plan for the prevention, containment, cleanup, and disposal of soil and water contaminated by accidental spills. Detail a plan for dealing with unexpected contaminated soil and water encountered during construction.

D. Spill Prevention, Control and Countermeasure (SPCC) Plan Requirements.

If the Project is subject to 40 CFR 112, then add a reference to the SPCC Plan in the SWPPP.

You may self-certify the SPCC Plan if total above ground oil storage capacity is 10,000 gallons or less, and you meet all the requirements for self-certification in 40 CFR 112. Otherwise the SPCC Plan must be certified by, stamped with the seal of, dated by, and signed by a Professional Engineer registered in the State of Alaska.

1. Prepare and implement a SPCC Plan when required by 40 CFR 112, including:
   a. When oil spills may reach navigable waters; and
   b. Total above ground oil storage capacity is greater than 1,320 gallons (including vehicle and equipment fuel tanks, but not counting containers that have a capacity less than 55 gallons).

2. Comply with 40 CFR 112 and address the following issues in your SPCC Plan:
   a. Operating procedures that prevent oil spills;
   b. Control measures installed to prevent a spill from reaching navigable waters; and
   c. Countermeasures to contain, clean up, and mitigate the effects of an oil spill.
Article 2.6  Materials

Use materials approved by the Engineer.

Straw must be certified as free of noxious weed by the United States Department of Agriculture, Natural Resources Conservation Service, Local Soil and Water Conservative District, Alaska Weed Free Forage Certification Program.

Silt fence must conform to the Storm Water Treatment Plan Review Guidance Manual or as approved by the Engineer.

Construct rock check dams with clean, well-graded stone that conforms to Sections 20.18 – Drain/Filter Rock and 20.23 – Cobbles or as approved by the Engineer.

Temporary seed must consist of annual rye grass or a specified permanent seeding mixture.

Article 2.7  Construction

A.  Prior to Construction

1.  Projects with less than one acre of land disturbing activity: Contractor shall not begin construction activity until authorized by the Engineer.

2.  Projects that disturb one acre or more of land: Contractor shall not begin construction activity until the EPA has listed both your eNOI and the Municipality’s eNOI as active status, and you are authorized by the Engineer. The EPA will post the status of the eNOIs on the EPA website.

3.  Post notices at publicly accessible locations near the active part of the Project. The posting must be protected from the weather, and located where the public can read it easily without obstructing construction activities (for example, at an existing pullout). Post notices near the beginning and end of the Project, and the project office, that include the following information:
   • Copy of eNOIs in effect;
   • Name and phone number of SWPPP Manager; and
   • Location of a SWPPP available for public viewing.

4.  Keep a copy of the updated SWPPP, HMCP and SPCC Plan at the Project site or locally available.

5.  Install an outdoor rain gauge and thermometer on the project site in a readily accessible location.

6.  The SWPPP Manager shall ensure that subcontractors and utility companies, understand and comply with the SWPPP, and avoid disturbing installed BMPs.
B. During Construction.

Comply with requirements of the approved HMCP, the submitted SPCC Plan, and all State and Federal regulations that pertain to the handling, storage, cleanup, and disposal of oil products or other hazardous substances. Contain, clean up, and dispose of discharges of oil and other hazardous materials. Perform fueling operations in a safe and environmentally responsible manner. Comply with the requirements of 18 AAC 75 and AS 46, Oil and Hazardous Substances Pollution Control. Report oil spills as required by federal, state and local law, and as described in the HMCP and SPCC Plan.

If storm water discharges threaten water quality, take immediate suitable action to preclude erosion and pollution.

BMPs that have been damaged or undercut, shall be repaired or replaced. If maintenance or modifications to existing BMPs are necessary following a storm or inspection, complete implementation as soon as possible and before the next storm event whenever practicable.

Maintain BMPs so they properly perform their function. Remove accumulated sediment and debris before the BMP loses fifty percent (50%) of its storage capacity, except silt fence shall be cleaned before it loses thirty percent (30%) of its storage capacity.

Maintain temporary and permanent erosion and sediment control measures in effective operating condition. Coordinate BMPs with subcontractors and utility companies doing Work in the Project area.

If you fail to install and maintain effective BMPs then payment may be withheld according to Division 10, Section 10.07, Article 7.5 – Progress Payments.

Additional requirements for projects with Type 3 SWPPPs:

• Keep the official updated SWPPP at the construction site.

• Comply with the requirements of the Construction General Permit, implement temporary and permanent erosion and sediment control measures identified in the SWPPP, and ensure that the SWPPP remains current.

• Report noncompliance which may endanger health or the environment to the EPA. Information must be provided orally within twenty-four (24) hours from the time the Contractor becomes aware of the circumstances. Submit written notice as described in the standard permit conditions #12 of the CGP.
C. Stabilization.

Land may be disturbed multiple times during a project. Stabilize disturbed areas as soon as practicable but no later than fourteen (14) days after each cessation of land-disturbing activities. Stabilization may be accomplished using temporary or permanent measures.

Temporary measures of stabilization could include a combination of temporary seeding, mulch, stabilizing emulsions, cover of gravel sub-base, cover with tarp or other methods.

Temporary seeding may be done by any application method at a rate of one-half pound per thousand square feet (0.5lb/1000sf). Roughen the surface to be seeded and apply seed. Maintain seeded areas and reseed areas not showing evidence of satisfactory growth. Within twenty-four (24) hours of installing a culvert, temporary seed the disturbed area or within a twenty-five foot (25') radius of the inlet and outlet of the culvert, whichever is greater.

When the land-disturbing activity is permanently ceased, apply permanent seed according to Division 75 – Landscaping.

D. Ending SWPPP Activities and Maintenance.

You may end SWPPP activities when the Engineer has determined:
1. That land disturbing activities authorized by the CGP have ceased;
2. The project site (including material sources, waste disposal sites, etc.) has achieved final stabilization;
3. Storm water discharges from construction activities have ceased; and
4. Temporary BMPs have been removed.

See Article 2.3 – Plan and Permit Submittals for submitting eNOTs and ending CGP coverage.

E. Record Retention for Type 3 SWPPPs

Retain copies of the SWPPP and other records required by the Construction General Permit, for at least three years after the date of eNOT.

Article 2.8 SWPPP Inspections, Reports and Amendments

Perform inspections, prepare inspection reports, and prepare SWPPP amendments in compliance with the project SWPPP and the Construction General Permit. The Contractor's SWPPP is the SWPPP of record that regulatory agencies will examine.

A. Construction Site Inspections. The SWPPP Manager shall conduct joint site inspections with the Engineer and additional personnel named at the following times:
1. Before starting construction with the SWPPP Preparer. Discuss implementation of the SWPPP, and placement dates of BMPs in relation to the progress schedule.

2. At least once per month during construction, with the SWPPP Preparer. Review the BMPs and SWPPP for conformance with the Construction General Permit. This meeting may be combined with other required meetings.

3. At least once every seven (7) days during construction and within twenty-four (24) hours of the end of a storm exceeding one-half inch (1/2") in twenty-four (24) hours (as recorded at the Project site).

4. Before winter shutdown, to ensure that the site has been adequately stabilized and BMP devices are functional.

5. At project completion, to ensure final stabilization of the project.

B. Winter Site Inspections. The SWPPP Manager shall conduct joint site inspections with the Engineer, at least once every month and within twenty-four (24) hours of a storm resulting in rainfall of one-half inch (1/2") or greater in twenty-four (24) hours if the following requirements are met:

1. The entire site is temporarily stabilized; and
2. Runoff is unlikely due to winter conditions (e.g. the site is covered with snow, ice or the ground is frozen).

The Engineer may waive winter monthly inspection requirements until one month before thawing conditions are expected to result in a discharge, if the following requirements are met:

- Below-freezing conditions are anticipated to continue for more than one month;
- Land disturbance activities have been suspended; and
- The beginning and ending dates of the waiver period are documented in the SWPPP.

C. Items to Inspect. Inspect the following items during a construction or winter site inspection:

1. Disturbed areas that have not been finally stabilized
2. Areas used for storage of erodible materials that are exposed to precipitation
3. BMPs
4. Locations where vehicles enter or exit the site
5. Offsite material sources and waste disposal sites
6. Staging and equipment storage areas
7. Petroleum storage, handling and fueling sites
8. Hazardous material storage sites

D. Inspection Reports.

Type 2 SWPPPs: Prepare reports on forms prepared and provided in the SWPPP.

Type 3 SWPPPs: Sign and certify the report according to the Construction General Permit Appendix G. Include reports as an appendix to the SWPPP. If the report identifies incidents of non-compliance with either the SWPPP or the Construction General Permit, then implement corrective action and record when the action was taken.

E. Updating Type 2 or Type 3 SWPPPs.

The SWPPP Manager shall keep the Contractor’s SWPPP up to date at all times. Keep SWPPP amendments in the SWPPP document, with a summary of amendments. Keep inspection reports in the SWPPP document.

1. Record in the SWPPP:
   a. The location, date of installation, date maintenance was performed and the date of removal for all BMPs;
   b. The dates when major grading activities occur (begin and end dates);
   c. The dates when construction activities temporarily or permanently cease on a portion of the site;
   d. The dates when stabilization measures are initiated; and
   e. The rainfall and outside air temperature each day.

2. Prepare SWPPP amendments, and initial and date them, within seven (7) days following an inspection:
   a. whenever there is a change in design, construction, operation, or maintenance that could have a significant effect on the discharge of pollutants in storm water leaving the project site;
   b. if during inspections it is determined that the SWPPP is ineffective in eliminating or significantly minimizing pollutants in storm water discharges from the project site; and
   c. Whenever inspections identify a problem that requires additional or modified BMPs.

F. Submittals for Type 2 and 3 SWPPPs:

1. SWPPP inspection reports within three working days of the inspection.
2. SWPPP amendments within three working days from date they were initialed.

3. Record of updates within seven working days of recording them in the SWPPP.

4. Contractor’s SWPP to Engineer within 30 days of filing NOT.

**Article 2.9   Measurement**

The Work in this Section is measured by lump sum and will consist of all labor, materials, and equipment required to prepare and implement a SWPPP, including all required SWPPP amendments, revisions, inspections, and all other measures necessary to complete the Work.

**Article 2.10   Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following item:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Storm Water Pollution Prevention Plan (Type)</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 20.03 EXPLORATORY TEST PITS

Article 3.1 General

Work under this Section consists of furnishing an excavator, operator, and all related supplies in order to dig and fill exploratory test pits as directed by the Engineer prior to the commencement of construction activities.

Article 3.2 Materials

Contractor shall furnish an excavator capable of excavating to a minimum depth of twelve feet (12').

Article 3.3 Construction

Contractor shall excavate as directed by the Engineer. After inspection of the test pit is complete, Contractor shall backfill test pits with native material and compact them so that the ground is returned to its original condition. If directed by the Engineer, Contractor shall segregate the cast piles to avoid contamination.

Article 3.4 Measurement

Work performed under this Section is measured by the cost per hour for all personnel, equipment, and supplies necessary for completion of said Work. Down time or delays caused by equipment failure is included in the measurement and no additional payment will be made.

Article 3.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment is made under the following item:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Exploratory Excavation</td>
<td>Hour</td>
</tr>
</tbody>
</table>
SECTION 20.04    CLEARING AND GRUBBING

Article 4.1    General

The Work under this Section consists of removing all vegetation, brush, trees, logs, tree
stumps, roots, and root mat to a Contractor-provided disposal site, and the preservation
from damage of all items designated to remain. Limits of clearing and grubbing shall be in
conformance with right-of-way easements, and stipulations, and as shown on the
Drawings, staked by the Contractor, and approved by the Engineer.

Article 4.2    Construction

The Contractor shall do all clearing and grubbing necessary in the construction of
roadways, bike trails, and utilities. Prior to clearing and grubbing, the Contractor shall
stake the clearing limits. Trees, brush, roots, and root mat removed in the clearing, and
grubbing operations shall be hauled to a disposal site provided by the Contractor as
delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites.

Any areas designated to remain shall be protected per Division 75, Section 75.02, Article
2.3 – Construction.

Article 4.3    Measurement

The measurement of clearing and grubbing shall be by the acre or portion thereof as
shown on the Drawings and staked by the Contractor and approved by the Engineer, or
lump sum.

Article 4.4    Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 -
Measurement and Payment, and shall include full payment for all Work described in this
Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Clearing and Grubbing</td>
<td>Acre</td>
</tr>
<tr>
<td>Clearing and Grubbing</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 20.05  CLEARING

Article 5.1  General

The Work under this Section consists of clearing the areas shown on the Drawings, staked by the Contractor, and approved by the Engineer of all logs, trees, brush, and other vegetation, and removal to a Contractor-provided disposal site, and the preservation from damage of all items designated to remain.

Article 5.2  Construction

The Contractor shall perform all clearing necessary within the areas shown on the Drawings and staked by the Contractor. All stumps shall be cut off a maximum of two inches (2") above the ground.

Any areas designated to remain shall be protected per Division 75, Section 75.02, Article 2.3 – Construction.

All material removed in the clearing operation shall be hauled to a disposal site provided by the Contractor as delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites. With prior approval of the Engineer, chipping may be an acceptable alternate to clearing and hauling away of spoils.

A. Clearing for Bike Trails

Overhanging limbs shall be pruned and treated to give a six foot (6’) clear corridor on either side of the centerline with a nine foot (9’) clearance above finished trail. Where filter fabric is specified, the stumps shall be cut flush with existing ground, or fill material may be placed to create a level surface for fabric placement.

B. Clearing for Sidewalks/Curb Ramps

Contractor shall prune and treat overhanging limbs and other vegetation to give full clearance of the sidewalk to a minimum height of ten feet (10’) above and a minimum width of two feet (2’) from the outside edges of the sidewalk, unless otherwise specified on the Drawings or directed by the Engineer.

Article 5.3  Measurement

The measurement of clearing shall be measured by the acre or portions thereof, as shown on the Drawings and staked by the Contractor, or lump sum. Clearing for bike trails, sidewalks, and curb ramps shall be measured by linear feet along the centerline of the improvement.
### Article 5.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Clearing</td>
<td>Acre</td>
</tr>
<tr>
<td>Clearing</td>
<td>Lump Sum</td>
</tr>
<tr>
<td>Clearing for Bike Trail/Sidewalk/Curb Ramp</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 20.06  REMOVAL OF TREES

Article 6.1  General

The Work under this Section consists of the performance of all operations pertaining to the removal and disposal of trees nine and one-half inches (9 1/2”) or greater in diameter measured at a point one foot (1’) above the natural ground. This item will not be a pay item if clearing or clearing and grubbing is included in the Bid Schedule.

Article 6.2  Construction

Trees of the size described above which interfere with construction under this Contract shall be disposed of at a disposal site provided by the Contractor as delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites.

Removal and disposal of all trees less than nine and one-half inches (9 1/2”) in diameter one foot (1’) above the natural ground will be considered an incidental part of the excavation unless either the items clearing and/or clearing and grubbing are included in the Bid Schedule.

Article 6.3  Measurement

Each tree removed in the size range described will be counted.

Article 6.4  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tree Removal</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 20.07  REMOVAL OF SIDEWALK AND CONCRETE APRON

Article 7.1  General

The Work under this Section consists of performing all operations pertaining to the removal and disposal of sidewalks and concrete aprons designated for removal, including wire mesh or steel reinforcement within the concrete sidewalk and apron, in accordance with the limits shown on the Drawings or as directed by the Engineer.

Article 7.2  Construction

Sidewalks or concrete aprons to be removed shall be saw cut or broken at a joint. Broken joints shall be finished, as required by the Engineer, to eliminate jagged edges. The Contractor shall dispose of this material at a Contractor-provided disposal site as delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites.

Article 7.3  Measurement

Sidewalk and concrete apron designated for removal will be measured in square yards regardless of thickness.

Article 7.4  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Sidewalk</td>
<td>Square Yard</td>
</tr>
<tr>
<td>Remove Concrete Apron</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 20.08  REMOVAL OF CURB AND GUTTER

Article 8.1  General

The Work under this Section consists of performing all operations pertaining to the removal and disposal of existing curb and gutter designated for removal, including any wire mesh or steel reinforcement within the curb and gutter, in accordance with the limits shown on the Drawings or as directed by the Engineer.

Article 8.2  Construction

Curb and gutter to be removed shall be saw cut or broken at a joint. Broken joints shall be finished, as required by the Engineer, to eliminate jagged edges. The Contractor shall dispose of removed curb and gutter at a Contractor-provided disposal site as delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites.

Article 8.3  Measurement

Curb and gutter removal designated for removal will be measured in linear feet removed, measured along the face of the curb.

Article 8.4  Basis of Payment

Payment for this item shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Curb and Gutter</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 20.09   REMOVAL OF PAVEMENT

Article 9.1   General

The Work under this Section consists of performance of all operations pertaining to the removal and disposal of existing pavement in accordance with the limits indicated on the Drawings and as directed by the Engineer.

The Contractor will remove existing pavement (parking areas, driveways, etc.) within the right-of-way to a line one foot (1’) back of the proposed improvements during the initial clearing/excavation operations. Further removal will be as directed by the Engineer in order to provide a proper transition between new and existing pavement. The intent is to minimize unnecessary removal of pavement.

The Contractor shall remove all pavement designated for removal, including pavement placed within the gutter pan. Removal of the pavement within the gutter pan shall be considered incidental to the bid item “Remove Existing Pavement” and no separate payment shall be made.

Article 9.2   Construction

Pavement shall be removed by the Contractor in a manner that will produce a straight, uniform edge along the section removed. The method of producing the straight edge shall be by cutting the section with an air chisel, wheel, power-driven saw, or other methods approved by the Engineer.

Contractor shall keep all pavement that is designated for removal free from objectionable material (concrete, steel, etc.) and shall dispose of pavement designated for removal at the Kloep Maintenance Station, 5701 Northwood Street. Contractor shall coordinate exact location and time of delivery with the General Foreman, Maintenance & Operations Department, Street Maintenance Division. If the removed pavement material under this Section contains objectionable material, as identified by the Engineer, then Contractor shall dispose of this material in accordance with Division 10, Section 10.04, Article 4.9 - Disposal Sites.

Article 9.3   Measurement

Pavement removed will be measured by the square yard of pavement designated for removal, regardless of thickness, except that no measurement will be made of pavement less than one inch (1”) thick.
Article 9.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Pavement</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 20.10 EXCAVATION FOR TRAFFIC WAYS

Article 10.1 General

The Work under this Section consists of furnishing all plant, labor, equipment, supplies, and material in performance of all operations pertaining to the excavation of unsuitable and/or surplus material for street, alleys, access roads, parking lots, sidewalks, curbs, gutter, and bike trails.

Additional excavation for roadways may be required when authorized in writing by the Engineer. Contractor shall not be entitled to additional compensation for performing excavation not previously authorized by the Engineer.

Article 10.2 Survey Stakes

The Contractor shall place control stakes on each side of, and beyond the limits of, the proposed excavation. Stakes will be set at grade breaks and on even grades at intervals not to exceed fifty feet (50’), with additional stakes on vertical curves. These shall be marked with the station, offset, and show the cut or fill to centerline or grid design grade.

Article 10.3 Miscellaneous

Public property lying within the right-of-way, such as signs and markers, that interferes with construction shall be removed and reset at the time and place as directed by the Engineer. Any damage by the Contractor shall be repaired or the item replaced in kind at the Contractor's expense.

Contractor shall remove culverts designated for salvage. Contractor shall deliver salvaged culverts to the location specified in the Contract Documents or as directed by the Engineer.

A disposal site for non-salvageable materials shall be provided by the Contractor per Division 10, Section 10.04, Article 4.9 – Disposal Sites

All existing valve boxes, cleanouts, manholes, etc. shall be located and exposed by the Contractor and carefully protected during the course of the Work. The Contractor, in conjunction with the Engineer, shall check all utilities prior to the start of the construction and record their condition. All manholes, catch basins, cleanouts, etc. will be checked for damage resulting from the Contractor's operation prior to final acceptance by the Owner. The Contractor is responsible for restoring all existing utilities to pre-existing conditions, and shall coordinate with the affected utility in having any necessary repairs completed.

All existing utilities requiring adjustment to grade shall be adjusted by the Contractor in accordance with the applicable Standard Details. Payment for such adjustment shall be as specified under the applicable Section of these Specifications.
Article 10.4 Unusable and Usable Excavation

Unusable excavation shall consist of all excavation which is excess or not suitable for classified fill or backfill as determined by the Engineer. When grubbing of the surface organic or root mat is not required elsewhere on the Drawings or Specifications, unusable excavation shall include the surface mat.

Usable excavation shall consist of material from excavation that is designated by the Engineer as suitable for fill or backfill.

If usable soil conditions are encountered at elevations different from those indicated on the Drawings, the Engineer may direct, in writing, that the excavation be altered to elevations either above or below those specified.

Any unauthorized excavation beyond the specified lines, grades, and cross sections shall be filled with classified fill or backfill and compacted without additional cost to the Owner. The Contractor shall control the banks of all excavated areas as necessary to prevent movement of soil in areas supporting existing foundations, slabs, poles or other structures.

Where unusable soils are encountered in the subgrade within the specified depth below finish grade as indicated on the Drawings, the Contractor shall excavate to a depth such that usable soils are uncovered or the depth below finished grade as directed by the Engineer. The excavations shall be uniformly shaped so that classified backfill material can be properly placed and compacted. The area shall be feathered to adjoining areas where usable material is found. Excavated area shall not be backfilled cross sectional elevations and measurements of the area excavated have been taken.

The Contractor shall be responsible for keeping all embankments and excavation well shaped and drained. The subgrade shall be maintained, compacted in cut sections if required, and kept free of leaves, sticks, or other debris.

The Contractor shall perform whatever work necessary to prevent flow and accumulation of surface water or ground water in excavations. Unless otherwise provided in the Special Provisions, all Work associated with pumping or dewatering shall be considered incidental to the Contract and no separate payment shall be made.

Article 10.5 Utilization or Disposal of Excavated Material

Excavated material conforming to the specifications for classified fill and backfill shall be used where practical for fill and backfill as directed by the Engineer. When this material is used, it shall be considered usable excavation. Usable excavation shall be compacted in accordance with Section 20.01, Article 1.5 - Compaction Standards. When not used on the Project site, the material shall be hauled away and treated as unusable excavation. Unless otherwise specified in the Special Provisions, the Contractor will not be required to transport usable excavation from one schedule of a Contract for use in another schedule of the same Contract unless they are continuous or adjacent.
Article 10.6 Excavation

The Contractor shall utilize whatever methods and equipment necessary to excavate to the limits designated by the Drawings and Specifications and authorized by the Engineer, except that no equipment or method may be utilized that because of its action deteriorates the subgrade making additional excavation necessary beyond the limits originally authorized.

Article 10.7 Measurement

The measurement of excavation will not include water or other liquids but will include topsoil, mud, muck, or other similar semi-solid material which cannot be drained or pumped away.

Usable excavation will be measured per cubic yard by cross section or at the option of the Engineer per cubic yard by truck count. Computation of truck volumes will be by actual measurement to arrive at truck loading, adjusted by an appropriate swell factor.

Unusable excavation will be measured per cubic yard by cross section, or per ton; or at the option of the Engineer per cubic yard by truck count. Computation of truck volumes will be by actual measurement to arrive at truck loading, adjusted by an appropriate swell factor.

Cross-section measurement of usable or unusable excavation shall be based on in-place volumes as determined by the average end areas of cross sections.

For all scale measured quantities, the Contractor shall furnish a scale certified by the State of Alaska for weighing excavation at a location agreeable to the Engineer. Weight tickets will be serialized and witnessed at the time of weighing by a Contractor-furnished weighman. The Engineer may at any time verify load weights and weighing process. Tickets shall be presented for each load at time of delivery to the Engineer or his designated representative.
Article 10.8 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for usable excavation includes the costs of subsequent placement and compaction of the excavated material and shall not be paid separately as Classified Fill or Backfill. Payment for unusable excavation includes removal from the project site and disposal in accordance with Section 20.27 – Disposal of Unusable or Surplus Material.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Usable Excavation</td>
<td>Cubic Yard</td>
</tr>
<tr>
<td>Unusable Excavation</td>
<td>Cubic Yard</td>
</tr>
<tr>
<td>Unusable Excavation</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.11  GRADING EXISTING SURFACES

Article 11.1  General

The Work under this Section consists of performing all operations necessary to shape the existing ground prior to placement of the fill or surfacing material.

Article 11.2  Construction

To the extent indicated on the Drawings, and as directed by the Engineer, the Contractor shall grade the existing ground. Material removed from the high areas shall be used to fill the depressions. Where the existing ground has a slope greater than one vertical to four horizontal, the surface of such ground shall be plowed, steeped or broken up in such a manner that graded material will blend with the existing surface.

On trails, the graded material shall be compacted to ninety percent (90%) of the maximum density; for roads, the required compaction shall be ninety-five percent (95%) of the maximum density. Graded material which is excessively wet shall be aerated by means of blade graders, harrows, or other suitable equipment until the moisture content is satisfactory.

When the bid item is "Grading Existing Surfaces," no separate payment will be made for "Usable Excavation."

Article 11.3  Measurement

Measurement for grading shall be per lineal foot along the centerline of the constructed trail or roadway.

Article 11.4  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Grading Existing Surfaces</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 20.12 DEWATERING

Article 12.1 General

The Work under this Section consists of performing all operations pertaining to the dewatering of Work areas or diversion of surface and subsurface water flows for excavation and backfill during construction operations.

Article 12.2 Materials

Contractor shall be responsible for the Dewatering Plan preparation, selection of materials and equipment, mobilization, operation, maintenance, removal of pumping facilities, piping, etc., used in dewatering operations.

Article 12.3 Construction

All construction requirements for design, installation, and operation of dewatering systems shall comply with current safety and environmental regulations.

The Contractor shall submit his Dewatering Plan to the Engineer a minimum of seven (7) days prior to beginning dewatering activities. The Dewatering Plan shall contain copies of all Contractor obtained permits and approvals. Dewatering activities shall not commence until the Engineer has approved the Plan.

Acceptance of Contractor’s Dewatering Plan by the Engineer shall not relieve the Contractor of responsibility for the exercise of reasonable precaution, sound engineering judgment, prudent construction practices, overloading or misuse of existing or new structures, the adequacy and safety of such Works, and potential damage or undermining of existing or completed Work.

Water resulting from Contractor's dewatering effort may not be pumped or otherwise diverted into existing storm drains unless required permits, including, but not limited to, the Alaska Department of Environmental Conservation and Environmental Protection Agency, are obtained by Contractor. Under no circumstances will Contractor be allowed to divert water from the excavation onto roadways. Contractor shall provide disposal site for excess water and shall be responsible for securing all necessary permits and approvals. Contractor shall provide copies of permits and approvals to the Engineer.

The Contractor shall dispose all water from trench dewatering in accordance with Anchorage Municipal Code, Section 15.40, an ADEC-approved dewatering plan, and in a manner approved by the Engineer. All ground water shall be screened to prevent debris from entering creeks, lakes, ponds, wetlands areas and drainage systems. The Contractor shall furnish and install a silt fence at the perimeter of the easement area within the wetlands areas as an incidental item of construction. When dewatering is required during the course of construction, the Contractor shall submit an ADEC-approved dewatering plan prior to any dewatering activity.
**Article 12.4 Measurement**

The method of measurement for dewatering shall be lump sum for all Work.

**Article 12.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Dewatering</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 20.13 TRENCH EXCAVATION AND BACKFILL

Article 13.1 General

The Work under this Section consists of providing all materials and performance of all operations pertaining to items of Work involved in excavation, bedding, backfill, and compaction of trenches. When unsuitable or surplus material is removed from the job site, it will be paid for under the appropriate item. When material is imported, it will be paid for under the appropriate item.

The Contractor is subject to the same utilities check requirements as described under Section 20.10, Article 10.3 - Miscellaneous.

Article 13.2 Trench Excavation and Backfill - Description

This Work shall consist of all excavation and backfill of trenches as specified for pipe installation and all other miscellaneous items as specified in this Section.

Trench limits shall be shown on the Drawings, and staked in the field. Trench width at or below the top of the pipe shall be of a width that will allow compaction equipment to be utilized at the sides of the pipe. Trenches shall be of the necessary width for proper laying of pipe, conduit, or cable and the banks shall be sloped so as to conform to the prevailing safety requirements.

Trench depth shall be excavated not less than six inches (6") below the barrel of the pipe unless otherwise directed by the Engineer. Where maximum trench width is limited, as shown on the Drawings, the Contractor shall provide trench shoring or supports systems as necessary to ensure that the trench width does not exceed the established limits. The Contractor shall erect and maintain continuous trench barricades to prevent access around all excavations left open at the end of the workday. The Contractor shall provide and maintain adequate barricades to insure public safety at all times during the prosecution of the Work. All excavated material shall be stockpiled on geotextile fabric to limit damage to the existing vegetation.

If at any time the Engineer determines that the construction trench section is greater than the pay limits as shown on the Drawings and described herein, the Contractor may be required to implement appropriate construction techniques to reduce the trench section or absorb all costs associated with the greater trench section, including, but not limited to: replacement of pavement, curb and gutter, sidewalk, street amenities, landscaping, disposal of surplus material and furnishing classified backfill. The pay limits as shown on the Drawings and described herein are to limit pay quantities and incidental costs only and are not intended to limit or in any way alter the requirements of Occupational Safety and Health Administration (OSHA) or State of Alaska safety regulations. The Contractor is required to conduct all trenching operations in accordance with current safety standards.

The Contractor shall be responsible for any and all costs resulting from over excavation, including the need for additional backfill beyond the maximum pay limits as shown on the Drawings or described herein. In addition, the Contractor shall be responsible for the repair
or replacement of streets, alleys, driveways, buildings, sidewalks, curb and gutter, drainage patterns, gravel pads, fences, lawns, property corner markers, survey monumentation, street name signs, traffic control signs, light poles, trees, utilities, shrubbery, gardens, retaining walls, utility markers, rockeries, landscaping, or other public or private improvements damaged by the Contractor which are located outside of the horizontal pay limits defined above. The cost of repairing damage or replacing such facilities within the horizontal pay limits shall be included as part of the unit price for the pay item under construction or shall otherwise be considered incidental to the Contract.

Resurfacing of trench excavation and backfill shall conform to the appropriate sections of this Division, Division 40 – Asphalt Surfacing, and the Standard Details, as appropriate.

Article 13.3 Construction

A. Trench Excavation

The Contractor shall perform all excavation of every description and whatever substance encountered including rock and permafrost. Excavation will be to the extent indicated on the Drawings, and as staked in the field. All excavated materials for backfill shall be placed in an orderly manner and placed at a distance from the trench section which conforms to all state and/or federal safety codes.

All excavated organic or other unsuitable backfill materials shall be placed in a similar manner, but shall be kept separate from all excavated sandy, silty, or gravelly material. In addition, excavated materials suitable for bedding, foundation material, Type II or Type III material, shall be stockpiled separate from each other.

Time is of the essence; therefore, the Contractor shall not begin excavation of the trench until all materials, equipment, and personnel are present to complete the Work in the most expedient manner. Not more than four hundred feet (400’) of trench shall be open in advance of pipe or conduit installation unless authorized, in writing, by the Engineer. Unless otherwise indicated in the Drawings and Specifications, all excavation will be open cut.

Where rock or permafrost is encountered, it shall be removed as shown on the Drawings or as directed by the Engineer, and shall be replaced with approved material.

B. Trench Dewatering

Contractor shall protect adjacent utilities and property by trench dewatering and to successfully install the new utility lines. Dewatering shall be performed in accordance with Section 20.12 - Dewatering.

C. Bedding

All pipe shall be placed in Class B or C bedding, as specified.
Bedding materials for the type specified shall conform to the requirements of Section 20.16 – Furnish Bedding Material.

Where Class B or C bedding material is available from trench excavation, the Contractor shall use care to separate it from unsuitable material. Class B or C bedding material shall be placed under and around the pipe in lifts not to exceed twelve inches (12”), and compacted to ninety-five percent (95%) of maximum density. In no case shall bedding material be placed above the spring line of the pipe in a single lift.

Where Class B or C bedding materials are encountered in the trench bottom, the trench shall be accurately graded to provide uniform bearing and support for each section of the pipe for its entire length, except for the portion of the pipe sections where it is necessary to excavate for the bell holes and other type joints and for the proper sealing of the joints. Bell holes and depressions for joints shall be dug after the trench bottom has been graded and, in order that the pipe will rest on the prepared bottom for as nearly its full length as practical, bell holes and depressions shall be only of such length, depth, and width as required for properly making the particular type of joint. Where unsuitable material such as, but not limited to hard pan or rock is encountered, the trench shall be over-excavated so a minimum of six inch (6”) depth of bedding material is required to bring the trench bottom up to the specified grade. This bedding material shall be compacted to a minimum of ninety-five percent (95%) of maximum density prior to the installation of the pipe. If the Engineer determines that excavated material is unsuitable for bedding, he may direct the Contractor to "Furnish Bedding Material."

D. Trench Backfill

Trench backfill is defined as the placement of material above the level of bedding material. Material for backfill shall be obtained from trench excavation if the material is suitable or conforms to the specifications for backfill. If the Engineer determines that excavated material is unsuitable for trench backfill, he may direct the Contractor to "Furnish Trench Backfill." Backfill shall be placed in lifts and compacted in such a manner that ninety-five percent (95%) of maximum density is obtained unless otherwise specified in the Contract Documents. No separate payment will be made for compaction to ninety-five percent (95%) of maximum density. Where mechanical compaction is required, compaction shall be accomplished in accordance with Section 20.01, Article 1.5 - Compaction Standards. Backfill shall not contain broken bituminous pavement or Portland Cement Concrete, and shall be placed in accordance with Section 20.21 - Classified Fill and Backfill.

E. Locator Tape

Contractor shall provide and install a detectable locator tape properly coded and labeled identifying the utility or utilities installed in the trench. The locator tape shall not be less than five (5) mil, foil backed, and six inch (6") wide vinyl tape. The Contractor shall install the locator tape above and parallel to the axis of the utility
with no breaks in continuity. The Contractor shall install the locator tape three feet (3') below finish grade or two feet (2') deep in the street structural section. Installation of the locator tape is considered incidental to Trench Excavation and no separate payment shall be made.

F. Cleanup

This item consists of cleanup and finishing of all construction areas to their original condition or better. All Work shall be in accordance with Division 10, Section 10.05, Article 5.25 - Final Trimming of Work.

G. Insulation

Refer to Standard Detail XX for insulation installation.

Article 13.4 Measurement

Measurement of trench excavation and backfill will be per linear foot of horizontal distance for the various depths as set forth in the Bid Schedule. On sanitary sewer and storm drain construction, measurement will be from center to center of manholes, from center of manhole to center of catch basins, from center of manhole to center of cleanout wye, from center of manhole to end of out-fall piping. On all other construction, measurement will be from station to station as shown on the Drawings. Trench depth shall be measured from original ground to the bottom of bedding along centerline of pipe. If the trench excavation is performed under the same Contract with a roadway project, the depth of trench shall be measured from the bottom of bedding to the subgrade as it exists after the excavation necessary under the roadway project is complete.

If trench excavation is performed under the same Contract with a roadway project, the depth of trench shall be measured from the bottom of bedding to the subgrade as it exists after the excavation necessary under the roadway project is complete.

When rock or permafrost is encountered for the full depth of trench, it will be measured as stated above. When the rock or permafrost is encountered in the lower part of the trench only, measurement will be by the cubic yard of material excavated. The material overlying the rock or permafrost will also be paid by the cubic yard of material excavated.

Locator tape is incidental to this Bid Item.
Article 13.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Trench Excavation and Backfill (various depths)</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Trench Excavation and Backfill (various depths)</td>
<td>Cubic Yard</td>
</tr>
<tr>
<td>Rock Excavation and Backfill</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Rock Excavation and Backfill</td>
<td>Cubic Yard</td>
</tr>
<tr>
<td>Permafrost Excavation &amp; Backfill</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Permafrost Excavation &amp; Backfill</td>
<td>Cubic Yard</td>
</tr>
<tr>
<td>Trench Dewatering</td>
<td>Lump Sum</td>
</tr>
<tr>
<td>Insulation (R-Value)</td>
<td>Square Foot</td>
</tr>
</tbody>
</table>
SECTION 20.14  TRENCH EXCAVATION, BACKFILL AND COMPACTION FOR SERVICE CONNECTIONS

Article 14.1  General

The Work under this Section consists of performing all operations necessary for excavation, backfill, and compaction required for service connections and all other miscellaneous items as specified in this Section.

Article 14.2  Construction

A.  Excavation

Excavation for service connections shall be unclassified and the Contractor shall excavate whatever substances that are encountered to the depth required for the connections. However, if rock or permafrost is encountered in the trench section different from what is shown on the Drawings, measurement and payment will be as delineated in Section 20.13, Articles 13.4 - Measurement and 13.5 - Basis of Payment.

Depth for service connections shall be as required by the utility. Variations in required depth will not be grounds for additional payment. It shall be the Contractor's responsibility to familiarize himself with the depth of the main line utilities and storm drain systems for the project. The Contractor shall excavate for service connections in such a manner that the excavation is ninety (90) degrees to the street line, whenever possible. The ditch shall be long enough to allow the service connection to be stubbed at the property line.

Trenches shall be of sufficient width at the bottom to allow for laying of the particular service (minimum two and one-half feet [2-1/2'] for single service). Excavation of all fill materials to virgin ground is required to provide safety for workmen utilizing the trench.

The Contractor shall be responsible for, and shall bear expenses incurred, in the event that a main line utility should be damaged during excavation or backfilling.

It shall be the responsibility of the Contractor during construction to keep all embankments and excavation well shaped and drained. The subgrade shall be maintained, compacted in cut sections if required, and kept free of leaves, sticks, and other debris.

The Contractor shall perform all Work necessary to prevent flow and accumulation of surface water or ground water in trenches. Unless otherwise provided in the Special Provisions, all Work associated with pumping or dewatering shall be considered a responsibility of the Contractor and shall be accomplished at no additional cost to the Owner.
The Contractor shall submit as a part of his proposal the method to be used in the
dewatering of the trench section.

B. Backfill

At such time as the Engineer may direct, but only after the service lines and
appurtenances have been properly completed and inspected, the trenches and
appurtenant structures shall be backfilled. The backfill material, free from clods or
boulders, shall be placed by the Contractor in conformance with the codes and
regulations of the Municipality. Backfill shall be placed and compacted in
conformance with Section 20.13 - Trench Excavation and Backfill.

The material shall be placed and spread uniformly in successive layers not
exceeding twelve inches (12") in loose thickness. The Engineer may approve lifts of
greater thickness provided the equipment and method used will consistently achieve
the specified density. The layers shall be carried up full width from the bottom of the
fill to avoid the necessity of widening the edges after the center has been brought to
grade. Each layer shall be compacted to a minimum of ninety-five percent (95%) of
the maximum density at optimum moisture as determined by the method of testing
noted in Section 20.01, Article 1.5 - Compaction Standards. Reasonable time shall
be provided the Engineer to make field density determinations prior to placement of
successive layers of material.

The maximum dimensions of any particle of the embankment material shall not be
greater than two-thirds (2/3) of the compacted thickness of the layer in which it is
placed. The top six inches (6") of embankment material for streets shall be
Type II-A classified fill and backfill. Oversize material shall be removed. Portions of
any layer in which the embankment material becomes segregated shall be removed
and replaced with satisfactory material or shall be added to and remixed to secure
proper gradation as directed by the Engineer. No separate payment will be made
for any material removed or regraded in areas where material becomes segregated.

The Engineer may permit lifts in excess of twelve inch (12") thickness when fill or
backfill is placed over swampy or saturated ground, or where he is satisfied that the
Contractor's method and equipment will consistently produce the specified density.
No frozen material shall be used for backfill. Backfill shall not be placed in frozen
trench.

C. Notification

The Contractor shall notify the Engineer forty-eight (48) hours before starting
excavation (excluding Saturday, Sunday and holidays) on all service connection
requests which involve twelve (12) or less connections. On connection requested
for subdivisions involving more than twelve (12) connections, one (1) week
notification prior to excavating is required.
Article 14.3 Measurement

Trench excavation, backfill and compaction for service connections shall be measured as completed units. A standard connection will be considered fifty feet (50’). The Contractor will be allowed additional compensation if the distance from the main line utility to the property line exceeds fifty feet (50’). The additional compensation will be per linear foot.

Article 14.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Trench Excavation, Backfill, and Compaction for Service Connections (up to fifty feet [50’])</td>
<td>Each</td>
</tr>
<tr>
<td>Single Service in Trench</td>
<td></td>
</tr>
<tr>
<td>Trench Excavation, Backfill, and Compaction per Linear Foot of Trench (in excess of fifty feet [50’])</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Single Service in Trench</td>
<td></td>
</tr>
</tbody>
</table>
SECTION 20.15 FURNISH TRENCH BACKFILL

**Article 15.1 General**

The work under this Section consists of performing all operations necessary to furnish trench backfill.

**Article 15.2 Construction**

The Engineer shall order in writing the amount and type of backfill material to be transported to the Project site. No payment will be made for backfill material under this item that has not been ordered in writing. Material hauled to the Project site shall meet the requirements for the type specified in Section 20.21 - Classified Fill and Backfill.

**Article 15.3 Measurement**

Trench backfill material furnished to the Project site shall be measured in tons (2000 lbs.) delivered to the Project site. Weights shall be obtained on a scale certified by the State of Alaska. All loads shall be accompanied with a serialized weight ticket witnessed at the time of weighing by a Contractor-furnished weighman. The Engineer may at any time verify load weights and the weighing process. Measurement of delivered material may include moisture up to a maximum of four percent (4.0%) of dry weight of material. When tests by the Engineer indicate that moisture contents in excess of four percent (4.0%) may be occurring consistently, the frequency of testing will be increased as necessary and the results averaged over a period of one week. When the average is greater than four percent (4.0%), the tonnage, as measured over the above period, shall be reduced by the difference. No credit will be due the Contractor when moisture content is less than four percent (4.0%). Testing will be done in accordance with standards provided in this Specification.

**Article 15.4 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall be full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish Trench Backfill (Type)</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.16  FURNISH BEDDING MATERIAL

Article 16.1  General

The Work under this Section consists of performance of all operations pertaining to providing Class B and C bedding material for underground utilities.

Article 16.2  Materials

The coarse aggregate material conforming to the requirements specified below shall have a percentage of wear not to exceed thirty (30) after five hundred (500) revolutions, as determined by the current requirements of ASTM C-131.

A. Class "B" Bedding

Materials furnished by the Contractor for use as "B" bedding classified fill and/or backfill shall be graded within the limitations delineated below:

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>1&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>60-100</td>
</tr>
<tr>
<td>#4</td>
<td>40-85</td>
</tr>
<tr>
<td>#10</td>
<td>25-70</td>
</tr>
<tr>
<td>#40</td>
<td>5-40</td>
</tr>
<tr>
<td>#200</td>
<td>0-6</td>
</tr>
</tbody>
</table>

In addition to the grading limits listed above, the fraction of material passing the #200 sieve shall not be greater than thirty-five percent (35%) of that fraction passing the #4 sieve. The bedding material shall not include mechanically fractured materials.

B. Class "C" Bedding

Materials furnished by the Contractor for use as "C" bedding classified fill and/or backfill shall be graded within the limitations delineated below:
Class “C” Bedding

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>2&quot;</td>
<td>100</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>40-100</td>
</tr>
<tr>
<td>#4</td>
<td>20-75</td>
</tr>
<tr>
<td>#10</td>
<td>12-60</td>
</tr>
<tr>
<td>#40</td>
<td>2-30</td>
</tr>
<tr>
<td>#200</td>
<td>0-6</td>
</tr>
</tbody>
</table>

In addition to the grading limits listed above, the fraction of material passing the #200 sieve shall not be greater than twenty percent (20%) of that fraction passing the #4 sieve. The bedding material shall not include mechanically fractured materials.

C. Class "D" Bedding

Materials furnished by the Contractor for use as "D" bedding classified fill and/or backfill shall be graded within the limitations delineated below:

Class “D” Bedding

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>1&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>90-100</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>50-70</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>20-50</td>
</tr>
<tr>
<td>#4</td>
<td>0-10</td>
</tr>
<tr>
<td>#200</td>
<td>0-1</td>
</tr>
</tbody>
</table>

The bedding material shall not include mechanically fractured materials.

Article 16.3 Construction

Placement of Class B, C, or D bedding shall conform to the requirements of Section 20.13, Article 13.3 - Construction.
Article 16.4 Measurement

Measurement of Class B, Class C, and Class D bedding shall be per ton or per linear foot of bedding material placed in the trench.

Article 16.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Bedding Material (Class)</td>
<td>Ton</td>
</tr>
<tr>
<td>Bedding Material (Class)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 20.17 FURNISH FILTER MATERIAL

Article 17.1 General

This Work under this Section consists of performance of all operations pertaining to providing filter material.

Article 17.2 Materials

Filter material shall be gravel or sand consisting of crushed or naturally-occurring granular material. It shall be free of clay particles and conforming to the gradation requirements below.

The coarse aggregate material conforming to the requirements specified below shall have a percentage of wear not to exceed thirty (30) after five hundred (500) revolutions, as determined by the current requirements of ASTM C-131.

Requirements for Grading of Filter Material

Gradation (% Passing)

<table>
<thead>
<tr>
<th>Sieve</th>
<th>2&quot;</th>
<th>1-1/2&quot;</th>
<th>1&quot;</th>
<th>3/4&quot;</th>
<th>1/2&quot;</th>
<th>3/8&quot;</th>
<th>#4</th>
<th>#16</th>
<th>#50</th>
<th>#100</th>
<th>#200</th>
</tr>
</thead>
<tbody>
<tr>
<td>Type A</td>
<td>100</td>
<td>95-100</td>
<td>45-80</td>
<td>10-30</td>
<td>0-10</td>
<td>0-3</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Type B</td>
<td>100</td>
<td>0-5</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Type C</td>
<td>100</td>
<td>95-100</td>
<td>0-20</td>
<td>0-5</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Type D</td>
<td>100</td>
<td>90-100</td>
<td>50-70</td>
<td>20-50</td>
<td>0-10</td>
<td>0-1</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Foundry sand and other material which may be cementitious or not suitable for water percolation shall not be used.

Article 17.3 Construction

Filter material is defined as the material which is placed below, above, and on each side of a perforated pipe to form a subdrain. Filter material may also be used directly in the trenches without a perforated pipe to form a French drain. Refer to Standard Detail 55-3 for construction of a subdrain.

Article 17.4 Measurement

Measurement of filter material shall be per ton or per linear foot of material placed in the trench.
Article 17.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for placing filter material for French drains is included in Section 20.13 - Trench Excavation and Backfill.

Payment for furnishing and placing filter material for subdrains is included in payment for Division 55, Section 55.03 - Subdrains.

Payment for this item includes furnishing the required type of filter material.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Filter Material (Type)</td>
<td>Ton</td>
</tr>
<tr>
<td>Filter Material (Type)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 20.18 DRAIN/FILTER ROCK

Article 18.1 General

The Work under this Section consists of performing all operations pertaining to furnishing and placing a layer of drain/filter rock as shown on the plans or as directed by the Engineer.

Article 18.2 Materials

Materials furnished by the Contractor for drain/filter rock shall be graded within the limitations delineated below:

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Drain Rock</th>
<th>Filter Rock</th>
</tr>
</thead>
<tbody>
<tr>
<td>8&quot;</td>
<td>100</td>
<td>-</td>
</tr>
<tr>
<td>6&quot;</td>
<td>50-80</td>
<td>100</td>
</tr>
<tr>
<td>4&quot;</td>
<td>25-50</td>
<td>50-80</td>
</tr>
<tr>
<td>3&quot;</td>
<td>0-25</td>
<td>-</td>
</tr>
<tr>
<td>2&quot;</td>
<td>0-10</td>
<td></td>
</tr>
<tr>
<td>1&quot;</td>
<td>-</td>
<td>0-10</td>
</tr>
<tr>
<td>#200</td>
<td>0-1</td>
<td>0-1</td>
</tr>
</tbody>
</table>

Article 18.3 Construction

The drain/filter rock shall be handled, dumped, or spread into place so as to secure a stone mass of the dimensions shown on the Drawings.

Article 18.4 Measurement

Drain/filter rock shall be measured in tons complete and accepted in place.
## Article 18.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Drain Rock</td>
<td>Ton</td>
</tr>
<tr>
<td>Filter Rock</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.19  FURNISH FOUNDATION BACKFILL

Article 19.1  General

The Work under this Section consists of performing all operations necessary for excavation, backfilling, compacting foundation materials and trenches.

Article 19.2  Materials

Foundation material for backfill shall consist of Type II, II-A, III, V, or VI classified backfill as specified in the Contract Documents or by the Engineer.

Article 19.3  Construction

If the trench material at the bottom of bedding does not furnish a suitable foundation, the Contractor shall remove the unsuitable material to whatever depth the Engineer determines and replace with foundation material from borrow. Foundation material shall be placed the full width of trench, in lifts not to exceed twelve inches (12") in thickness and compacted to a minimum of ninety-five percent (95%) of maximum density.

In the event of unauthorized over-excavation, the Contractor shall backfill with foundation material to the proper grade and compact to a minimum of ninety-five percent (95%) of maximum density for the full length of the over-excavated trench, all at no additional expense to the Owner.

Article 19.4  Measurement

Where the Contractor is ordered to remove unsuitable material below grade and replace it with foundation material, the material shall be paid for on a cubic yard or ton basis.

Article 19.5  Basis of Payment

Payment for the Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Foundation Backfill (Type)</td>
<td>Cubic Yard</td>
</tr>
<tr>
<td>Foundation Backfill (Type)</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.20  UNCLASSIFIED FILL AND BACKFILL

Article 20.1  General

The Work under this Section consists of furnishing all plant, labor, equipment, supplies, and material in performance of all operations pertaining to the excavation, stockpiling on site, and placement of Unclassified Fill and Backfill.

Article 20.2  Material

Unclassified Fill and Backfill shall be defined as excavated non-organic material that is determined by the Engineer to be unsuitable for Classified Fill and Backfill and suitable for deposition in non-structural fill zones.

Article 20.3  Construction

Excavated material not conforming to the specifications of Section 20.21 - Classified Fill and Backfill shall be used as Unclassified Fill and Backfill adjacent to the fill-slopes to provide additional slope stability to the fill-slopes. Excess Unclassified Fill and Backfill not used shall be disposed of at a Contractor-furnished disposal site as delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites.

Article 20.4  Measurement

The measurement of excavation will not include water or other liquids, but will include topsoil, mud, muck, or other similar semi-solid material which cannot be drained or pumped away.

Unclassified Fill and Backfill will be measured per cubic yard by cross section.

Cross section measurement of Unclassified Fill and Backfill shall be based on in-place volumes as determined by the average end areas of cross sections.

Article 20.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Unclassified Fill and Backfill</td>
<td>Cubic Yard</td>
</tr>
</tbody>
</table>
SECTION 20.21  CLASSIFIED FILL AND BACKFILL

Article 21.1  General

The Work under this Section consists of performing all operations necessary to furnish, place, and compact classified fill and backfill.

Article 21.2  Material

Classified fill and backfill shall contain no lumps, frozen material, organic matter, or other deleterious matter, and shall be durable and sound. It shall have a plasticity index not greater than six (6) as determined by ASTM D-424 and shall conform to one of the following types as required by the Drawings and Specifications. The coarse aggregate material conforming to the requirements specified below shall have a percentage of wear not to exceed thirty (30) after five hundred (500) revolutions, as determined by the current requirements of ASTM C-131.

The portion of the material retained on a #4 sieve shall be known as coarse aggregate. Both coarse and fine aggregates shall conform to the quality requirements of AASHTO M-147.

A.  Type II

Materials furnished by the Contractor for use as Type II classified fill and/or backfill shall be graded within the limitations delineated below:

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>8&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3&quot;</td>
<td>70-100</td>
</tr>
<tr>
<td>1-1/2&quot;</td>
<td>55-100</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>45-85</td>
</tr>
<tr>
<td>#4</td>
<td>20-60</td>
</tr>
<tr>
<td>#10</td>
<td>12-50</td>
</tr>
<tr>
<td>#40</td>
<td>4-30</td>
</tr>
<tr>
<td>#200</td>
<td>2-6</td>
</tr>
</tbody>
</table>

* In addition to the grading limits listed above, the fraction of material passing the #200 sieve shall not be greater than fifteen percent (15%) of that fraction passing the #4 sieve.
B. Type II-A

Materials furnished by the Contractor for use as Type II-A classified fill and/or backfill shall be graded within the limitations delineated below:

**Type II-A**

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>3&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>50-100</td>
</tr>
<tr>
<td>#4</td>
<td>25-60</td>
</tr>
<tr>
<td>#10</td>
<td>15-50</td>
</tr>
<tr>
<td>#40</td>
<td>4-30</td>
</tr>
<tr>
<td>#200</td>
<td>2-6</td>
</tr>
</tbody>
</table>

* In addition to the grading limits listed above, the fraction of material passing the #200 sieve shall not be greater than twenty percent (20%) of that fraction passing the #4 sieve.

C. Type III

Materials furnished by the Contractor for use as Type III classified fill and/or backfill shall be approved sand or gravel with a maximum of ten percent (10%) passing the #200 sieve.

D. Type IV

Materials furnished by the Contractor for use as Type IV classified fill and/or backfill shall be an approved material consisting of sand or gravel with a maximum of twenty-five percent (25%) passing the #200 sieve.

E. Type V

Materials furnished by the Contractor for use as Type V classified fill and/or backfill shall be graded within the limitations delineated below:
### Type V

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>3&quot;</td>
<td>100</td>
</tr>
<tr>
<td>1 1/2&quot;</td>
<td>60-90</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>40-80</td>
</tr>
<tr>
<td>#4</td>
<td>25-55</td>
</tr>
<tr>
<td>#10</td>
<td>15-45</td>
</tr>
<tr>
<td>#40</td>
<td>4-30</td>
</tr>
<tr>
<td>#200</td>
<td>2-6</td>
</tr>
</tbody>
</table>

* In addition to the grading limits listed above, at least thirty percent (30%) of the coarse aggregate particles shall have one or more mechanically fractured face.

### F. Type VI

Materials furnished by the Contractor for use as Type VI classified fill and/or backfill shall be graded within the limitations delineated below:

### Type VI

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>2&quot;</td>
<td>100</td>
</tr>
<tr>
<td>1 1/2&quot;</td>
<td>65-95</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>50-80</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>30-60</td>
</tr>
<tr>
<td>#4</td>
<td>20-50</td>
</tr>
<tr>
<td>#10</td>
<td>10-30</td>
</tr>
<tr>
<td>#40</td>
<td>5-25</td>
</tr>
<tr>
<td>#200</td>
<td>2-6</td>
</tr>
</tbody>
</table>

* In addition to the grading limits listed above, at least forty percent (40%) of the coarse aggregate particles shall have one or more mechanically fractured face.

### Article 21.3 Construction

The subgrade shall be cleared of all debris and organic material. All depressions or holes below the general area surface level, whether caused by removal of debris or
unacceptable material, or otherwise, shall be backfilled with approved material and compacted to specified density and to a level, uniform surface before the placement of other layers. Embankment shall not be placed on frozen ground, nor on ground having a slope greater than one vertical to four horizontal (slope 1:4).

The specified material shall be constructed at the locations and to the lines and grades indicated on the Drawings. The material shall be placed and spread uniformly in successive layers not exceeding twelve inches (12") in loose thickness. The Engineer may approve lifts of greater thickness provided the equipment and method used will consistently achieve the specified density. The layers shall be carried up full width from the bottom of the fill to avoid the necessity of widening the edges after the center has been brought to grade. Each layer shall be compacted to not less than ninety-five percent (95%) of the maximum density at optimum moisture as determined by the method of testing noted in Section 20.01, Article 1.5 – Compaction Standards. Reasonable time shall be provided the Engineer to make field density determinations prior to placement of successive layers of material.

Blading, rolling, and tamping shall continue until the surface is smooth, free from waves and irregularities, and conforms to elevations shown on the Drawings. If at any time the material is excessively wet, it shall be aerated by means of blade graders, harrows, or other suitable equipment until the moisture content is satisfactory. The surface shall then be compacted and finished as specified above.

Contractor shall submit a processing and blending plan to the Engineer for review and approval prior to utilization of classified fill or backfill from more than one source. The plan must be accompanied by materials analysis reports for each material source and fully describe how the material will be placed and blended to ensure that timely and accurate in-place density testing can be achieved.

The maximum dimensions of any particle of the embankment material shall not be greater than two-thirds (2/3) of the compacted thickness of the layer in which it is placed unless specified elsewhere. The top six inches (6") of embankment material for roads, streets, parking lots, and bike trails, shall be Type II-A classified fill and backfill. Oversize material shall be removed. Portions of any layer in which the embankment material becomes segregated shall be removed and replaced with satisfactory material or shall be added to and remixed to secure proper gradation as directed by the Engineer. No separate payment will be made for any material removed or regraded in areas where material becomes segregated.

The Engineer may permit lifts in excess of twelve inch (12") thickness when classified fill or backfill is placed over swampy or saturated ground, or where he is satisfied that the Contractor’s method and equipment will consistently produce the specified density.

Embankments for bike trail sections will be brought to grade in one (1) single lift for embankments less than eighteen inches (18") to finish grade. Trail embankments over eighteen inches (18") shall be brought to grade in lifts as directed by the Engineer.
Article 21.4 Measurement

Classified fill or backfill material, obtained from borrow pits, will be measured in tons (2000 lbs.) of material delivered and placed in accordance with these Specifications. The measurement may include moisture up to a maximum of four percent (4.0%) of dry weight of the material. When tests by the Engineer indicate that moisture contents in excess of four percent (4.0%) may be occurring consistently, the frequency of testing will be increased as necessary and the results averaged over a period of one week. When this average is greater than four percent (4.0%), the tonnage as measured over the above period, shall be reduced by the difference. No credit will be due the Contractor when moisture content is less than four percent (4.0%). Testing shall be done in accordance with Section 20.01, Article 1.3 – Applicable Standards.

Imported classified fill and backfill will be weighed on a scale certified by the State of Alaska. Weight tickets will be serialized and witnessed at the time of weighing by a Contractor-furnished weighman. The Engineer may at any time verify load weights and the weighing process.

Where excavation of unsuitable material beyond the lines and grades shown on the Drawings is ordered in writing, the measurement of classified backfill will include the material required for replacement. No measurement will be made for quantities placed beyond the lines and grade authorized or for quantities placed outside the limits of required excavation.

The Contractor and the Engineer shall verify daily the quantity of material delivered to the Project site. Weight tickets not presented at time of delivery will require special verification by the Contractor before payment can be made.

Article 21.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for the placement and compaction of usable excavation shall not be paid under this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Classified Fill and Backfill (Type)</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.22    LEVELING COURSE

Article 22.1  General

The Work under this Section consists of performing all operations necessary to complete construction of the leveling course on the prepared subbase.

Article 22.2  Material

The leveling course shall consist of crushed gravel, rock, sand, or other approved material. The aggregate shall be free from lumps, balls of clay, or other objectionable matter, and shall be durable and sound. The portion of the material retained on a No. 4 sieve shall be known as coarse aggregate. Both coarse and fine aggregates shall conform to the quality requirements of AASHTO M-147.

Upon written approval by the Engineer, recycled asphalt concrete pavement (RAP) may be substituted for leveling course, on an inch for inch basis. All RAP shall conform to Division 40, Section 40.08 – Recycled Asphalt Pavement. RAP which has been derived from environmentally contaminated aggregates shall not be accepted.

A.  Coarse Aggregate

The coarse aggregate material conforming to the requirements specified above shall have a percentage of wear not to exceed thirty-five (35) after five hundred (500) revolutions, as determined by the current requirements of ASTM C-131. It shall consist of angular fragments reasonably uniform in density and quality, and reasonably free from thin and elongated pieces, dirt, and other objectionable material. At least fifty percent (50%) of the coarse aggregate particles shall have two or more mechanically fractured faces.

B.  Fine Aggregate

The fine aggregate shall consist of material free of organic or other objectionable matter. The fine aggregate, either naturally combined with the coarse aggregate or separately obtained and mixed therewith, shall be of such character that the composite material will conform to the gradation and other requirements specified.

C.  Gradation

The composite mixture of coarse aggregate and fine aggregate, processed as hereinafter specified, shall conform to the following gradation limits as required by the Drawings:
### Leveling Course

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>1&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>70-100</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>50-80</td>
</tr>
<tr>
<td>#4</td>
<td>35-65</td>
</tr>
<tr>
<td>#8</td>
<td>20-50</td>
</tr>
<tr>
<td>#50</td>
<td>8-28</td>
</tr>
<tr>
<td>#200</td>
<td>*2-6</td>
</tr>
</tbody>
</table>

*In addition to the grading limits stipulated above, fractions passing the #200 sieve shall not be greater than seventy-five percent (75%) of the fractions passing the #50 sieve.

### Article 22.3 Construction

The leveling course shall be placed to the lines, grades, and thicknesses shown on the Drawings and shall consist of the materials hereinbefore specified. The leveling course shall provide a smooth stabilized surface on which to place the pavement.

**A. Preparation of Subbase**

Subbase preparation shall consist of dressing, shaping, wetting, and compacting of the subbase to a minimum density of ninety-five percent (95%) in accordance with Section 20.01, Article 1.5 - Compaction Standards. Surfaces shall be cleaned of all foreign substances and debris. Any ruts or soft yielding spots that may appear in the subbase surface shall be corrected by loosening, removing and adding approved material, reshaping, and recompacting the affected areas to the line, grade, and to the specified density requirements.

**B. Surveying**

Subbase and leveling course control stakes shall be wooden bluetops set to finish subbase. The subbase bluetops will be the reference used by the Contractor to set top of leveling course. Subbase bluetops shall be set at breaks in grade and on even grade at intervals not to exceed fifty feet (50’), with additional stakes at vertical curves. Side control will be from the lip or gutter, or in the case of strip paving, additional bluetops shall be provided.
C. Placing

The approved leveling course material shall be deposited and spread in a uniform layer to the required contour and grades and to such loose depth that when compacted to the density required will achieve the specified thickness. The material shall be spread uniformly on the prepared subbase from moving vehicles or spreading boxes, then leveled to the required contour and graded with blade graders. Portions of the layer which become segregated in spreading shall be remixed to the required gradation.

D. Compacting

The leveling course shall be compacted to a minimum of ninety-five percent (95%) of maximum density. In all places not accessible to the rolling equipment, the mixture shall be compacted with tamping equipment. Blading, rolling and tamping shall continue until the surface is smooth and free from waves and inequalities. If at any time the mixture is excessively moistened by rain, it shall be aerated by means of blade graders, harrows or other approved equipment until the moisture content is such that the surface can be recompacted and finished as above. The finished leveling course shall be maintained by the Contractor in the above condition until the pavement is applied.

E. Smoothness Test

The surface of the leveling course, when finished, shall not show any deviation in excess of three-eighths inch (3/8”) when tested with a ten foot (10’) straightedge applied parallel with, and at right angles to, the centerline of the area to be paved. Any deviation in excess of this amount shall be corrected by loosening, adding, or removing material and reshaping and compacting to satisfy the above requirement.

Contractor shall obtain written approval from the Engineer for the final leveling course grade prior to pavement placement.

Article 22.4 Measurement

The leveling course shall be measured in tons of materials delivered and placed in accordance with these Specifications. The measurement may include moisture up to a maximum of four percent (4.0%) of dry weight of the material. When tests by the Engineer indicate that moisture contents in excess of four percent (4.0%) may be occurring consistently, the frequency of testing will be increased as necessary and the results averaged over a period of one week. When this average is greater than four percent (4.0%), the tonnage as measured over the above period, shall be reduced by the difference. No credit will be due the Contractor when moisture content is less than four percent (4.0%). Testing shall be done in accordance with Section 20.01, Article 1.3 – Applicable Standards.
Article 22.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Leveling Course</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.23 COBBLES

Article 23.1 General

The Work under this Section consists of performing all operations pertaining to furnishing and placing a layer of cobbles as shown on the Drawings or as directed by the Engineer.

Article 23.2 Materials

Materials furnished by the Contractor for cobbles shall be graded within the limitations delineated below:

<table>
<thead>
<tr>
<th>U.S. Std. Sieve</th>
<th>Cumulative % Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>12&quot;</td>
<td>100</td>
</tr>
<tr>
<td>8&quot;</td>
<td>50-80</td>
</tr>
<tr>
<td>6&quot;</td>
<td>25-50</td>
</tr>
<tr>
<td>3&quot;</td>
<td>0-25</td>
</tr>
<tr>
<td>2&quot;</td>
<td>0-10</td>
</tr>
<tr>
<td>#200</td>
<td>0-1</td>
</tr>
</tbody>
</table>

Article 23.3 Construction

The cobbles shall be handled, dumped, or spread into place so as to secure a stone mass of the dimensions shown on the Drawings.

Article 23.4 Measurement

Cobbles shall be measured in tons complete and accepted in place.

Article 23.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Cobbles</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.24  RIPRAP

Article 24.1  General

This work shall consist of furnishing and placing a protective covering of stone as shown on the Drawings or as directed by the Engineer.

Article 24.2  Materials

Stone for this work shall be hard angular quarry stones and have a percentage of wear of not more than fifty (50) at five hundred (500) revolutions as determined by ASTM C-535. The least dimension of any piece of stone shall be not less than one-fourth (1/4) its greatest dimension. Stones shall meet the following gradation requirement for the class specified:

A.  Class I

No more than ten percent (10%) of the stones by total weight shall weigh more than fifty (50) pounds per piece and no more than fifty percent (50%) by total weight of the stones shall weigh less than twenty-five (25) pounds per piece.

B.  Class II

No more than ten percent (10%) of the stones by total weight shall weigh more than four hundred (400) pounds per piece and no more than fifteen percent (15%) by weight of the stones shall weigh less than twenty-five (25) pounds per piece. The stones shall be evenly graded and a minimum of fifty percent (50%) by weight of the stones shall weight two hundred (200) pounds or more per piece.

C.  Class III

No more than ten percent (10%) of the stones by total weight shall weigh more than one thousand four hundred (1,400) pounds per piece and no more than fifteen percent (15%) of the stones shall weigh less than twenty-five (25) pounds per piece. The stones shall be evenly graded and a minimum of fifty (50%) by weight of the stones shall weigh seven hundred (700) pounds or more per piece.

Article 24.3  Construction

A footing trench shall be excavated along the toe of the slope when shown on the plans. The stones shall be handled or dumped into place so as to secure a stone mass of the thickness, height and length shown on the plans, or as staked with a minimum of voids.

Undesirable voids shall be filled in with small stones or spalls. The rock shall be manipulated sufficiently by means of a bulldozer, rock tongs, or other suitable equipment to secure a reasonably regular surface and mass stability.
Riprap protection shall be placed to its full course thickness at one operation and in such manner as to avoid displacing the underlying material. Placing of riprap protection in layers or by dumping into chutes or by similar methods likely to cause segregation will not be permitted.

All material going into riprap protection shall be so placed and distributed that there will be no large accumulation or area composed largely of either the larger or smaller sizes of stone.

Unless otherwise authorized by the Engineer, the riprap protection shall be placed in conjunction with the construction of the embankment with only sufficient lag in construction of the riprap protection as may be necessary to prevent mixture of embankment and riprap material.

The Contractor shall provide a level compact area of sufficient size to dump and sort typical loads of riprap at approved location(s). He shall further dump loads specified in this area and assist the Engineer as needed to sort and measure the stones in the load for the purpose of determining if the riprap is within specifications. Mechanical equipment as needed to assist in this sorting shall be provided by the Contractor at no additional cost to the Owner.

**Article 24.4 Method of Measurement**

Riprap shall be measured in cubic yards measured by neat line measure, or tons, completed and accepted in place. Excavation and backfill required for placement of riprap is considered incidental to the bid item.

**Article 24.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

When more than one class of riprap is specified for any pay item, letter suffixes shall be included within the parentheses of the item numbers in order to differentiate between the different classes.

Payment will be made under the following item:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Riprap (Class)</td>
<td>Cubic Yard</td>
</tr>
<tr>
<td>Riprap (Class)</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 20.25  GEOTEXTILE FABRIC

Article 25.1  Description

The Work under this Section shall consist of furnishing and installing Geotextile Fabric for embankment separation, subgrade reinforcement of roadways, subsurface drainage, or riprap lining in a manner and at locations as shown in the Drawings or as directed by the Engineer.

Article 25.2  Materials

A.  Type A Geotextile

Type A Geotextile shall be used for separation. The Type A Geotextile shall be a woven or nonwoven pervious fabric constructed from long chain polymeric filaments such as polypropylene, polyethylene, polyester, polyvinylidene chloride or polyamide formed into a stable network such that the filaments or yarns retain their relative position to each other. The geotextile shall be inert to commonly encountered chemicals and shall be free from defects.

Non-woven geotextile may be formed by the needle-punched, spun-bonded or melt-bonded process.

Woven geotextile shall be a pervious sheet of yarn woven into a uniform pattern with distinct and measurable openings. Edges of the cloth shall be salvaged to prevent the outer yarn from pulling away from the cloth.

Acceptance of geotextile material is to be determined according to ASTM D-4873.

Geotextile manufacturer shall provide a letter certifying that its geotextile product meets the specified requirements.

Type A Geotextile supplied shall meet the physical and mechanical properties as follows:
**Type A Geotextile**

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>Test Method</th>
<th>Value (1)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile Strength, lbs., min.</td>
<td>ASTM D-4632</td>
<td>180</td>
</tr>
<tr>
<td>Tensile Elongation, %</td>
<td>ASTM D-4632</td>
<td>30</td>
</tr>
<tr>
<td>Burst Strength, psi.</td>
<td>ASTM D-3786</td>
<td>400</td>
</tr>
<tr>
<td>Trapezoid Tear, lbs.</td>
<td>ASTM D-4533</td>
<td>70</td>
</tr>
<tr>
<td>Puncture Strength, lbs.</td>
<td>ASTM D-4833</td>
<td>70</td>
</tr>
<tr>
<td>Permittivity, Sec(^{-1})</td>
<td>ASTM D-4491</td>
<td>0.02</td>
</tr>
<tr>
<td>Ultraviolet Resistance, % (2)</td>
<td>ASTM D-4355</td>
<td>70</td>
</tr>
<tr>
<td>Apparent Opening Size (AOS), US Sieve</td>
<td>ASTM D-4751</td>
<td>50</td>
</tr>
</tbody>
</table>

Notes:

(1) Percent of tensile strength retained per ASTM D-4632 after conditioning of 500 hours in accordance with ASTM D-4355.

(2) Minimum Average Roll Values

---

**B. Type B Geotextile**

Type B Geotextile shall consist of a regular grid structure formed by biaxially drawing a continuous sheet of select polypropylene material; it shall have aperture geometry and rib and junction cross sections sufficient to permit significant mechanical interlock with the material being reinforced.

Type B Geotextile shall have high flexural rigidity and high tensile strength at ribs and junctions of the grid structure.

Type B Geotextile shall maintain its reinforcement and interlock capabilities under repeated dynamic loads while in service and shall also be resistant to ultraviolet degradation, to damage under normal practices, and to all forms of biological or chemical degradation normally encountered in the material being reinforced.

Type B Geotextile shall meet the mechanical and physical properties listed below:
## Type B Geotextile

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>Test Method</th>
<th>Value</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>INTERLOCK</strong></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Aperture size (1)</td>
<td>ID Calipered(2)</td>
<td>1.0 (nom)</td>
</tr>
<tr>
<td>- MD, inch</td>
<td></td>
<td></td>
</tr>
<tr>
<td>- CMD, inch</td>
<td></td>
<td>1.3 (nom)</td>
</tr>
<tr>
<td>Open Area, %</td>
<td>COE Method(3)</td>
<td>70 (min)</td>
</tr>
<tr>
<td>Thickness</td>
<td>ASTM D-1777</td>
<td></td>
</tr>
<tr>
<td>- ribs, inch</td>
<td></td>
<td>0.03 (nom)</td>
</tr>
<tr>
<td>- junctions, inch</td>
<td></td>
<td>0.11 (nom)</td>
</tr>
<tr>
<td>Secant aperture stability modulus @ 20cm-kgs/deg(4)</td>
<td>Grid Aperture Test</td>
<td>3.20</td>
</tr>
</tbody>
</table>

| REINFORCEMENT       |             |               |
| Flexural rigidity   | ASTM D-1388(6) | 250,000 (min) |
| - MD, mg-cm         |             |               |
| - CMD, mg-cm        |             | 270,000 (min) |
| Tensile modulus     | GRI-GG1(7)  |               |
| - MD, pound/foot    |             | 14,000 (min)  |
| - CMD, pound/foot   |             | 20,000 (min)  |
| Junctions           | GRI-GG2(8)  |               |
| - Strength          |             |               |
| - MD, pound/foot    |             | 765 (min)      |
| - CMD, pound/foot   |             | 1,260 (min)    |
| - Efficiency        | GRI-GG2(8)  |               |
| - MD, %             |             | 90 (min)       |
| - CMD, %            |             | 90 (min)       |

| MATERIAL            |             |               |
| Polypropylene, %    | ASTM D-4101 Group 1/Class 1/ Grade 2 | 98 (min) |
| Carbon black, %     | ASTM D-4218 | 0.5 (min)     |
Notes:

(1) MD dimension is along roll length. CMD dimension is across roll width.

(2) Maximum inside dimension in each principal direction measured by calipers.

(3) Percent open area measured without magnification by Corps of Engineers method as specified in CW 02215 Civil Works Construction Guide, November 1977.

(4) Secant aperture stability modulus value listed is equal to the mean value less approximately one standard deviation.

(5) Grid Aperture Stability Test developed by Dr. T. Kinney at the University of Alaska, Fairbanks.

(6) ASTM D-1388 modified to account for wide specimen testing.

(7) Secant modulus at 2% elongation measured by Geosynthetic Research Institute test method GG1 “Geogrid Tensile Strength.” No offset allowances are made in calculating secant modulus.

(8) Geogrid junction strength and junction efficiency measured by Geosynthetic Research Institute test method GG2 “Geogrid Junction Strength.”

Geotextile manufacturer shall provide a letter certifying that its geotextile product meets the specified requirements.

C. Type C Geotextile

Type C Geotextile shall be used for drainage or riprap lining. The geotextile shall be constructed from long chain polymeric filament or yarns such as polypropylene, polyethylene, polyester, nylon, polyvinylidene chloride or polyamide formed into a stable network such that the filaments or yarns retain their relative position to each other. The geotextile shall be inert to commonly encountered chemicals and shall be free from defects.

Non-woven geotextile may be formed by the needle punched, spun-bonded or melt-bonded process.

Woven geotextile shall be a pervious sheet of yarn woven into a uniform pattern with distinct and measurable openings. Edges of the cloth shall be salvaged to prevent the outer yarn from pulling away from the cloth.

The geotextile fabric supplied shall meet the physical and mechanical properties listed below:
Subsurface/Riprap Liner Geotextile

<table>
<thead>
<tr>
<th>Geotextile Property</th>
<th>Test Method</th>
<th>Subsurface Drainage</th>
<th>Riprap Liner Unprotected</th>
<th>Riprap Liner Protected</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile Strength, lbs.</td>
<td>ASTM D-4632</td>
<td>90</td>
<td>200</td>
<td>90</td>
</tr>
<tr>
<td>Elongation, %</td>
<td>N/A</td>
<td>15-70</td>
<td>15-70</td>
<td></td>
</tr>
<tr>
<td>Burst Strength, psi</td>
<td>ASTM D-3786</td>
<td>125</td>
<td>320</td>
<td>140</td>
</tr>
<tr>
<td>Puncture Strength, lbs.</td>
<td>ASTM D-4833</td>
<td>25</td>
<td>80</td>
<td>40</td>
</tr>
<tr>
<td>Trapezoid Tear, lbs.</td>
<td>ASTM D-4533</td>
<td>25</td>
<td>50</td>
<td>30</td>
</tr>
<tr>
<td>Apparent Opening Size</td>
<td>ASTM D-4751</td>
<td>70 min.</td>
<td>(3)</td>
<td>(4)</td>
</tr>
<tr>
<td>Seam Strength, lbs.</td>
<td>N/A</td>
<td>180</td>
<td>80</td>
<td></td>
</tr>
<tr>
<td>Permittivity</td>
<td>ASTM D-4491</td>
<td>50</td>
<td>0.5</td>
<td>0.5</td>
</tr>
<tr>
<td>(gal./min. ft.²)</td>
<td></td>
<td></td>
<td>(sec.⁻¹)</td>
<td>(sec.⁻¹)</td>
</tr>
<tr>
<td>Ultraviolet Degradation (5), %</td>
<td>ASTM D-4355</td>
<td>70</td>
<td>70</td>
<td>70</td>
</tr>
</tbody>
</table>

Notes:

(1) Unprotected Erosion Control applications are those in which fabrics are used under conditions where installation stresses are more severe than Class B, i.e., stone placement height should be less than three feet (3’) and stone weights should not exceed two hundred fifty (250) pounds.

(2) Protected Erosion Control applications are those in which fabrics are used in structures or under conditions in which the fabric is protected by sand cushions or by "zero drop height" placement of stone.

(3) Soil with 50% or less particles by weight passing US No. 200 Sieve.

(4) Soil with more than 50% particles by weight passing US No. 200 Sieve.

(5) Percent of minimum tensile strength (ASTM D-4632) retained after weathering per ASTM D-4533 for 500 hours.

Acceptance of geotextile material shall be determined according to ASTM D-4759.

Geotextile manufacturer shall provide a letter certifying that its geotextile product meets the specified requirements.

D. Submittal Requirements

The following information shall be submitted to the Engineer for review and acceptance:

1. Full-scale laboratory testing and in-ground testing of pavement structures reinforced with the proposed geotextile product which illustrates significant structural contribution of the geotextile product to the pavement structure.
2. Certified test results stating that the geotextile product meets the material and physical properties in all respects.

3. Guidelines to pavement design using proposed geotextile product.

4. A list of not less than ten (10) comparable projects, in terms of size and application, in the United States, with references and phone numbers, where the results of the proposed geotextile product’s use can be verified after a minimum of three years continuous service life.

5. Geotextile product samples and certified material property data sheets.

6. Recommended installation instructions.

Article 25.3 Construction

A. Surface Preparation

Prepare surface by removal of stumps, boulders, and sharp objects in accordance with Section 20.05 - Clearing. Contractor shall fill holes and large ruts with material shown on the Drawings or as approved by the Engineer.

Clearing shall be considered incidental to this item. Material used to fill ruts and holes shall be paid for at the unit price for the type of material used, as shown on the Drawings or as approved by the Engineer.

In Areas to Be Surcharged: All trees and brush having a trunk base diameter greater than one-half inch (1/2") shall be cut to within two inches (2") of original ground surface. Grass shall be flattened with no more than two passes of a tracked vehicle.

B. Geotextile Placement

Unroll geotextile directly onto the prepared surface. Exposure of geotextile to the elements after removal of protective covering shall not exceed five days.

Unroll geotextile for embankment reinforcement parallel to the embankment centerline.

Geotextile shall be placed in daily work sections so the lap adjustment can be made should movement of the geotextile occur during placement of fill.
C. Joining

1. Type A Geotextile

Fabric shall be joined with adjacent pieces of fabric by sewing or overlapping.

If fabric is sewn, the fabric shall have all seams sewn by butterfly or J-seams and shall develop a minimum of eighty-five percent (85%) of the specified strength. Seams shall be sewn with a double-thread chain-lock stitch. High strength polyester, polypropylene or Kevlar thread shall be used. The seam shall be one and one-half inch plus or minus one-quarter inch (1-1/2” ±1/4”) from the outside edge of the geotextile.

2. Type B Geotextile

Sections shall be overlapped a minimum of three feet (3’), or as shown on the Drawings, to prevent shifting of geotextile during installation and filling.

Lap joints shall be tied with plastic ties specifically manufactured for this purpose at five foot (5’) intervals.

3. Type C Geotextile

Fabric shall be joined with adjacent pieces of fabric by sewing or overlapping.

If fabric is sewn, the fabric shall have all seams sewn by butterfly or J-seams and shall develop a minimum of eighty-five percent (85%) of the specified strength. Seams shall be sewn with a double-thread chain-lock stitch. High strength polyester, polypropylene or Kevlar thread shall be used. The seam shall be one and one-half inch plus or minus one-quarter inch (1-1/2” ±1/4”) from the outside edge of the geotextile. If the fabric is overlapped, the sections shall be overlapped a minimum of three feet (3’) or as shown on the Drawings.

D. Material Placing and Spreading

Fill material placement shall not occur until the Engineer accepts surface preparation and geotextile laps.

Contractor shall maintain minimum laps and fabric continuity without fabric loops or kinks during material placement and spreading.

Follow the manufacturer’s recommendations for material placing and spreading of the geotextile. During placing and spreading, the Contractor shall maintain a minimum depth of one foot (1’) of cover material at all times between the fabric and the wheels or tracks of the construction equipment. At no time shall equipment operate on the unprotected geotextile. Construction equipment shall not make
sudden stops, starts, or turns on the over material. Use a smooth drum roller to achieve the specified density.

Spread the material in the direction of the fabric overlap.

On weak subgrades, spread the cover material simultaneously with dumping to minimize the potential of a localized subgrade failure.

E. Geotextile Repair

Should it be determined during or after embankment construction that specified geotextile lap widths have not been achieved, or that the Contractor otherwise damaged the installed geotextile, the Contractor shall correct the geotextile installation at no additional cost to the Municipality.

The Contractor shall expose the geotextile and add additional geotextile extending in all directions to achieve specified laps and anchorage. After correcting the geotextile, the embankment shall be reconstructed in accordance with the Contract Documents.

Article 25.4 Method of Measurement

Geotextile shall be measured in square yards of ground surface covered by fabric as shown on the Drawings or as approved by the Engineer. Overlapping and stitching of fabric will be considered incidental to this pay item and no additional payment will be made.

Material used to fill ruts and holes shall be paid for at the unit price of the appropriate bid item for the type material used, as shown on the Drawings or as approved by the Engineer.

Article 25.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Geotextile (Type)</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 20.26 ROADWAY INSULATION

Article 26.1 General

The Work under this Section consists of performing all operations including labor and material pertaining to the placement of insulation for roadway construction. Contractor shall provide polystyrene insulation board(s), extruded or expanded, in conformance with the Drawings and these Specifications.

The Work under this Section also includes shaping and compacting a level area under the horizontal insulation boards and placing the insulation as indicated on the drawings.

Article 26.2 Materials

The insulation board shall have a minimum full board size of two foot by eight foot (2’ x 8’), have the specified R-Value or better, and shall conform to the requirements of AASHTO M230, except that extrusion is not required. R-Value of insulation shall be based on manufacturer’s warranted R-Value. The insulation board shall be rigid, homogeneous, and conform to the following:

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Value</th>
</tr>
</thead>
<tbody>
<tr>
<td>Compressive Strength psi, minimum at yield or 5 percent strain</td>
<td>ASTM D-1621</td>
<td>60.0</td>
</tr>
<tr>
<td>Water Absorption, maximum percent by volume</td>
<td>ASTM C-272</td>
<td>0.3%</td>
</tr>
<tr>
<td>Thermal Resistance, minimum R-Value at 75ºF, ºF-Ft²-Hr/ BTU</td>
<td>ASTM C-177</td>
<td>As Specified</td>
</tr>
</tbody>
</table>

Article 26.3 Construction

Contractor shall install the insulation board with staggered joints. Layering of insulation to obtain the specified R-Value is allowed as long as joints are overlapped at least one foot (1’). Contractor shall blade, shape, and compact the area prior to placing the insulation board in accordance with this Division. Contractor shall shape the subgrade to the lines and grades shown on the Drawings and provide a smooth surface on which to place the insulation board. Prior to placing the insulation board on the prepared subgrade, the Contractor shall furnish straightedges to the Inspector for checking surface uniformity. Surface irregularities shall not exceed one inch (1”) within eight feet (8’), or three-eighths inch (3/8”) in two feet (2’). Contractor shall uniformly compact the subgrade. Contractor shall hand-rake smooth and recompact the ridges left by the compaction equipment. Contractor shall accurately set the horizontal insulation boards to the line and grade established and in such a manner as to hold the board firmly in place by mechanically connecting it to the subgrade.
Contractor shall replace or repair insulation panels broken, crushed, or cracked, as determined by the Engineer, at no additional cost to the Owner.

Contractor shall cover the insulation board with approved three inch (3”-) minus Classified Fill and Backfill material, placed in a twelve inch (12”) lift, spread, and compacted for the full width of the insulation layer prior to placing subsequent lifts. Contractor shall place, spread, and compact in such a manner as not to damage the insulation board. Engineer will approve spreading and compacting equipment prior to its use.

**Article 26.4 Measurement**

The insulation board is measured per square foot regardless of thickness, complete and accepted in place.

Additional Work required for preparing the subgrade to the smoothness required is incidental to the bid item “Insulation Board” and no separate payment is made.

**Article 26.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Roadway Insulation Board (R-Value)</td>
<td>Square Foot</td>
</tr>
</tbody>
</table>
SECTION 20.27  DISPOSAL OF UNUSABLE OR SURPLUS MATERIAL

Article 27.1  General

The Work under this Section consists of performing all operations pertaining to the disposal of unusable or surplus material encountered in the excavation. This material may include peat, roots, large rocks, unstabilized soil, cesspools, privy pits, or any other material, which in the opinion of the Engineer is objectionable for use as fill or backfill.

Article 27.2  Construction

The Contractor shall use care in separating unusable material from usable material. When unusable material shall be disposed of, the Engineer will order the same in writing, stating the limitations of the Work. Should the trench be, in the opinion of the Engineer, wider than is necessary for the safety of the workmen, a deduction may be made for the excess width. Payment will not be made for disposal of unusable material unless the material is moved in excess of one hundred feet (100’) from the excavation.

All unusable material shall be hauled to a disposal site provided by the Contractor as delineated in Division 10, Section 10.04, Article 4.9 – Disposal Sites.

Article 27.3  Cesspools, Privy Pits and Septic Tanks

If cesspools and privies are encountered in right-of-way areas and have to be removed to allow construction, the following procedures for removal are to be used.

In the case of a privy encountered, the Contractor shall remove the privy from the right-of-way area and set it over onto the private property where the privy belongs.

In the case of septic tanks, cesspools and privy pits, the liquid sewage and sludge from the cesspool or privy pit shall be pumped into a watertight container and disposed of at a designated manhole. Care shall be exercised in transporting cesspool and privy pit liquids and sludge so that spillage does not occur during transportation and disposal.

The Contractor shall then remove the remaining sludge, cesspool and privy pit logs or cribbing, and any saturated gravel remaining in the trench area, and shall dispose of this material at the Municipal Landfills. Disposal of this material will be coordinated with the Engineer, in order that the materials disposed of can be covered with fill material by others at the landfill site immediately after it is dumped. Care shall be exercised in transporting this material so that spillage does not occur during transportation and disposal.

Article 27.4  Measurement

The method of measurement for this item will be per cubic yard measured by truck count or by cross section measurement before and after removal of unusable materials. Unless otherwise noted in the Bid Schedule, measurement will be by truck count.
Article 27.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Disposal of Unusable or Surplus Material</td>
<td>Cubic Yard</td>
</tr>
</tbody>
</table>
SECTION 20.28 RECONSTRUCT DRIVEWAY

Article 28.1 Description

The Work under this Section consists of performing all operations and furnishing all materials pertaining to removing, disposing of, re-grading and replacing existing driveway approaches, including removal and disposal of existing pavement, excavation, surfacing, classified fill and backfill, leveling course, and working adjacent to existing landscaping amenities, as indicated on the Drawings.

Driveway reconstruction consists of installing a section of driveway that provides a smooth transition from the existing driveway to the street improvements. The transition length is measured from the back of curb or back of sidewalk and shall be as shown on the Drawings or as directed by the Engineer.

Contractor shall not disturb existing driveways that have imbedded heating systems.

Article 28.2 Materials

All materials used in the reconstruction of driveways shall conform to the requirements for Portland cement concrete Class A-3 conforming with Division 30 – Portland Cement Concrete and asphalt concrete pavement conforming with Division 40 – Asphalt Surfacing for asphalt paved driveways. Subbase material shall conform to the requirements of this Division.

Article 28.3 Construction

All construction practices, tests and other controls shall conform to Division 20 – Earthwork, Division 30 – Portland Cement Concrete, and Division 40 – Asphalt Surfacing.

The Contractor shall neatly and cleanly saw cut and remove existing driveway surfacing. Contractor shall cut a minimum of two inches (2") deep for asphalt surfaces and three inches (3") deep for concrete surfaces. If any portion of the remaining asphalt or concrete surfacing is under-cut or damaged during construction operations, Contractor shall saw cut, remove, and replace the affected area at no additional cost to the Owner.

The Contractor shall reconstruct existing driveways with asphalt or concrete surfacing to match existing driveway surface. Contractor shall place two inches (2") of asphalt surfacing over two inches (2") of leveling course and concrete surfacing at a thickness of six inches (6"). Concrete driveways shall have a minimum six by six inch (6" x 6") woven wire mesh reinforcement installed. Contractor shall provide all areas of reconstructed driveway with a minimum eighteen inches (18") of Type II-A Classified Fill and Backfill subbase, and, when required on the Drawings, geotextile fabric.

Contractor shall perform asphalt paving by utilizing a mechanical spreader and compact by a mechanical roller weighing not less than ten (10) tons, except that where the area of the asphalt replacement patch is less than three hundred (300) square feet, a mechanical spreader need not be employed.
Contractor shall tamp small inaccessible areas to produce a compression and surface texture equivalent to that produced by the specified rolling. Hand tampers shall have a maximum tamping face of fifty (50) square inches and minimum weight of twenty-five (25) pounds.

Contractor shall maintain access and parking accommodations for each resident during driveway work. Contractor shall notify and coordinate with the affected resident(s) prior to necessary driveway closures.

**Article 28.4 Measurement**

Driveway reconstruction is measured per square yard of replaced driveway surface complete and in place for the specified type of surface. No separate measurement is to be made for asphalt, classified backfill, excavation, geotextile fabric, of leveling course as these items are incidental to the Work item. No measurement is made for temporary relocation of driveways or required driveway maintenance during construction as these items are incidental to the Work item.

**Article 28.5 Basis of Payment**

Payment for this item shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Reconstruct Driveway, Asphalt (Class)</td>
<td>Square Yard</td>
</tr>
<tr>
<td>Reconstruct Driveway, Concrete (Class)</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 20.29 PIPE CASING

Article 29.1 General

The Work under this Section consists of performing all operations necessary for furnishing and placing a casing by trenchless method under structures, roadways, railroad tracks, or runways.

Article 29.2 Materials

Materials shall be as required by the Contract Documents.

Article 29.3 Construction

Method of installing a pipe casing shall be optional to the Contractor, except that prior to commencing jacking or augering operations, the Contractor shall furnish a work plan to the Engineer and show that his planned method of installation has worked satisfactorily in other areas under similar conditions. The excavation at both ends of the casing shall be considered incidental to this bid item and no separate payment shall be made.

A vertical and horizontal tolerance shall be as shown on the Drawings, provided that the Contractor will be responsible, and use such fittings as are required to adjust alignment and grade to accomplish the connections.

The pipe within the casing (barrier pipe) shall be arrested from movement by sand filling or wood slats and banding according to Standard Detail 20-18.

Article 29.4 Measurement

Measurement shall be from end to end of pipe casing acceptably installed and completed. No measurement will be made for trench excavation and backfill where casing is installed. No compensation will be made for casing installations abandoned or aborted due to deviations in excess of allowable tolerances.

Article 29.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described in this Section, including arrestment of pipe.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install Casing</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 20.30   SHORING, SHEETING AND BRACING/SHORING AND
SHEETING LEFT IN THE TRENCH AND PORTABLE

Article 30.1  General

The Work under this Section consists of all operations pertaining to furnishing and installing
sheeting, shoring, and bracing to support the trench section to prevent any movement that
might damage adjacent facilities or injure workmen or the public, and the use of portable
steel shielding.

Article 30.2  Materials

The Contractor shall obtain approval from the Engineer for all sheeting, bracing and
shoring materials and/or equipment to be used on the project. Materials used shall be in
accordance with Section 1926.651, Subparagraph 1 of the Federal Register, Volume 37,
No. 243, OSHA Regulations.

Article 30.3  Construction

All construction requirements for design, installation, and use of sheeting, shoring, bracing,
and shielding shall be in accordance with current safety regulations. All sheeting, shoring,
bracing, and shielding shall be designed by a Professional Engineer commissioned by the
Contractor. All shop drawings and design data shall be submitted to the Engineer for
approval.

When shoring and sheeting is left in the trench, sheeting must be driven lower than the
bottom of the pipe and cut off one foot (1’) below ground surface. No transverse bracing
will be permitted to remain.

Article 30.4  Measurement

The method of measurement for sheeting, shoring, and bracing shall be per linear foot of
trench section to be supported, per linear foot of shoring and sheeting authorized to be left
in the trench, or lump sum.
**Article 30.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall be full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Shoring, Sheeting and Bracing</td>
<td>Linear Foot of Trench Section Supported</td>
</tr>
<tr>
<td>Shoring, Sheeting and Bracing</td>
<td>Lump Sum</td>
</tr>
<tr>
<td>Shoring and Sheeting Left in Trench</td>
<td>Linear Foot of Trench Section Supported</td>
</tr>
<tr>
<td>Portable Steel Shield</td>
<td>Linear Foot of Trench Section Supported</td>
</tr>
</tbody>
</table>
MUNICIPALITY OF ANCHORAGE
STANDARD SPECIFICATIONS

DIVISION 20
EARTHWORK
STANDARD DETAILS
INDEX OF STANDARD DETAILS

20-1 Typical Sections - 20’/24’ Strip Paved and R.A.P. Streets
20-2 Typical Sections - Secondary Streets
20-3 Typical Sections - Primary Streets
20-4 Typical Section - Deep Excavation
20-5 Typical Section - Alley
20-6 Typical Section - Pathway
20-7 Driveway Connection Detail
20-8 Trench Backfill and Bedding Layout
20-9 Pipe Insulation
20-10 Class “B” Bedding Material
20-11 Class “C” Bedding Material
20-12 Class “D” Bedding Material
20-13 Foundation Materials
20-14 Type II Classified Fill and Backfill
20-15 Type II-A Classified Fill and Backfill
20-16 Type V Classified Fill and Backfill
20-17 Type VI Classified Fill and Backfill
20-18 Leveling Course
20-19 Pipe Encasement
20-20 Compaction of Backfill within the Right-of-Way
TYPICAL SECTION - 20'24' STRIP PAVING

TYPICAL SECTION - 20'24' RECYCLED ASPHALT STREET

NOTES:
1. PLACE OR REMOVE AND GRADE MATERIAL IN A NEAT MANNER FROM EXCAVATION LIMITS TO EXISTING ELEVATION AT PROPERTY LINE OR AS DIRECTED BY THE ENGINEER. (MAXIMUM 2:1 CUT AND FILL SLOPES)

2. ENGINEER WILL DETERMINE THE DEPTH OF EXCAVATION.

3. UNLESS OTHERWISE APPROVED, THE CENTERLINE OF STREET SHALL BE THE CENTERLINE OF R.O.W.

4. RECYCLED ASPHALT PAVING (R.A.P.) SHALL BE SEAL-COATED AS SPECIFIED IN DIVISION 40, SECTION 40.08 - RECYCLED ASPHALT PAVEMENT.
NOTES:

1. DIMENSIONS AND ELEVATIONS SHOWN ON THIS STANDARD DETAIL ARE TYPICAL. PROJECT SPECIFIC DIMENSIONS SHOWN ON THE DRAWINGS SHALL CONTROL.

2. PLACE OR REMOVE AND GRADE MATERIAL IN A NEAT MANNER FROM EXCAVATION LIMITS TO EXISTING ELEVATION AT PROPERTY LINE OR AS DIRECTED BY THE ENGINEER. (MAXIMUM 2:1 CUT AND FILL SLOPES)

3. ENGINEER WILL DETERMINE THE DEPTH OF EXCAVATION.

4. UNLESS OTHERWISE APPROVED, THE CENTERLINE OF STREET SHALL BE THE CENTERLINE OF R.O.W.

5. WHERE SIDEWALKS ARE NOT CONSTRUCTED, SEE STANDARD DETAIL 20–4 FOR SLOPING BETWEEN CURB AND PROPERTY LINE.
NOTES:
1. DIMENSIONS AND ELEVATIONS SHOWN ON THIS DRAWING ARE TYPICAL. PROJECT SPECIFIC DIMENSIONS SHOWN ON THE DRAWINGS CONTROL.
2. PLACE OR REMOVE AND GRADE MATERIAL IN A NEAT MANNER FROM EXCAVATION LIMITS TO EXISTING ELEVATION AT PROPERTY LINE OR AS DIRECTED BY THE ENGINEER (MAXIMUM−2:1 CUT AND FILL SLOPES).
3. ENGINEER WILL DETERMINE THE DEPTH OF EXCAVATION.
4. UNLESS OTHERWISE APPROVED, THE CENTERLINE OF STREET SHALL BE THE CENTERLINE OF R.O.W.
5. WHERE SIDEWALKS ARE NOT CONSTRUCTED, SEE STANDARD DETAIL 20−4 FOR SLOPING BETWEEN CURB AND PROPERTY LINE.
6. A.C. PAVEMENT DEPTH SHALL BE 1−1/2″ FOR CLASS D PAVEMENT AND 2″ FOR CLASS A PAVEMENT.
NOTES:

1. DIMENSIONS AND ELEVATIONS SHOWN ON THIS DRAWING ARE TYPICAL. PROJECT SPECIFIC DIMENSIONS SHOWN ON THE DRAWINGS SHALL CONTROL.

2. PLACE OR REMOVE AND GRADE MATERIAL IN A NEAT MANNER FROM EXCAVATION LIMITS TO EXISTING ELEVATION AT PROPERTY LINE OR AS DIRECTED BY THE ENGINEER. (MAXIMUM 2:1 CUT AND FILL SLOPES)

3. ENGINEER WILL DETERMINE THE DEPTH OF EXCAVATION.

4. SEE APPLICABLE STANDARD DETAIL FOR SPECIFIC STREET DIMENSIONS.
NOTES:

1. ENGINEER WILL DETERMINE THE DEPTH OF EXCAVATION.
NOTES:

1. DIMENSIONS AND ELEVATIONS SHOWN ON THIS STANDARD DETAIL ARE TYPICAL. PROJECT SPECIFIC DIMENSIONS SHOWN ON THE DRAWINGS SHALL CONTROL.

2. ENGINEER WILL DETERMINE THE DEPTH OF EXCAVATION.

3. ADJUST DEPTH OF DITCH AS NECESSARY FOR POSITIVE DRAINAGE AS SHOWN IN THE DRAWINGS OR AS DIRECTED BY THE ENGINEER.

4. PLACE CROSS CULVERTS AS SHOWN ON THE DRAWINGS OR AS DIRECTED BY THE ENGINEER.
NOTES:
1. SURFACING SHALL MATCH EXISTING DRIVEWAY.
2. ASPHALT PAVEMENT SHALL CONSIST OF 2” LEVELING COURSE AND 2” AC PAVEMENT (CLASS D). APPLY TACK COAT AT SAWCUT AND BACK OF CURB.
3. CONCRETE PAVEMENT SHALL CONSIST OF 6” P.C.C. WITH BROOM FINISH PARELLEL TO CURB AND GUTTER. PROVIDE EXPANSION JOINT AT CURB.
4. LENGTH OF DRIVEWAY REMOVED AND REPLACED VARIES AS SHOWN IN THE DRAWINGS OR AS DIRECTED BY THE ENGINEER.
5. INSULATION TRANSITION SHALL BE PROVIDED BELOW DRIVEWAY WHEN ADJACENT TO AN INSULATED ROADWAY.
NOTES:
1. TRENCH BACKFILL MATERIAL PLACED AND COMPACTED TO DEPTHS SHOWN IN THE DRAWINGS OR AS DETERMINED BY ENGINEER. COMPACT TRENCH BACKFILL TO A MINIMUM OF 95% MAXIMUM DENSITY.
2. TRENCH WALL SLOPES WILL VARY WITH SOIL STRENGTH AND CHARACTER. SLOPES SHALL CONFORM TO OSHA SAFETY STANDARDS.
3. WHEN DUCTILE IRON PIPE IS INSTALLED, BEDDING IS REQUIRED ONLY TO SPRING LINE.
4. BACKFILL SHALL BE FREE OF CLAYS AND ORGANIC MATERIALS.
5. SEE STANDARD DETAIL 20–9 FOR INSULATION DETAILS.
1. THIS DETAIL APPLIES ONLY WHERE INSULATION IS REQUIRED.
2. "A" IS DEPTH FOR PAYMENT UNDER "TRENCH EXCAVATION AND BACKFILL" WHERE INSULATION IS PLACED OVER EXISTING PIPE
3. "B" AS SHOWN ON DRAWINGS OR TO BE DETERMINED BY ENGINEER, FOUR FOOT (4’) MINIMUM.

PIPE INSULATION
GRADING LIMITS

U.S. STANDARD SIEVE

<table>
<thead>
<tr>
<th>SIZE OF OPENING (INCHES)</th>
<th>CUMULATIVE % PASSING BY WEIGHT</th>
</tr>
</thead>
<tbody>
<tr>
<td>1&quot;</td>
<td>100</td>
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<tr>
<td>#40</td>
<td>5–40</td>
</tr>
<tr>
<td>#200</td>
<td>*0–6</td>
</tr>
</tbody>
</table>

*IN ADDITION TO THE GRADING LIMITS LISTED ABOVE, THE FRACTION OF MATERIAL PASSING THE #200 SIEVE SHALL NOT BE GREATER THAN 35% OF THAT FRACTION PASSING THE #4 SIEVE. THE BEDDING MATERIAL SHALL NOT INCLUDE MECHANICALLY FRACTURED MATERIALS.

SIEVE ANALYSIS

<table>
<thead>
<tr>
<th>SIZE OF OPENING (INCHES)</th>
<th>NUMBER OF MESH PER INCH (U.S. STANDARD)</th>
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<tr>
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CLASS "B"
BEDDING MATERIAL

MUNICIPALITY
OF ANCHORAGE

SECTION # 20.16

NATIONAL TRANSPORTATION SAFETY BOARD

APPROVED:

REVISED: 11/08

SECTION # 20–10
GRADING LIMITS

U.S. STANDARD SIEVE CUMULATIVE % PASSING
                      BY WEIGHT

2"                        100
1/2"                      40–100
#4                       20–75
#10                       12–60
#40                       2–30
#200                      *0–6

*IN ADDITION TO THE GRADING LIMITS LISTED ABOVE, THE FRACTION OF MATERIAL PASSING THE #200 SIEVE SHALL NOT BE GREATER THAN 20% OF THAT FRACTION PASSING THE #4 SIEVE.

<table>
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<th>NUMBER OF MESH PER INCH (U.S. STANDARD)</th>
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<th>FINE</th>
<th>COARSE</th>
<th>MEDIUM</th>
<th>FINE</th>
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<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>SAND</td>
<td></td>
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CLASS 'C'
BEDDING MATERIAL

MUNICIPALITY OF ANCHORAGE

SCALE: NTS
APPROVED:
REVISED: 11/08

SECTION # 20.16
DETAIL # 20-11
GRADING LIMITS

U.S. STANDARD SIEVE CUMULATIVE % PASSING
BY WEIGHT

1"
3/4"
1/2"
3/8"
#4
#200

100
90–100
50–70
20–50
0–10
0–1

THE BEDDING MATERIAL SHALL NOT INCLUDE MECHANICALLY FRACTURED MATERIALS.

CLASS 'D'
BEDDING MATERIAL

MUNICIPALITY OF ANCHORAGE
SCALE:
NTS
APPROVED:
REVISED:
11/08
SECCTION # 20.16
DETAIL # 20-12
AREA FORMULA

PAY LIMITS
O.D. = OUTSIDE PIPE DIAMETER
a = 2' + O.D.
b = a + h
h = HEIGHT OF FOUNDATION MATERIAL
Area = 1/2 (a + b)h

SAMPLE CALCULATION
FOR 18" SINGLE WALL HDPE,
3' FOUNDATION HEIGHT
OUTSIDE PIPE DIAMETER = 21.8"
h = 3'
a = 2' + 21.8" = 23.8" = 3.82'
b = a + h = 3.82' + 3' = 6.82'
Area = 1/2 (a + b)h
    = 1/2 (3.82' + 6.82') * 3'
    = 15.96 SQUARE FEET

NOTES:
1. TRENCH WALL SLOPES WILL VARY WITH SOIL STRENGTH AND CHARACTER. SLOPES SHALL CONFORM TO OSHA SAFETY STANDARDS.
2. FOUNDATION MATERIALS PLACED AND COMPACTED TO DEPTHS ShOWN IN THE DRAWINGS OR AS DETERMINED BY THE ENGINEER. COMPACT FOUNDATION MATERIAL TO 95% MAXIMUM DENSITY, UNLESS OTHERWISE SPECIFIED.
3. USE THE AREA FORMULA TO CALCULATE THE AREA OF PAY LIMITS FOR ALL TYPES AND SIZES OF PIPE.
GRADING LIMITS

U.S. STANDARD SIEVE

<table>
<thead>
<tr>
<th>SIEVE SIZE (INCHES)</th>
<th>CUMULATIVE % PASSING BY WEIGHT</th>
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<tbody>
<tr>
<td>8&quot;</td>
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<tr>
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<td>70–100</td>
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<tr>
<td>1–1/2&quot;</td>
<td>55–100</td>
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<tr>
<td>3/4&quot;</td>
<td>45–85</td>
</tr>
<tr>
<td>#4</td>
<td>20–60</td>
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<td>#10</td>
<td>12–50</td>
</tr>
<tr>
<td>#40</td>
<td>4–30</td>
</tr>
<tr>
<td>#200</td>
<td>*2–6</td>
</tr>
</tbody>
</table>

*IN ADDITION TO THE GRADING LIMITS LISTED ABOVE, THE FRACTION OF MATERIAL PASSING THE #200 SIEVE SHALL NOT BE GREATER THAN 20% OF THAT FRACTION PASSING THE #4 SIEVE.*

SIEVE ANALYSIS

<table>
<thead>
<tr>
<th>SIZE OF OPENING (INCHES)</th>
<th>NUMBER OF MESH PER INCH (U.S. STANDARD)</th>
</tr>
</thead>
</table>

COBBLES: COARSE, FINE
GRAVEL: COARSE, MEDIUM, FINE
SAND: COARSE, MEDIUM, FINE

TYPE II
CLASSIFIED FILL AND BACKFILL

SECTION # 20.21
DETAIL # 20–14
**GRADING LIMITS**

<table>
<thead>
<tr>
<th>U.S. STANDARD SIEVE</th>
<th>CUMULATIVE % PASSING BY WEIGHT</th>
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<tbody>
<tr>
<td>3&quot;</td>
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<tr>
<td>3/4&quot;</td>
<td>50–100</td>
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<tr>
<td>#4</td>
<td>25–60</td>
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<td>#10</td>
<td>15–50</td>
</tr>
<tr>
<td>#40</td>
<td>4–30</td>
</tr>
<tr>
<td>#200</td>
<td>*2–6</td>
</tr>
</tbody>
</table>

*In addition to the grading limits listed above, the fraction of material passing the #200 sieve shall not be greater than 20% of that fraction passing the #4 sieve.*

---

**SIEVE ANALYSIS**

<table>
<thead>
<tr>
<th>SIZE OF OPENING (INCHES)</th>
<th>NUMBER OF MESH PER INCH (U.S. STANDARD)</th>
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---

**TYPE II-A**

**CLASSIFIED FILL AND BACKFILL**

MUNICIPALITY: Anchorage

 SCALE: NTS

 APPROVED: 11/08

 REvised: 20-15

SECTION #: 20.21

DETAIL #: 20-15
**GRADING LIMITS**

**U.S. STANDARD SIEVE**

<table>
<thead>
<tr>
<th>SIZE</th>
<th>CUMULATIVE % PASSING BY WEIGHT</th>
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</thead>
<tbody>
<tr>
<td>3&quot;</td>
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<tr>
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<td>4–30</td>
</tr>
<tr>
<td>#200</td>
<td>2–6</td>
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</table>

*IN ADDITION TO THE GRADING LIMITS LISTED ABOVE, AT LEAST THIRTY PERCENT (30%) OF THE COARSE AGGREGATE PARTICLES SHALL HAVE ONE OR MORE MECHANICALLY FRACTURED FACE.*

**SIEVE ANALYSIS**

<table>
<thead>
<tr>
<th>SIZE OF OPENING (INCHES)</th>
<th>NUMBER OF MESH PER INCH (U.S. STANDARD)</th>
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</thead>
</table>

**TYPE V**

**CLASSIFIED FILL AND BACKFILL**

**SECTION #**

20.21

**DETAIL #**

20–16
GRADING LIMITS

U.S. STANDARD SIEVE        CUMULATIVE % PASSING
BY WEIGHT

<table>
<thead>
<tr>
<th>Size</th>
<th>By Weight</th>
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<tr>
<td>3/4&quot;</td>
<td>50–80</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>30–60</td>
</tr>
<tr>
<td>#4</td>
<td>20–50</td>
</tr>
<tr>
<td>#10</td>
<td>10–30</td>
</tr>
<tr>
<td>#40</td>
<td>5–25</td>
</tr>
<tr>
<td>#200</td>
<td>2–6</td>
</tr>
</tbody>
</table>

*IN ADDITION TO THE GRADING LIMITS LISTED ABOVE AT LEAST FORTY PERCENT (40%) OF THE COARSE AGGREGATE PARTICLES SHALL HAVE ONE OR MORE MECHANICALLY FRACTURED FACE.

SIEVE ANALYSIS

<table>
<thead>
<tr>
<th>Size of Opening (Inches)</th>
<th>Number of Mesh Per Inch (U.S. Standard)</th>
</tr>
</thead>
</table>

TYPE VI

CLASSIFIED FILL AND BACKFILL
GRADING LIMITS

U.S. STANDARD SIEVE CUMULATIVE % PASSING
BY WEIGHT

1"  100
3/4"  70-100
3/8"  50-80
#4    35-65
#8    20-50
#50   8-28
#200  *2-6

*IN ADDITION TO THE GRADING LIMITS LISTED ABOVE, THE FRACTION OF MATERIAL PASSING THE #200 SIEVE SHALL NOT BE GREATER THAN 75% OF THAT FRACTION PASSING THE #50 SIEVE.

SIEVE ANALYSIS

<table>
<thead>
<tr>
<th>SIZE OF OPENING (INCHES)</th>
<th>NUMBER OF MESH PER INCH (U.S. STANDARD)</th>
</tr>
</thead>
<tbody>
<tr>
<td>6</td>
<td>3</td>
</tr>
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</tr>
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LEVELING COURSE

SECTION # 20.22
DETAIL # 20-18

MUNICIPALITY OF ANCHORAGE
SCALE: NTS
APPROVED:
REVISED: 10/08
NOTES:

1. CASING PIPE SHALL BE WELDED STEEL PIPE, AND GAUGE SHALL BE AS SHOWN ON DRAWINGS. CASING PIPE SHALL BE DESIGNED FOR ALL LOADS FOR EACH APPLICATION.

2. INSTALL CASING SPACERS A MAXIMUM OF ONE FOOT (1') FROM EACH SIDE OF EACH PIPE JOINT. CASING SPACERS SHALL BE CASCADE WATERWORKS MFG. STAINLESS STEEL WITH POLYETHYLENE RUNNERS OR APPROVED EQUAL.

3. ENDS OF CASING PIPE SHALL BE SEALED WITH SYNTHETIC RUBBER SEAL WITH STAINLESS STEEL BANDS. CASING SHALL BE WATERTIGHT.

4. CARRIER PIPE SHALL HAVE FIELD LOK® GASKETS OR APPROVED EQUAL INSTALLED ENTIRE LENGTH OF CASING PIPE AND EXTEND ONE FULL PIPE LENGTH BEYOND END OF CASING.

5. JOINT BONDS OR THAW WIRES SHALL BE INSTALLED THE ENTIRE LENGTH OF CARRIER PIPE PER AWWU DCPM CORROSION CONTROL MAGNESIUM BAG ANODE INSTALLATION DETAIL.

6. CORROSION ANALYSIS SHALL BE PERFORMED FOR CASING PIPE.
NOTES:
1. REPLACE ALL MATERIAL THAT IS TO BE BACKFILLED WITHIN THE ABOVE-DESCRIBED AREA IN ONE-FOOT LIFTS PER DIVISION 20, SECTION 20.21, ARTICLE 21.3 — CONSTRUCTION.
2. BACKFILL SHALL BE FREE OF CLAYS AND ORGANIC MATERIALS.
3. COMPACT BACKFILL BY MECHANICAL MEANS WITHOUT THE AID OF WATER.
4. RESHAPE DITCH LINE IN SUCH A MANNER AS TO PROVIDE PROPER DRAINAGE; REPLACE SHOULDER OF THE ROAD AT A UNIFORM SLOPE NOT TO EXCEED 2 TO 1.
## STANDARD CONSTRUCTION SPECIFICATIONS FOR
PORTLAND CEMENT CONCRETE
DIVISION 30

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SECTION 30.01 GENERAL

Article 1.1 Scope of Work

The Work covered by this Section consists of providing all plant, labor, equipment, supplies, material, transportation, handling, storage and protection for performing all operations in connection with the placement of Portland Cement Concrete in accordance with the Specifications and the Drawings.

Article 1.2 Applicable Standards

The latest revision of the following standards of the American Society for Testing and Materials (ASTM) and American Society of State Highway and Transportation Officials (AASHTO) are hereby made a part of these Specifications:

American Concrete Institute "Manual of Concrete Practice"
Concrete Reinforcing Steel Institute "Manual of Standard Practice"
ASTM A-185 AASHTO M-55 Specification for Welded Steel Wire Fabric for Concrete
ASTM A-615 AASHTO M-31 Specification for Billet-Steel Bars for Concrete Reinforcement
ASTM C-31 AASHTO T-23 Method of Making and Curing Concrete Compression and Flexure Test Specimens in the Field
ASTM C-33 Specification for Concrete Aggregates
ASTM C-330 Specification for Lightweight Aggregates for Structural Concrete
ASTM C-39 AASHTO T-22 Test for Compressive Strength of Molded Concrete Cylinders.
ASTM C-40 AASHTO T-21 Test for Organic Impurities in Sands for Concrete
ASTM C-42 AASHTO T-24 Method of Securing, Preparing and Testing Specimens from Hardened Concrete for Compression and Flexure Strengths
ASTM C-90 Hollow Load-Bearing Concrete Masonry
ASTM C-94  AASHTO M-157  Specification for Ready-Mix Concrete  
ASTM C-150  AASHTO T-119  Specification for Portland Cement  
ASTM C-156  AASHTO T-155  Test for Water Retention Efficiency of Liquid Membrane-Forming Compounds and Impermeable Sheet Materials for Curing Concrete.  
ASTM C-171  AASHTO M-171  Specification for Waterproof Paper for Curing Concrete  
ASTM C-172  AASHTO T-141  Sampling Fresh Concrete  
ASTM C-192  AASHTO T-126  Method of Making and Curing Concrete Compression and Flexure Test Specimens in the Laboratory  
ASTM C-231  AASHTO T-152  Test for Air Content of Freshly Mixed Concrete by the Pressure Method  
ASTM C-260  AASHTO M-154  Specification for Air-Entraining Admixture for Concrete  
ASTM C-270  Mortar for Unit Masonry (Including Tentative Revision)  
ASTM C-309  AASHTO M-148  Specification for Liquid Membrane-Forming Compounds for Curing Concrete  
ASTM C-494  AASHTO M-194  Specifications for Chemical Admixtures for Concrete  
ASTM D-1751  AASHTO M-213  Specification for Preformed Expansion Joint Filler for Concrete Paving and Structural Construction (Nonextruding and Resilient Bituminous Types)  
AASHTO M-6  Specification for Fine Aggregate for Portland Cement Concrete  
AASHTO M-32  Specification for Cold Drawn Steel Wire for Concrete Reinforcement Cement  
AASHTO M-80  Specification for Coarse Aggregate for Portland Concrete  

Article 1.3  Materials  

A.  Reinforcing Steel  

Concrete reinforcing shall be deformed steel bars conforming to the requirements of ASTM A-615 (AASHTO M-31). It shall be free from loose scales, excessive rust,
and coatings of any character which will reduce the bond between steel and concrete.

If reinforcing bars are to be welded, these Specifications shall be supplemented by requirements assuring satisfactory weldability in conformity with AWS D12.1, "Recommended Practices for Welding Reinforcing Steel, Metal Inserts and Connections in Reinforced Concrete Construction."

B. Welded Steel Wire Fabric

The welded steel wire fabric shall be cold-drawn steel wires or galvanized, fabricated into mesh formed by the process of electric welding. The grade of wire shall conform to AASHTO M-32. Welded Steel Wire Fabric shall conform to ASTM A-185 (AASHTO M-55).

C. Cement

The cement shall be of a recognized standard brand of Portland Cement conforming to the specification requirements listed below and of a type listed below:

<table>
<thead>
<tr>
<th>Specification</th>
<th>Type Portland Cement</th>
</tr>
</thead>
<tbody>
<tr>
<td>ASTM C-150</td>
<td>Type I, III*</td>
</tr>
<tr>
<td>AASHTO M-85</td>
<td>Type I, III*</td>
</tr>
</tbody>
</table>

*Type III cement may be used upon written authorization of the Engineer subject to the following modification:

Minimum design strength shall be achieved in seven (7) days in lieu of the twenty-eight (28) days required for Type I cement.

For architectural concrete only one brand of cement shall be used unless otherwise approved by the Engineer. When no type cement is specified, the requirements of Type I shall govern.

Cement reclaimed from cleaning bags or leaking containers shall not be used.

The Engineer may require an additional one-half sack of Portland concrete over the design specification.

D. Water

Water used for the mixing of concrete shall be clean and free of oil or acid, and shall not contain salt, alkali, or organic matter.

E. Aggregates

Aggregates for Portland Cement Concrete shall be well graded, clean, hard gravel, and coarse sand, non-frost susceptible material, and free of deleterious (organic)
matter, and coatings of silt or clay. The gradations shall be determined by standard laboratory sieves with square openings. Material retained on a No. 4 screen shall be classified as coarse aggregate, which shall conform to the requirements of AASHTO M-80 and have the following limits of gradation:

### Coarse Aggregate for Portland Cement Concrete

<table>
<thead>
<tr>
<th>U.S. Standard Sieve Size</th>
<th>AASHTO No. 67 (¾” to #4) % Passing By Weight</th>
<th>AASHTO No. 4 (1 ½” to ¾”) % Passing By Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>2”</td>
<td>-----</td>
<td>100</td>
</tr>
<tr>
<td>1 ½”</td>
<td>-----</td>
<td>90–100</td>
</tr>
<tr>
<td>1”</td>
<td>100</td>
<td>20–55</td>
</tr>
<tr>
<td>3/4”</td>
<td>90–100</td>
<td>0–15</td>
</tr>
<tr>
<td>1/2”</td>
<td>-----</td>
<td>-----</td>
</tr>
<tr>
<td>3/8”</td>
<td>20–55</td>
<td>0–5</td>
</tr>
<tr>
<td>#4</td>
<td>0-10</td>
<td>-----</td>
</tr>
<tr>
<td>#8</td>
<td>0-5</td>
<td>-----</td>
</tr>
</tbody>
</table>

All material passing a No. 4 sieve shall be classified as fine aggregate and shall conform to the requirements of AASHTO M-6 and have the following gradation:

### Fine Aggregate for Portland Cement Concrete

<table>
<thead>
<tr>
<th>U.S. Standard Sieve Size</th>
<th>Cumulative % Passing By Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>3/8”</td>
<td>100</td>
</tr>
<tr>
<td>#4</td>
<td>95-100</td>
</tr>
<tr>
<td>#8</td>
<td>80-100</td>
</tr>
<tr>
<td>#16</td>
<td>50-85</td>
</tr>
<tr>
<td>#30</td>
<td>25-60</td>
</tr>
<tr>
<td>#50</td>
<td>10-30</td>
</tr>
<tr>
<td>#100</td>
<td>2-10</td>
</tr>
</tbody>
</table>

Aggregates shall consist of washed sand gravel; fine and coarse aggregates shall be regarded as separate ingredients. Aggregates for normal weight concrete shall conform to the requirements of ASTM C-33 and aggregates for light weight concrete, shall conform to the requirements of ASTM C-330.
The maximum size of coarse aggregates shall not exceed one and one-half inches (1 1/2") nor one-fifth (1/5) of the narrowest dimension between the forms nor three-quarters (3/4) of the clear spacing between reinforcing bars nor one-third (1/3) of the depth of slabs. The combined aggregates, coarse and fine, shall be of such composition of sizes that when separated on the No. 4 standard sieve, the weight passing shall not be less than thirty percent (30%) nor greater than fifty percent (50%) of the total weight.

The volume removed by sedimentation shall not exceed three percent (3%). Proportioning of the coarse and fine aggregate shall be obtained by weight. The weighing equipment shall be accurate within one percent (1%) of the net weight of the batch and shall permit adjustment for variations in the water content of the aggregate. Batching for fractional sacks of cement will not be permitted unless the cement is weighed for each batch. The water added shall be measured by an approved calibrated device capable of metering within one percent (1%) of the total amount of water to be used for each batch.

F. Air-Entrainment

Air-entrainment agents shall be used in all concrete. Entrainment shall be achieved by the addition of an approved air-entaining mixture to the concrete mix. Air-entrainment shall conform to ASTM C-231 (AASHTO T-152). Refer to Article 1.6 - Mix Requirements for Classes of Concrete for air-entrainment percentages for each class of concrete.

G. Curing Materials

Curing materials shall be one the following types as approved by the Engineer:

1. Kraft paper conforming to the requirements of ASTM C-171 (AASHTO M-171).
2. Mats of commercial quality and of a type used for curing concrete.
3. Burlap of commercial quality weighing not less than fourteen (14) ounces per square yard.
4. Membrane curing compound conforming to the requirements of ASTM C-309 (AASHTO M-148).

H. Expansion Joints

Premolded joint filler for use in expansion joints shall conform to the requirements of ASTM D-1751 (AASHTO M-213).

Article 1.4 Mix

Portland Cement Concrete may be mixed at a central mixing plant or in transit mixers. All mixing equipment and operations shall conform to the requirements of ASTM C-94
(AASHTO M-157). All concrete shall be delivered to the work site thoroughly mixed to a uniform color and show uniform distribution of aggregates and cement throughout the mixture.

Concrete shall be delivered to the Project site, discharged from the truck completely and in the forms ready for vibration within one and one-half (1-1/2) hours after introduction of the cement to the aggregates. At the discretion of the Engineer, the above period may be extended one (1) minute for every degree of temperature at which the concrete is delivered below seventy degrees (70°F) Fahrenheit to a maximum total time of two (2) hours.

In hot weather, or under conditions contributing to quick setting of the concrete, a discharge time less than one and one-half (1-1/2) hours may be required by the Engineer. Any concrete remaining undischarged at the end of the respective time period shall be rejected.

The use of non-agitating equipment for transporting concrete will not be permitted. The mixing drums of transit-mix trucks shall be thoroughly washed after discharging each load to prevent the accumulation of adherent layers of concrete. The discharge of particles of hardened concrete with any batch will be sufficient grounds for the rejection of the entire batch. On curb, gutter and sidewalk work, transit mix trucks shall be operated parallel to the forms while discharging.

The addition of water to the mix at the job site will not be permitted except with the approval of the Engineer. Any additional water that is added shall be documented and recorded on the delivery ticket by a representative of the supplier.

**Article 1.5 Subbase**

Prior to placement of forms, the Engineer shall inspect the subbase to insure that it is smooth, compacted and free of soft or yielding spots and that compaction at optimum moisture is at least ninety-five percent (95%) of maximum density (AASHTO T-180 D). Backfilling within the forms will be permitted if the subbase is brought to the above specification and care is taken to maintain the forms to line, shape elevation.

**Article 1.6 Mix Requirements for Classes of Concrete**

The minimum mix requirement for classes of concrete shall be as set forth below, unless otherwise specified in the Contract Documents.
CLASS OF CONCRETE

<table>
<thead>
<tr>
<th></th>
<th>C-6</th>
<th>B-3</th>
<th>B-6</th>
<th>A-3</th>
<th>A-6</th>
<th>AA-3</th>
<th>AA-6</th>
</tr>
</thead>
<tbody>
<tr>
<td>Minimum Cement Content in Sacks/Cu. Yd.</td>
<td>4.5</td>
<td>5.0</td>
<td>5.0</td>
<td>5.5</td>
<td>5.5</td>
<td>6.0</td>
<td>6.0</td>
</tr>
<tr>
<td>Maximum Water Content Ratio in Gal./Sack</td>
<td>6.5</td>
<td>6.5</td>
<td>6.5</td>
<td>6.5</td>
<td>6.5</td>
<td>5.75</td>
<td>5.5</td>
</tr>
<tr>
<td>Slump Ranges in Inches</td>
<td>1-5</td>
<td>2-4</td>
<td>1-3.5</td>
<td>2-4</td>
<td>1-3.5</td>
<td>2-4</td>
<td>1-2</td>
</tr>
<tr>
<td>Entrained Air Range in Percentage</td>
<td>3-6</td>
<td>4-7</td>
<td>3-6</td>
<td>4-7</td>
<td>3-6</td>
<td>4-7</td>
<td>3-6</td>
</tr>
<tr>
<td>Coarse Aggregate (AASHTO Gradation)</td>
<td>No. 4 and No. 67</td>
<td>No. 67</td>
<td>No. 4* and No. 67</td>
<td>No. 67</td>
<td>No. 4* and No. 67</td>
<td>No. 67</td>
<td>No. 4* and No. 67</td>
</tr>
</tbody>
</table>

Fine Aggregate Shall Conform to AASHTO M-6 Gradation

| Minimum Design Strength (fc), psi | 2000 | 2500 | 2500 | 3000 | 3000 | 3500 | 3500 |

Minimum design compressive strength specification shall be achieved in twenty-eight (28) days.

*The coarse aggregate for Class B-6, A-6, and AA-6 concrete shall be furnished in two (2) separate sizes.

Alternate mix designs will be considered upon submitting to the Engineer the following information:

1. MOA mix design designation for which the substitution is intended.
2. Design strength.
3. Air content.
4. Slump.
5. Aggregate gradation and maximum size.
7. Minimum cement content.
8. List of admixtures, strength overdesign, and other special features.
9. Fine aggregate weight/percent moisture of fine aggregate.
10. Intermediate aggregate weight/percent moisture of intermediate aggregate.
11. Coarse aggregate weight/percent moisture of coarse aggregate.
12. Weight of cement.
13. Weight of water.
14. Unit weight.

Water shall not be used to increase slump beyond four inches (4”). If additional slump is desired, a plasticizing agent may be used subject to the Engineer’s written approval.

**Article 1.7 Ready-Mixed Concrete**

Ready-mixed concrete shall conform to the requirements of ASTM C-94 (AASHTO M-157). For each batch of concrete, it is the responsibility of the Contractor to furnish to the Engineer, before unloading at the site, a delivery ticket from the manufacturer on which is printed, stamped or written, information concerning said concrete as follows:

1. Name of ready-mix batch plant.
2. Serial number of ticket.
3. Date and truck number.
4. Name of Contractor.
5. Specific designation of Project (name and location).
6. Specific class of concrete in conformance with that employed in Specifications.
7. Amount of concrete (cubic yards).
8. Time loaded or first mixing of cement and aggregate.
9. Type of cement.
10. Admixtures and amount of same.
11. Slump requested by the Contractor and recorded in inches.
12. Percentage of entrained air requested by the Contractor.
13. Fine aggregate weight/percent moisture of fine aggregate.
15. Coarse aggregate weight/percent moisture of coarse aggregate.
16. Weight of cement.
17. Weight of water.
18. Unit weight.
Article 1.8 Sampling and Testing

The Engineer shall take concrete samples for concrete cylinders in accordance with AASHTO T-141. Samples shall not be taken at the beginning or end of discharge. Making and curing the specimens shall be done in accordance with AASHTO T-23. Testing and sampling shall be done by the Engineer.

Slump tests shall be taken in accordance with AASHTO T-119 or ASTM C-143. Slump tests shall be taken by the Engineer.

Should the analysis of any test cylinder not meet the requirements of these Specifications, all concrete placed from the batch represented by the cylinder shall be removed and replaced at the Contractor's expense.

Article 1.9 Weather Limitations

Placement of Portland Cement Concrete is subject to the following requirements:

1. Salt, chemicals, or other material shall not be mixed with the concrete to prevent freezing.
2. Placement of concrete shall be prohibited whenever there is standing water on the grade or in the forms, the subgrade is yielding due to saturation, or rain is threatening.
3. Approved admixture shall be used in accordance with the manufacturer's recommendations.

Placement of concrete shall be prohibited at an ambient air temperature of less than forty degrees (40°) Fahrenheit or where the foundation material is frozen, except in special situations where authorized by the Engineer in writing. Exemption from the temperature clause of these Specifications shall be considered only under the following conditions:

A written proposal shall be submitted by the Contractor to the Engineer outlining a procedure for maintaining the placed concrete temperature of at least fifty degrees (50°) Fahrenheit for seventy-two (72) hours where Type III cement has been used and one hundred and twenty (120) hours where Type I cement has been used. When the temperature is reduced, the drop in temperature must be gradual and not exceed thirty degrees (30°) Fahrenheit in the first twenty-four (24) hours.

Article 1.10 Protection of Work

The Contractor shall protect all newly placed concrete from damage of any kind to prevent disfigurement during the curing period. Damaged concrete shall be repaired or replaced to the Engineer's satisfaction at no additional cost.

Type I/II Portland Cement Concrete must have been placed and finished a minimum of seven (7) days prior to material being distributed against, or vibrated (compaction) adjacent to the structure.
Type III Portland Cement Concrete must have been placed and finished a minimum of three (3) days prior to material being distributed against, or vibrated (compaction) adjacent to the structure.

**Article 1.11 Clean-up**

When all concrete Work has been completed and cured, the Contractor shall remove the forms, stakes, blocking, and concrete spoil from the site. The area adjoining the concrete that was excavated to permit the construction and placement of forms shall be filled with select material, and the slopes and parking areas, if any, shall be filled, shaped and smoothed to the level as shown on the Drawings or Standard Details.
SECTION 30.02 PORTLAND CEMENT CONCRETE, CURB AND GUTTER AND VALLEY GUTTER

Article 2.1 Description

The Work covered under this Section consists of the construction of curbs, gutters, miscellaneous median shapes and parking stops.

Article 2.2 Materials

Portland Cement Concrete, joint filler, reinforcing steel and curing materials shall conform to Section 30.01, Article 1.3 - Materials. Concrete mix for curbs shall conform to the requirements for Class A-3 unless otherwise specified.

A. Reinforcing Steel and Steel Dowels

Refer to Section 30.01, Article 1.3, SubArticle A. - Reinforcing Steel and SubArticle B. - Welded Steel Wire Fabric.

B. Preformed Expansion and Dummy Joint Filler

Refer to Section 30.01, Article 1.3, SubArticle H. - Expansion Joints.

C. Curing Compounds

Refer to Section 30.01, Article 1.3, SubArticle G. – Curing Materials.

D. Forms

Forms may be of wood or metal or any other material at the option of the Contractor, provided that the forms as set will result in a curb, or curb and gutter of the specified thickness, cross section, grade and alignment shown on the Drawings.

Forms may be removed on the day following pour if the concrete is sufficiently set that removal can be accomplished without danger of chipping or spalling. Form materials shall be free from warp, with smooth and straight upper edges, and if used for the face of a curb, shall be surfaced on the side against which the concrete is to be placed. Wooden forms for straight work shall have a net thickness of at least one and one-half inches (1.5”). Metal forms for such a work shall be of a gage that will provide equivalent rigidity and strength. Curb face forms used on monolithic curb and gutter construction shall be a single plank width when the curb face is ten inches (10”) or less, except for those used in curb returns. All forms used in curb returns shall not be less than three-quarters inches (3/4”) in thickness, cut in the length and radius as shown on the Drawings, and held rigidly in place to line and grade by the use of metal stakes and clamps. The curb face form shall be cut to conform exactly with the curb face batter as well as being cut to the required length and radius. Forms shall be of sufficient rigidity and strength, and shall be
supported to adequately resist springing or deflection from placing and tamping of concrete.

Form material shall be clean and free from defect at the time of use.

All forms including back planks of curb shall be set with upper edges flush with specified alignment and grade of the finished surface of the improvements to be constructed, and all forms shall be not less than a depth equivalent to full specified thickness of the concrete to be placed.

Forms shall be held securely in place by means of metal stakes driven in pairs at intervals not to exceed three feet (3’), one at the front form and one at the back form. Clamps, spreaders, and braces shall be used to the extent as may be necessary to insure proper form rigidity. Forms for walk and similar work shall be firmly secured by means of stakes driven at intervals not to exceed four feet (4’). Form stakes shall be of sufficient size and be driven so as to adequately resist lateral displacement.

Commercial form clamps for curb and gutter may be used provided they fulfill the requirements specified herein.

Pump trucks may be used upon approval of the Engineer. Prior to approval, the Contractor must demonstrate to the satisfaction of the engineer that the pumping equipment will not segregate, or in any other way degrade, the concrete. Additional test samples for such alternate placement methods may be taken from the discharge side of the machine for compressive strength determination assurance tests.

**Article 2.3 Construction**

**A. Erecting Forms**

All forms shall be set to the lines, grade, and dimensions shown on the Drawings. The forms shall be thoroughly braced and secured to resist deformation or displacement under load, and shall be installed to permit easy removal without hammering or prying against the fresh concrete. The top of the forms shall not deviate more than one-eighth inch (1/8”) in ten feet (10’), and the alignment of forms shall be within one-fourth inch (1/4”) in ten feet (10’).

Before placement of concrete, steel forms shall be lightly oiled with a good grade of form oil. Excess oil shall be removed by wiping with clean rags, dampened in diesel or fuel oil. Wooden forms may be oiled in the same manner as metal forms, or they may be watered immediately in advance of the placement of concrete. Watering of the form shall be done with clean water of the same quality as that specified for mixing water, and only when the atmospheric temperature is not less than forty degrees (40°) Fahrenheit. Concrete shall not be placed until all forms have been inspected and approved by the Engineer. Wherever form work is exposed to pedestrian traffic, bridges (not attached to the forms) shall be provided...
at all regular pedestrian crossings where it is required to maintain safety standards. Barricades and other safety features shall be installed as necessary.

B. Placing Concrete

Prior to the delivery of the first load of concrete for curbs, the Contractor shall furnish rigid straightedges, ten feet (10’) or sixteen feet (16’) in length, to the Engineer for checking surface uniformity. String shall not be used as a straightedge. Surface irregularities, as measured along the top face of curb and the curb pan, shall not exceed three-sixteenth inch (3/16”) within ten feet (10’), or five-sixteenth inch (5/16”) within sixteen feet (16’). Non-conforming surfaces shall be subject to rejection by the Engineer. All surfaces rejected by the Engineer shall be corrected by the Contractor at the Contractor’s expense.

The subgrade shall be properly compacted and brought to specified grade in accordance with the Drawings before placing concrete. The subgrade shall be thoroughly dampened immediately prior to the placement of the concrete. Forms shall not be splashed with concrete in advance of placing.

Concrete shall be discharged from transport vehicle to the point of final placement in a continuous manner as rapidly as practicable. The rate of placement shall not exceed the rate at which the various placing and finishing operations can be performed in accordance with these Specifications. Concrete shall not be allowed to free fall more than three feet (3’).

If concrete is to be placed by the extruded method, the Contractor shall demonstrate to the satisfaction of the Engineer that the machine is capable of placing a dense, uniformly compacted concrete to exact section, line and grade. Extruded curb which does not meet all requirements of the Contract Documents, shall be replaced at the Contractor’s expense.

C. Stripping Forms and Finishing

The face form of the curb shall be stripped at such time in the early curing as will enable inspection and correction of all irregularities that appear thereon.

Forms shall not be removed until the concrete has set sufficiently to retain its true shape. The face of the curb shall be troweled with a tool cut to the exact section of the curb and at the same time maintain the shape, grade, and alignment of the curb. Both front and back edges shall be troweled to a radius of one-half inch (1/2”). Final finish shall be obtained by brooming the surface, including the troweled edge to a gritty finish after all free moisture has disappeared from the surface. Sprinkling of cement or sand for blotting will not be permitted.

It is the intent of this Specification to insure the highest quality of workmanship in the construction and finishing of P.C.C. curb and gutter.
Unsightly or poorly finished surfaces will be considered grounds for rejection of the Work. The top and/or face and gutter of the finished concrete surfaces shall be true and straight, of uniform width and free of cracks, humps, sags, or other irregularities. The finished concrete surface shall not vary more than two-hundredths of a foot (0.02') from a ten foot (10') straight edge, except at grade changes or curves. No freestanding water is permitted on slopes at or greater than one percent (1%). No freestanding water deeper than one-sixteenth inch (1/16") is permitted on slopes of less than one percent (1%). The Contractor shall flow test all new concrete curb and gutter. Curb and gutter failing to meet this requirement will be rejected.

All defective areas shall be removed and replaced at the Contractor's expense, unless permission to patch is granted by the Engineer. Such permission shall not be construed as an acceptance of the Work or as a waiver of the Engineer's right to require the complete removal of the Work, if in his opinion the patch does not satisfactorily restore the quality or appearance of the surface.

Should patching be permitted, the area shall be chipped clean to a depth of one inch (1") perpendicular to the surface and saturated with clean water prior to being patched. The patch shall be made with a mortar extracted from fresh concrete by passing it through a three-eighths inch (3/8") screen. The mortar shall be thoroughly compacted and screeded off slightly higher than the surrounding surface to allow for contracting or setting after the maximum shrinkage has taken place. After one (1) to two (2) hours, the patch shall be troweled to the same finish as the surrounding area and shall be cured as specified herein. The use of special patching material will be permitted if approved by the Engineer.

D. Curing

Curing compounds shall be applied to all exposed surfaces immediately after finishing. Transparent curing compounds shall contain a color dye of sufficient strength to render the film distinctly visible on the concrete for a minimum period of four (4) hours after application.

If, at any time during the curing period any of the forms are removed, a coat of curing compound shall be applied immediately to the exposed surface. The curing compound shall be applied in sufficient quantity to obscure the natural color of the concrete. Additional coats shall be applied if the Engineer determines that the coverage is not adequate. The concrete shall be cured for the minimum period of time set forth below.

Curb and gutter constructed of Type I/II Portland Cement Concrete must have been placed and finished a minimum of seven (7) days prior to material being distributed against, or vibrated (compaction) adjacent to the structure.

Curb and gutter constructed of Type III Portland Cement Concrete must have been placed and finished a minimum of three (3) days prior to any material being distributed against, or vibrated (compaction) adjacent to the structure.
When forms are removed before the expiration of the curing period, the edges of
the concrete shall be protected with moist earth, or sprayed with curing compound.

Other standard methods of curing the curb and gutter may be used upon approval
of the Engineer. Concrete shall not be placed unless curing compounds and
necessary equipment for applying such is on the Project site.

E. Expansion and Contraction Joints

1. Expansion Joints

Expansion joints shall be placed along all structures and around all features
that project into, through, or against the concrete. An expansion joint shall
be constructed at the intersection of sidewalks; between sidewalk crossings
and sidewalks; between curbs and sidewalks (except parallel curb); and at
the beginning and end of curb returns. Additionally expansion joints shall be
constructed every fifty feet (50') where the sidewalk span exceeds seventy-
five feet (75') and expansion joints are not required for the above listed
reasons. Expansion joint material shall conform to the requirements of
ASTM D-1751 (AASHTO M-213). This material shall extend the full width of
the structure and shall be cut to such dimensions that the base of the
expansion joint shall extend to the subgrade and the top shall be depressed
not less than one-quarter inch (1/4") nor more than one-half inch (1/2") below
the finished surface of the concrete. The material shall be of one (1) piece in
the vertical dimension and shall be securely fastened in a vertical position to
the existing concrete face against which fresh concrete is to be placed. After
the concrete has set, the expansion joints shall be filled flush to the finish
concrete surface with an approved polyurethane sealant applied according
to the manufacturer's recommendation.

Before sealing, the joint shall be cleaned of all dirt, gravel, concrete mortar,
and other extraneous material. Sealing shall be done in a neat workmanlike
manner.

2. Contraction Joints

Transverse contraction joints, cut to a depth of one inch (1") prior to the final
set of the concrete, shall be tooled in the sidewalks at intervals of five feet
(5'), and at ten feet (10') intervals in the curb and gutter. Where the sidewalk
adjoins the curb (parallel to it), contraction joints in the sidewalk and curb
shall be made to match where practicable.

Article 2.4 Measurement

Curb or integral curb and gutter shall be measured per linear foot along the face of the
curb. Mountable (rolled) curb and gutter shall be measured per linear foot along the gutter
line. Portland Cement Concrete (P.C.C.) Valley Gutter shall be measured along the
straight flow line between expansion joints "A and B" and from expansion joint "C" to the intersection of this flow line (point "D") as shown on the Standard Detail.

Medians with curb noses shall be measured as follows: P.C.C. curb and gutter per linear foot, curb noses including yellow paint, as units complete in place.

Parking stops shall be measured as units complete in place.

**Article 2.5  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described in this Section.

Payment shall be made under the following units unless otherwise specified:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>P.C.C. Curb and Gutter (Type)</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>P.C.C. Valley Gutter</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Curb Nose</td>
<td>Each</td>
</tr>
<tr>
<td>Parking Stop</td>
<td>Each</td>
</tr>
</tbody>
</table>

Extra payment will not be made for depressed curb or special sections.
SECTION 30.03 PORTLAND CEMENT CONCRETE SIDEWALKS

Article 3.1 Description

The Work covered under this Section consists of all Work necessary for the provision of Portland Cement Concrete sidewalks.

Article 3.2 Materials

The Portland Cement Concrete, joint filler, reinforcing steel, and curing materials shall conform to Section 30.01, Article 1.3 - Materials. Concrete mix for sidewalks shall conform to the requirements for Class A-3.

Article 3.3 Construction

A. Excavation and Embankment

Excavation and embankment for sidewalks shall be as described in Division 20 - Earthwork. Where directed by the Engineer, unsuitable material in the subgrade shall be removed to a specific depth and then backfilled with classified fill. Payment will not be allowed for excavation below grade or for backfill materials required when such excavation is caused by negligence of the Contractor.

Embarkment shall be compacted to ninety-five percent (95%) maximum density in accordance with Division 20, Section 20.01, Article 1.5 - Compaction Standards. In areas that are inaccessible to normal compaction equipment, approved tampers shall be used.

Before the forms are set, the subgrade shall be graded to within one inch (1") of established grade and the area between the sidewalk and the adjacent private property line shall be shaped to line, grade, and section shown on the Drawings.

B. Forms and Fine Grading

Forms shall conform to requirements outlined in Section 30.02 - Portland Cement Concrete, Curb and Gutter, and Valley Gutter. Wood forms against unexposed concrete surfaces shall be No. 2 Common Lumber or better. Those against surfaces to be exposed shall be dressed and matched boards of uniform thickness, and widths not exceeding ten inches (10"). Rigid, nonporous and waterproof sheet material may be used provided the end result will be a smooth unmarked concrete surface without waves, fins or other noticeable markings.

Plywood conforming to the requirements for form work, as set forth by the American Plywood Association, may be used against both exposed and unexposed concrete surfaces. This plywood shall be not less than five (5) ply and at least nine-sixteenths inch (9/16") thick. Low areas in the subgrade shall be backfilled with classified fill or with suitable native material as directed by the Engineer. The backfill shall then be compacted to ninety-five percent (95%) maximum density and
any dry areas in the subgrade shall be thoroughly dampened prior to the time the concrete is placed. No payment will be made for water, and the work of placing and cost thereof shall be considered as incidental to the construction of the concrete sidewalk.

C. Placing and Finishing Portland Cement Concrete Sidewalk

The concrete shall be spread uniformly between the forms and thoroughly compacted with a steel shod strikeboard. After the concrete has been thoroughly compacted and leveled, it shall be floated with wood floats and finished at the proper time with a steel float. Joints shall be edged with a one-quarter inch (1/4") radius edger and the sidewalk edges shall be tooled with a one-half inch (1/2") radius edger. After final troweling, sidewalk on grades of less than six percent (6%) shall be given a fine hair broom finish applied transversely to the centerline. On grades exceeding six percent (6%), walk shall be finished by hand with a wood float. Walk shall be remarked as necessary after final finish to assure neat uniform edges, joints, and score lines. Unsightly, poorly finished, and sidewalk failing to meet the requirements of the Drawings, Specifications, and this Section will be rejected.

The sidewalk shall be divided into panels by scoring one inch (1") deep every five feet (5’). Refer to Section 30.02, Article 2.3, SubArticle E - Expansion and Contraction Joints for requirements for contraction and expansion joints. The expansion joints shall be placed at all structures such as catch basins and manholes, at driveways, and at all points of tangency and points of curvature.

Additional requirements for placing and finishing concrete in cold weather shall be as outlined in Section 30.01, Article 1.9 - Weather Limitations.

For all other exposed aggregate concrete sidewalks, Contractor shall float and trowel all surfaces to receive the exposed aggregate finish. Seeding the surface with aggregate shall not be allowed. After the concrete has taken its initial set, the surface aggregate shall be exposed using a wager fog spray and brooms to remove the surface matrix. The coarse surface aggregate shall be exposed very lightly, approximately one-sixteenth inch (1/16”). After the concrete has taken its final set, a weak acid wash shall be applied to clean and wash the exposed aggregate surfaces. The weak acid wash shall be thoroughly neutralized and flushed from the finished surface. Under no circumstances shall Contractor allow the acid wash to enter the storm drain lines.

Contractor shall protect adjacent construction, plantings, finishings, structures, and the public from damage and harm due to the acid wash. The finished appearance of the exposed aggregate concrete sidewalk shall produce an appearance and texture that matches the adjacent exposed aggregate sidewalk. Any significant difference in texture or appearance between two adjacent concrete panels, as determined by the Engineer, shall result in removal and replacement of concrete panels by Contractor at no additional cost.
Contractor shall provide a two foot by two foot (2’ x 2’) exposed aggregate concrete test panel prior to constructing the exposed aggregate concrete sidewalk. Location of the test panel will be on-site as approved by the Engineer. Notification of providing this test panel shall be made to the Engineer no less than 24 hours prior to making the test panels to allow the Engineer and materials analysis personnel to be present. The Engineer may require the Contractor to provide additional panel(s) if the test panel does not produce an appearance that matches the adjacent exposed aggregate sidewalk.

Providing the test panel and any other required test panel shall be considered incidental to the bid item “P.C.C. Sidewalk 4” Thick (Exposed Aggregate)” and no separate payment shall be made.

D. Curing and Protection

The materials and procedures outlined in Section 30.02, Article 2.3 - Construction, shall prevail. The curing agent shall be applied immediately after finishing and be maintained for a period of seven (7) days. The curing agent(s) and/or concrete mixtures shall in no way deter or prevent final finishing of concrete. The use of surface retarders may be permitted if application methods are accepted by the Engineer, in writing, no less than twenty-four (24) hours prior to concrete placement.

The Contractor shall have readily available sufficient protective covering, such as waterproof paper or plastic membrane, to cover the pour of an entire day in event of rain or other unsuitable weather.

The sidewalk shall be protected against damage or defacement of any kind until it has been accepted by the Owner. Sidewalk which is not acceptable to the Engineer because of damage or defacement shall be removed and replaced at the expense of the Contractor.

Additional requirements for curing in cold weather shall be as outlined in Section 30.01, Article 1.9 - Weather Limitations.

Article 3.4 Measurement

Sidewalk: Sidewalks shall be measured per square yard, complete in place, for both four (4”) and six inch (6”) thicknesses.

Sidewalk Retaining Walls: Sidewalks of specified thickness constructed in conjunction with sidewalk retaining walls constructed in accordance with Section 30.05 – Structures and Retaining Walls, shall be measured per square yard from the face of retaining wall, accepted in place.
Article 3.5  Basis of Payment

Payment for this item shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>P.C.C. Sidewalk (Thick) (Type Finish)</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 30.04 PORTLAND CEMENT CONCRETE CURB RAMPS

Article 4.1 General

The Work under this Section consists of performing all operations pertaining to furnishing and constructing Portland Cement Concrete curb ramps with a detectable warning surfacing in conformance with the Drawings. The ramps shall comply with the Americans with Disabilities Act Title II as identified in 28 CFR Part 35 – Nondiscrimination on the Basis of Disability in State and Local Government Services.

Article 4.2 Materials

A. General

The Portland Cement Concrete (P.C.C.) materials and installation shall conform to the requirements of Section 30.03 - Portland Cement Concrete Sidewalk and the Drawings. The P.C.C. shall have a slump range of four to seven inches (4” to 7”) to permit solid placement of the tactile warning panel. An overly wet mix will cause the tactile warning panel(s) to float. Contractor shall not add color to the concrete unless specified in the Drawings.

The Contractor shall not apply a troweled pattern to the curb ramps. Contractor shall provide a coarse broom finish for the ramps perpendicular to direction of pedestrian traffic.

B. Detectable Warning Panel

Each detectable warning panel shall have a truncated domed surface twenty-four inches (24”) in depth for the width of the ramp. The truncated domes shall have a height of two-tenths inch (0.2”), a diameter of nine-tenths inch (0.9”), a center-to-center spacing of one and six-tenth inches (1.6”) minimum and two and four-tenth inches (2.4”) maximum, and a base-to-base spacing of sixty-five one-hundredth inch (0.65”), measured between the most adjacent domes.

Contractor shall provide panels brick red in color, or approved equal. The specified color shall be homogeneous throughout the panel.
Contractor shall provide Armor Tile Cast-In-Place In-Line Dome Tactile Panel detectable warning panel(s), manufactured by:

Engineered Plastics, Inc.  
300 International Drive, Suite 100  
Williamsville, NY 14221  
Phone: 1-800-682-2525  

Local Contact:  
Polar Supply Company, Inc.  
300 E. 54th Avenue  
Anchorage, Alaska 99518-1230  
Phone: 907-563-5000  
Fax: 907-562-7001

or a gray cast iron detectable warning plates, or panels(s), manufactured by:

Neenah Foundry Company, Inc.  
4107 E. Madison Street  
Phoenix, AZ 85034  
Phone: 1-866-765-0850  

Local Contacts:  
East Jordan Iron Works, Inc.  
301 Spring Street  
East Jordon, MI 49727  
Phone: 1-800-626-4653

or an approved equal.

**Article 4.3 Construction**

The Contractor shall construct each curb ramp and install the detectable warning panel(s) in conformance with the Contract Documents and the manufacturer's recommendations.

No later than five (5) days prior to construction of the curb ramps, Contractor shall submit to the Engineer for review and approval, a layout drawing for each curb ramp to resolve issues related to pattern repeat, tile cuts, expansion joints, control joints, ramp curves, ramp end returns and surface interfaces, and truncated dome spacing.

Contractor shall install and finish the P.C.C. in accordance with the Contract Documents prior to installation of the detectable warning panel(s). Contractor shall tamp the plate(s) or panel(s) with a small sledge hammer with a two inch by six inch by twenty inch (2” x 6” x 20”) wood tamping plate, or lightly vibrate into the fresh concrete to ensure that the panel’s field level (base of truncated dome) is flush with the adjacent concrete and top back of curb. Contractor shall ensure that the panel’s field level is flush with the adjacent concrete surface, proper water drainage is provided, and potential tripping hazards are eliminated. Contractor shall ensure that the back edge of the detectable warning panel(s) form a smooth arc and is parallel to the top back of the curb.
Immediately after the panel placement, Contractor shall check and adjust accordingly the panel’s or plate(s) field level to be flush with the adjacent concrete surface. Following final field-level adjustment(s), place suitable twenty-five (25) pound weights, conforming to the manufacturer’s recommendations, on each panel and additional weights at panel-to-panel joints as necessary to provide a solid contact between the panel underside and the concrete.

During and after the panel installation and concrete curing time, Contractor shall ensure that there is no walking, leaning, or any external forces placed on the panel, thereby causing a void between the underside of the panel and the concrete.

After the concrete has cured, Contractor shall remove protective plastic wraps. If “concrete bleeding” occurs between the panels, Contractor shall remove the residue without damage to the panel surfaces, in accordance with the manufacturer’s recommendation.

Contractor shall maintain, on-site, an electronic level, a five foot (5’) diameter circle template, and a three foot by five foot (3’x5’) rectangular template. Template may be of any material, including paper. Contractor shall, when requested, demonstrate to the Engineer that there are adequate landing and turning areas that meet the dimensions and slopes required on the Drawings.

Backfill and grade areas disturbed by curb ramp construction and restore ground surface as shown on Drawings.

**Article 4.4 Tolerances**

In accordance with the Americans with Disabilities Act Public Rights-of-Way Accessibility Guidelines (PROWAG), dimension not stated as “maximum” or “minimum” are absolute. All dimensions are subject to conventional industry tolerances, except where the requirement is stated as a range with specific minimum and maximum end points.

Conventional industry tolerances recognized by the ADAAG include those for field conditions that may be a necessary consequence of a particular manufacturing process. Information on specific tolerances may be available from industry or trade organizations, code groups, building officials, and published references. (Example: American Concrete Institute Standard Specifications for tolerances for concrete construction and materials (ACI-117)).

**Article 4.5 Measurement**

The Work paid for under “P.C.C. Curb Ramp” shall be measured by the actual horizontal square yardage of curb ramp (including curb ramp under detectable warnings) and back curb furnished, constructed, finished, and accepted in place.

The Work paid for under “Detectable Warnings” is measured by the actual horizontal square footage of detectable warning tiles furnished, installed, and accepted in place.
Article 4.6  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for restoration of existing ground surface disturbed by curb ramp construction is made under “A.C. Pavement,” “P.C.C. Sidewalk,” “Topsoil,” and “Seeding,” as applicable. No separate payment is made for backfilling and grading in preparation of paid surface treatment. No separate payment is made for backfilling and grading in locations where the existing surface is gravel.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>P.C.C. Curb Ramp</td>
<td>Square Yard</td>
</tr>
<tr>
<td>Detectable Warnings</td>
<td>Square Foot</td>
</tr>
</tbody>
</table>
SECTION 30.05 STRUCTURES AND RETAINING WALLS

Article 5.1 Description

The Work under this Section consists of the construction of Portland Cement Concrete structures and retaining walls.

Article 5.2 Construction

A. Reinforcing Steel

Reinforcing bars shall be bent cold and shall conform accurately to the shape and dimensions shown on the diagram. Bent-up bars, unless otherwise specified, shall be bent up at an angle of forty-five degrees (45°). In no case shall the radius of any bend be less than four (4) times the diameter of the bar.

The reinforcement shall be positioned as indicated on the Drawings or as hereinafter specified. It shall be rigidly blocked and wired in place, using metal supports or concrete blocks and securely tied at each intersection with annealed iron wire of at least twelve (12) gauge.

Splicing bars at points not indicated on the Drawings will not be permitted except as an emergency measure and with the consent of the Engineer. Such splices shall be at the points of minimum tensile stress and the lap shall be not less than thirty-six (36) bar diameters.

Bar lists and Bending schedules shall be furnished by the Contractor for approval of the Engineer. Materials shall not be ordered until such lists and bending diagrams have been approved by the Engineer. The approval of order lists and bending diagrams will in no way relieve the Contractor of responsibility for the correctness of such lists and diagrams. Any expense incidental to the revision of material furnished in accordance with such lists and diagrams, to make it comply with the design drawings, shall be borne by the Contractor.

B. Forms

Forms shall conform to the shape and dimensions shown on the Drawings and shall be accurately set to line and grade. All sheeting in contact with concrete surfaces shall be matched tongue and groove lumber, sized to uniform thickness and free from wane, warp, splits, loose knots or other defects which will prevent obtaining a smooth, tight form. Forms for exposed surfaces shall be lined with plywood conforming to the requirements for form plywood as specified by the American Plywood Association. All plywood lining shall be used in as wide pieces as possible. Areas less than four feet (4’) in width shall be lined with a single width of plywood. Joints in lining and backing shall not occur at the same place and the abutting edges of adjacent sheet shall be nailed to the same board.
Joints in the lining shall be filled with cold water putty, patching plaster, plastic wood, or other plastic filler satisfactory to the Engineer. Lining material may be re-used if it is in satisfactory condition, well cleaned, re-oiled, and if specific permission from the Engineer is obtained for each separate operation.

All forms shall be securely tied with bolts or rods in such manner that after stripping, such bolts or rods may be either entirely removed or may be removed for a distance at least one inch (1") below the concrete surface. Such bolts or rods shall be threaded and provided with nuts to prevent slipping and to provide adjustments. No wire ties or clamping devices shall be permitted.

Forms for walls, etc., shall have large cleanout openings at their lowest points, which shall not be closed until just before placing concrete. All forms shall be thoroughly cleaned and soaked with water immediately before filling.

Weep drains shall be provided by the Contractor and installed as shown on the Drawings.

C. Placing

Concrete shall be placed by means of a bottom dumping bucket, cart, concrete chute, or tremie. At no time shall concrete have a free fall or more than three feet (3’). The concrete shall be brought up in approximately horizontal layers. The concrete shall be placed continuously from one side or end of the section to the other, using precaution to put the full load upon any given area of form as rapidly as possible. The rate of delivery of concrete to the work shall be such as to insure continuity of placement. No partially completed surface shall be allowed to stand more than forty-five (45) minutes before continuing the placing of concrete thereon.

D. Compacting Concrete

All concrete shall be thoroughly spaded, especially along the forms, to prevent the formation of gravel pockets and to permit the escape of trapped air. In addition to spading, the Contractor shall also provide suitable internal vibrating tampers of the type designed to be placed directly in the concrete. Vibration shall be such that the concrete becomes uniformly plastic. Vibrators shall be inserted to a depth sufficient to vibrate the bottom of each layer effectively, but shall not be allowed to penetrate partially hardened concrete which will not become plastic under the vibrator action, nor shall the vibrator be applied directly to steel which extends into partially hardened concrete. Placing of concrete shall not commence until the vibrator is on the job site and the mechanical efficiency of the vibrator is proven in the presence of the Engineer.

E. Finishing Concrete

All irregularities on exposed surfaces such as gravel pockets, bolt holes, etc., shall be neatly painted with mortar of the same proportions as used in the concrete. The
surface film of all such patches shall be rubbed off after initial setting has taken place.

All exposed surfaces shall, after irregularities have been corrected, be thoroughly wetted and brushed with a grout composed of one (1) part fine sand and one (1) part cement. It shall then be kept wet for forty-eight (48) hours.

All outside edges are to have three-quarter inch (3/4") chamfered corners, unless noted otherwise. They shall be obtained by putting a triangular strip of wood in the corners of the forms.

F. Joints, Horizontal and Vertical

Joints shall be made in walls or structures as indicated on the Drawings or where directed by the Engineer. If not indicated on the Drawings, the maximum distance between contraction joints shall be twenty-five feet (25’), and the maximum distance between expansion joints shall be fifty feet (50’). Where expansion joints are required, one-half inch (1/2") precast expansion joint material conforming to the requirements of ASTM D-1751 (AASHTO M-213) shall be used. The walls shall be poured one section at a time. The joint material shall be accurately cut to fit the bulkhead between sections, and nailed to the bulkhead with 6d nails. These nails shall be driven into the lumber only enough to hold the material in place, the heads being embedded in the concrete. The bulkheads between sections shall be removed not sooner than twelve (12) hours after the concrete has been placed and the nail points clinched into the joint material.

At the surface of the wall, unless otherwise shown, the joint shall end in a V-shaped groove, two inches (2") wide and one inch (1") deep. Unless noted otherwise, all joints will be truly horizontal or truly vertical.

G. Removal of Forms

Unless otherwise directed by the Engineer, forms may be removed from structures and retaining walls, which are not subject to supporting loads, after a period of three (3) days. All other forms shall be removed upon approval of the Engineer.

H. Curing

All exposed surfaces shall be covered by layers of absorptive burlap, mats or other approved material to a thickness weighing not less than fourteen (14) ounces per square yard, immediately after finishing. The cover material shall be kept saturated with clean water for a minimum initial curing period of twenty-four (24) hours.

Following this initial curing, the concrete shall be completely covered with a plastic waterproof membrane, or equal, for an additional six (6) days. Liquid spray type waterproof membrane will not be considered a satisfactory material for curing concrete used for retaining walls or structures.
I. Waterproofing

Unless otherwise specified, the back side of retaining walls and backfilled surfaces of other structures shall be painted with two (2) coats of a cold bituminous waterproof coating which shall be approved by the Engineer prior to application. Application shall be with a stiff masonry type brush, or as recommended by the manufacturer.

J. Earthwork

All earthwork involved in the construction of retaining walls shall be constructed as specified in Division 20 - Earthwork.

Excavation for retaining walls and structures shall be to the limits shown on the Drawings. Where limits are not indicated, excavation shall be sufficient to properly construct the work.

Where directed by the Engineer, unsuitable material in the subgrade shall be removed to a specific depth and then backfilled with classified fill.

The subbase under footings shall be compacted to ninety-five percent (95%) density at optimum moisture content in accordance with Section 30.01, Article 1.5 - Subbase.

Backfilling the lower portion of structures and retaining walls shall not begin until fourteen (14) days after concrete placement. Where High-Early Strength Cement is used, backfilling the lower portion of structures and retaining wall shall not begin until four (4) days after concrete placement. Waterproofing shall not be left exposed for longer than twelve (12) days before backfill. Where drain holes or drain tile are involved, or where the Engineer may direct, a filter fabric shall be used in conjunction with a six inch (6") layer of coarse gravel which shall be spread and compacted around the drains in such a manner that earth will not clog the voids in the gravel. The remainder of the backfill shall be classified fill placed in layers not exceeding one foot (1') in thickness. Each layer shall be thoroughly rammed with a rammer not more than ten inches (10") in diameter and weighing not less than forty (40) pounds, or with an approved mechanical tamper. Unsuitable earth or vegetable matter shall not be used as backfill. Filling with loose earth and hydraulic jetting will not be allowed except by written permission of the Engineer. Imported material for backfill shall conform to Division 20, Section 20.21 - Classified Fill and Backfill.

Article 5.3 Measurement

Concrete shall be measured per cubic yard for structures and retaining walls. Measurement will be determined from the neat line dimension. In the case of minor field changes such as changing the length, height, etc., payment shall be based on the same unit price per cubic yard as bid. Measurement shall be determined by the Engineer and Contractor prior to placing concrete. Where sidewalks are constructed in conjunction with
retaining walls, the sidewalk shall be measured under the provisions of Section 30.03 – Portland Cement Concrete Sidewalks.

**Article 5.4   Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section. Extra payment will not be made to the Contractor if he places additional concrete over and above the neat line volume to facilitate his operation and save on form work.

No additional payment shall be made for excavation and backfill around retaining walls and structures. Payment for disposal of unusable excavation shall be in accordance with Division 20, Section 20.10 – Excavation for Traffic Ways. Payment for classified fill shall be made in accordance with Division 20, Section 20.21 – Classified Fill and Backfill.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>P.C.C. Structure/Retaining Wall (Class)</td>
<td>Cubic Yard</td>
</tr>
</tbody>
</table>
SECTION 30.06 CONCRETE MASONRY RETAINING WALLS

Article 6.1 Description

The Work covered under this Section consists of the construction of concrete masonry walls on a reinforced, poured Portland concrete footing. This Work includes all excavation, classified fill or backfill, and mechanical compaction.

Article 6.2 Construction

A. Reinforcing Steel

Reinforcing steel shall conform to Section 30.05, Article 5.2 - Construction.

B. Survey

The Contractor shall set control stakes for the footings. The stakes will be set at breaks in grade and on even grade intervals not to exceed fifty feet (50'). They will be marked with the station offset, and the cut or fill to the top of the footing.

C. Forms

Forms may be of wood, metal, or any other material at the option of the Contractor, provided that the forms as set will result in a footing in accordance with thickness, cross section, grade, and alignment as required by the Contract Documents.

Forms may be removed after a period of three (3) days, unless otherwise directed by the Engineer.

D. Excavation and Embankment

Excavation and embankment for Masonry Retaining Walls shall be as described in Section 30.05, Article 5.2, SubArticle J - Earthwork.

E. Constructing Masonry Wall

The Contractor shall omit the head joint in the first course every thirty-two inches (32") O.C. to provide for weep holes.

All cells of the concrete masonry retaining wall shall be filled solid with grout. The grout shall develop a maximum compressive strength of 2,000 P.S.I. in 28 days. The grout mix shall be of a fluid consistency and mixed in the ratio by volume of one (1) part Portland cement, three (3) parts concrete sand, and two (2) parts No. 8 Coarse Aggregate. The concrete sand is to conform to the gradation requirements of AASHTO M-6. The coarse aggregate is to comply with gradation requirements as follows:
Coarse Aggregate Gradation
Requirements

<table>
<thead>
<tr>
<th>Sieve Size</th>
<th>Percent Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>1/2&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>85-100</td>
</tr>
<tr>
<td>#4</td>
<td>10-30</td>
</tr>
<tr>
<td>#8</td>
<td>0-10</td>
</tr>
<tr>
<td>#16</td>
<td>0-5</td>
</tr>
</tbody>
</table>

Mortar shall be freshly prepared and uniformly mixed in the ratio by volume of one (1) part cement, one-half (1/2) part lime putty, and four and one-half (4-1/2) parts sand and shall conform to requirements of ASTM C-270.

The use of admixtures shall not be permitted in mortar or grout unless substantiating data is submitted and approved by the Engineer.

Masonry units shall be Grade A Units, conforming to the requirements of ASTM C-90. These units shall be sound, dry, clean, and free from cracks when placed in the structure. Where masonry unit cutting is necessary, all cuts shall be neat and true.

Portland Cement Concrete used in footings, shall conform to Section 30.01, Article 1.3 - Materials.

The starting joint on the footing shall be laid with full mortar coverage on the bed joint. All mortar joints shall be straight, clean and uniform in thickness and shall be tooled.

Tooling shall be done with a round bar to produce a dense, slightly concave surface well bonded to the block at the edges. Tooling shall be done when the mortar is partially set but still sufficiently plastic to bond. All tooling shall be done with a tool which compacts the mortar, pressing the excess mortar out of the joint rather than dragging it out. Joints which are not tight at the time of tooling shall be raked out, pointed, and then tooled.

If it is necessary to move a unit after it has been set in place, the unit shall be removed from the wall, cleaned and set in fresh mortar.

All capping units shall be set in a full bed of mortar.
F. Curing and Waterproofing

Curing and waterproofing of concrete masonry retaining walls shall conform to the specifications for structures and retaining walls in Section 30.05, Article 5.2, SubArticles H - Curing, and I - Waterproofing.

Article 6.3 Measurement

The item concrete masonry retaining walls shall be on a lump sum basis, complete in place and no measurement of quantities shall be made.

Article 6.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Concrete Masonry Retaining Walls</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 30.07 CONCRETE – BUILDING STRUCTURES

Article 7.1 Description

The Work covered in this Section consists of providing all operations pertaining to the mixing and placement of Portland Cement Concrete.

Article 7.2 Materials

A. Portland Cement Concrete, reinforcing steel, curing materials, and miscellaneous concrete Work shall conform to the following paragraphs of Section 30.01, Article 1.3 - Materials:

1. Reinforcing Steel and Steel Dowels
   See Section 30.01, Article 1.3, SubArticles A – Reinforcing Steel and B – Welded Steel Wire Fabric.

2. Preformed Expansion and Dummy Joint Filler
   See Section 30.01, Article 1.3, SubArticle H – Expansion Joints.

3. Curing Compounds
   See Section 30.01, Article 1.3, SubArticle G – Curing Materials.

B. Forms

Forms may be of wood or metal or any other material at the option of the Contractor, provided that the forms as set will result in configurations, dimensions, and proper finish in accordance with the Drawings.

Forms may be removed on the day following pour if the concrete is sufficiently set so that removal can be accomplished without danger of chipping or spalling. All forms shall be cleaned, oiled, and be examined for defects before they are used again.

Article 7.3 Formwork Construction

A. Inspection

Verify lines, levels, and measurements before proceeding with formwork.

B. Preparation

Earthforms - Hand-trim sides and bottoms of earth forms; remove loose dirt prior to placing concrete.
Wood and Prefabricated Forms - Minimize form joints. Symmetrically align joints and make watertight to prevent leakage of mortar for exposed concrete. Arrange and assemble formwork to permit dismantling and stripping so that concrete is not damaged during its removal. Arrange forms to allow stripping without removal of principal shores, where required to remain in place.

C. Erection

Provide bracing to ensure stability of formwork. Strengthen formwork liable to overstressing by construction loads. Camber slabs and beams to achieve ACI 301 tolerances. Provide temporary ports in formwork to facilitate cleaning and inspection. Locate openings at bottom of forms to allow flushing water to drain. Close ports with tight-fitting panels, flush with inside face of forms, neatly fitted so that joints will not be apparent in exposed concrete surfaces. Provide chamfer strips on external exposed corners of beams, joists, and columns. Construct formwork to maintain tolerances in accordance with ACI 301.

D. Application of Form Release Agent

Apply form release agent on formwork in accordance with manufacturer's instructions. Apply prior to placing reinforcing steel, anchoring devices, and embedded items. Do not apply form release agent where concrete surfaces are scheduled to receive special finishes which may be affected by agent. Soak contact surfaces of untreated forms with clean water. Keep surfaces wet prior to placing concrete.

E. Inserts, Embedded Parts, and Openings

Provide formed openings where required for Work embedded in, or passing through, concrete. Coordinate Work of other sections in forming and setting openings, slots, recesses, chases, sleeves, bolts, anchors, and other inserts. Install accessories in accordance with manufacturer's instructions, level, and plumb. Ensure items are not disturbed during concrete placement.

F. Form Removal

Do not remove forms, shoring, and bracing until concrete has sufficient strength to support its own weight, and construction and design loads which may be imposed upon it. Remove load-supporting forms only when concrete has attained seventy-five percent (75%) of required 28-day compressive strength provided construction is reshored. Reshore structural members due to design requirements or construction conditions to permit successive construction. Remove formwork progressively so no unbalanced loads are imposed on structure. Do not damage concrete surfaces during form removal. Store reusable forms for exposed architectural concrete to prevent damage to contact surfaces. Remove formwork in same sequence as concrete placement to achieve similar concrete surface coloration.
G. Cleaning and Tightening

Thoroughly clean forms and adjacent surfaces to receive concrete as erection proceeds and just before concrete placement. Remove chips, wood, sawdust, dirt, and other debris and ensure that water and debris drain from formwork through cleanout ports.

During cold weather, remove ice and snow from forms. Do not use deicing salts. Unless within a heated enclosure, do not use water to clean out forms. Use compressed air to remove foreign matter.

Retighten forms immediately after concrete placement as required to eliminate mortar leaks.

Article 7.4 Concrete

A. Placing Concrete

Before placing concrete, ensure reinforcing is clean, free of loose scale, dirt, or other foreign coatings which would reduce bond to concrete. Ensure that reinforcing is supported and secured against displacement. Do not deviate from true alignment.

Place concrete in accordance with ACI 304.

Notify the Engineer a minimum of twenty-four (24) hours prior to commencement of concreting operations.

Ensure anchors, seats, plates, and other items to be cast into concrete are placed, held securely, and will not cause hardship in placing concrete. If problems are found, rectify same and proceed with Work.

Maintain records of poured concrete items. Record date, location of pour, quantity, air temperature, and test samples taken.

Ensure reinforcement, inserts, embedded parts, formed expansion and contraction joints are not disturbed during concrete placement.

A bonding agent is to be used where pouring against previously placed concrete. Submit manufacturer's product data for the bonding agent to the Engineer for approval. Surface preparation is to be in accordance with the manufacturer's recommendations for the approved product.

Pour concrete continuously between predetermined construction and control joints.

Pour slabs-on-grade in checkerboard pattern or provide control joints to form panels or patterns as shown on the Drawings. If not shown on the Drawings, control joints shall be made by sawing or by approved inserts. Saw cut control joints within twenty-four (24) hours after finishing and use a three-sixteenths inch
(3/16") thick blade, cutting one-fourth (1/4) of slab thickness. Unless otherwise specified or approved, control joint spacing shall not exceed fifteen feet (15’).

In locations where new concrete is to be dowelled to existing work, lay down dowels as shown on the Drawings, drill holes in existing concrete and embed dowels according to the recommendations of the manufacturer of the approved bonding agent. Submit manufacturer's product data for the bonding agent to the Engineer for approval.

Excessive honeycomb or embedded debris in concrete is not acceptable and the Contractor shall notify the Engineer upon discovery.

Conform to ACI 306 when concreting during cold weather.

Install vapor barrier under interior slabs on grade. Lap joints minimum six inches (6") and seal. Do not disturb or damage vapor barrier while placing concrete reinforcing. If damage does occur, repair areas before placing concrete. Use vapor barrier material, lapped over damaged areas a minimum of six inches (6") and seal.

Screed slabs-on-fill, maintaining surface flatness of maximum one-eighth inch (1/8") in ten feet (10’).

B. Patching

Notify the Engineer twenty-four (24) hours in advance to allow inspection of concrete surfaces immediately upon removal of forms. Patch imperfections as directed.

C. Defective Concrete

Modify or replace concrete not conforming to required lines, details, and elevations.

Repair or replace concrete not properly placed resulting in excessive honeycombing and other defects. Do not patch, fill, touchup, repair, or replace exposed concrete except upon express direction of the Engineer for each individual area.

D. Concrete Finishing

Provide standard form finish to all concrete formed surfaces that are to be concealed in the finish work or by other construction.

E. Floor Finishing

Finish concrete floor surfaces in accordance with ACI 302 and ACI 304.

Uniformly spread, screed, and float concrete. Do not use grate tampers or mesh rollers. Do not spread concrete by vibration.
Maintain surface flatness, with maximum variation of one-eighth inch (1/8") in ten feet (10').

In areas with floor drains, maintain floor levels at walls and pitch surfaces uniformly to drains at one-eighth inch (1/8"/ft) per foot, or as directed by the Engineer.

F. Curing and Protection

Beginning immediately after placement, protect concrete from premature drying, excessively hot or cold temperatures, and mechanical injury. Maintain concrete with minimal moisture loss at relatively constant temperature for period necessary for hydration of cement and hardening of concrete.

Article 7.5 Measurement

Payment for this Work shall be in accordance with Division 10, Section 10.07 – Measurement and Payment and shall include full payment for all Work described in this Section.

Article 7.6 Basis of Payment

Unit Cost shall be made on the following basis unless otherwise specified:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Floor Slabs - on grade</td>
<td>C.Y.</td>
</tr>
<tr>
<td>Structural Slabs - on grade</td>
<td>C.Y.</td>
</tr>
<tr>
<td>Spread Footings</td>
<td>C.Y.</td>
</tr>
<tr>
<td>Continuous Footings</td>
<td>C.Y.</td>
</tr>
<tr>
<td>Concrete Walls (Inches Thick)</td>
<td>C.Y.</td>
</tr>
</tbody>
</table>
SECTION 30.08 PORTLAND CEMENT CONCRETE CLUSTER MAILBOX BASE

Article 8.1 General

The Work under this Section consists of performing all operations pertaining to furnishing and constructing a cluster mailbox base.

Article 8.2 Materials and Installation

Each cluster mailbox base shall be constructed in conformance with the Drawings. Materials and installation shall meet the requirements of Section 30.03 - Portland Cement Sidewalks and the Drawings.

Article 8.3 Construction

Contractor shall contact the Manager of the Address Management Department, Anchorage District, United State Post Office at 564-2846, forty-eight (48) hours prior to beginning construction of the cluster mailbox base to schedule the final placement of the cluster mailbox unit by the United State Post Office.

Article 8.4 Measurement

The Work shall be measured per each actual cluster mailbox base furnished, constructed, and accepted in place.

Article 8.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>P.C.C. Cluster Mailbox Base</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 30.09 CONCRETE PARKING BUMPERS

Article 9.1 Description

This Work item includes the forming, casting, and installation of concrete parking bumpers as shown on the Drawings.

Article 9.2 Material

Reference Section 30.01 - General Provisions.

Concrete class shall be A-3.

Article 9.3 Construction

Contractor shall construct precast concrete parking bumpers in accordance with applicable construction techniques specified in Section 30.02 - Portland Cement Concrete Curb and Gutter, and Valley Gutter. Parking bumpers shall be seven feet (7’) long and seven inches (7”) tall. Parking bumpers shall be ten inches (10”) wide at the base and taper to six inches (6”) wide at the top. Parking bumpers shall be firmly set in place with two (2) eighteen inch (18”) long #5 rebar.

Article 9.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Concrete Parking Bumpers</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 30.10  IMPRINTED COLORED CONCRETE

Article 10.1  General

The Work under this Section consists of providing all operations and materials necessary to install imprinted colored concrete. Installation of the imprinted colored concrete shall only be performed by a licensed contractor as certified by the manufacturer. Submit license and material information to the Engineer prior to placement of imprinted colored concrete. All Work shall be in accordance with this Section and shall be placed at the locations shown on the Drawings.

Article 10.2  Materials

A. General

The Portland Cement concrete and curing materials shall conform to Section 30.01, Article 1.3 - Materials, as modified below:

“Bomanite Running Bond Brick” available from Bomanite Corporation, Palo Alto, California, (415) 321-0718, or another concrete imprinting system of approved equal performance, shall be used.

B. Concrete

Concrete mix for imprinted colored concrete shall conform to Section 30.01, Article 1.6 – Mix Requirements for Classes of Concrete, requirements for Class AA-3, normal weight concrete.

C. Coloring Materials

Bomacrom Color hardener, heavy duty grade, available from Bomanite Corporation, shall be applied to the concrete in accordance with these specifications and manufacturer’s recommendations. The color shall be “Colonial Red.”

D. Curing Compound

Apply Bomanite curing compound available from Bomanite Corporation or approved equal in accordance with manufacturer’s recommendations.

E. Expansion Joints

Filler material shall be non-asphaltic material, one-half inch (1/2") wide and four inches (4") deep, with the top one inch (1") strippable. Primer shall be “Externaflex” 1993 or approved equal. Backer rod shall be closed cell polyethylene. Sealant shall be “Externaflex” two-compound urethane, “pour grade” No. 1071-200, or approved equal. Contractor shall submit the color of the sealant for Engineer’s approval prior to installation.
Article 10.3 Construction

A. General

The excavation and embankment, forms and fine grading, placing and finishing, curing and protection operations for imprinted colored concrete shall conform to Section 30.03, Article 3.3 – Construction as amended below:

Place imprinted colored concrete in dry weather with temperatures above forty degrees (40°) Fahrenheit.

B. Color Hardener Application

In Section 30.03, Article 3.3, SubArticle C - Placing and Finishing Portland Cement Concrete Sidewalks, in the first paragraph, delete the second, third and fourth sentences, and substitute the following:

After the concrete has been thoroughly compacted and leveled, it shall be floated with wooden floats.

Color Hardener shall then be applied evenly to the plastic surface by the dry-shake method using a minimum of sixty (60) pounds per one hundred (100) square feet. The hardener shall be applied in two (2) or more shakes, floated after each shake, and finished with a steel float.

While concrete is still in the plastic stage of set, the “Bomanite” imprinting tools shall be applied in conformance to the manufacturer’s specifications to make the desired patterned surface. The patterns shall be “Running Bond Brick.”

C. Color Curing Compound

Bomacrom Color Curing Compound, thinned in the proportion of one part cure to one part mineral spirits (paint thinner), shall be applied uniformly with a roller or sprayer immediately after imprinting. The coverage shall be approximately six hundred (600) to six hundred and fifty square feet per gallon (650 sf/gal) of unthinned curing compound.

At times when the air temperature is at or near freezing, the slab shall instead be cured using a suitable curing blanket, and if possible, the slab shall later be sealed with Bomanite Color Curing Compound at such time as the temperature is safely above freezing.

If, at any time during the curing period, any of the forms are removed, a coat of curing compound shall be applied immediately to the exposed surface. Additional coats shall be applied if the Engineer determines that the coverage is not adequate. The concrete shall be cured for the minimum period of time set forth below:
Type I/II Portland Cement Concrete  7 days
Type III Portland Cement Concrete  3 days

Article 10.4 Measurement

Imprinted colored concrete shall be measured per square yard, complete and in place.

Article 10.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Imprinted Colored Concrete</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 30.11 SIDEWALK JOINT SEALANT

Article 11.1 General

The Work under this Section consists of performing all operations pertaining to preparing and sealing joints in the sidewalk.

Article 11.2 Materials

Sealant materials shall be Elastomeric (in accordance with ASTM C-920) long lasting, and resist deterioration caused by weather, street, movement, traffic, and water. Sealant materials shall be approved by the Engineer prior to use. The sealant color shall closely match adjacent concrete sidewalk.

Article 11.3 Construction

Joint sealant shall be applied to joints with dimensions between one-half inch (1/2") and one inch (1") in width, as directed by the Engineer. The sealant shall be applied in conformance with the manufacturer's instructions and to full depth of the joint.

The joints shall be cleaned and all loose material shall be blown out to the full depth of the joint. The interior of the joint shall be completely dried and existing sidewalk surfaces shall be exposed.

Article 11.4 Measurement

The application of joint sealant shall be measured per linear foot of sealant applied as directed by the Engineer.

Article 11.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Joint Sealant</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
STANDARD CONSTRUCTION SPECIFICATIONS  
MISCELLANEOUS  
DIVISION 30  
INDEX OF STANDARD DETAILS

30-1  Curb and Gutter Cross Sections
30-2  Curb and Gutter Termination Transition
30-3  Typical Section - Valley Gutter
30-4  Typical Section - Median/Curb Nose
30-5  Steel Curb Facing
30-6  Driveway Curb-cut with Attached Sidewalk
30-7  Driveway Curb Return with Attached Sidewalk
30-8  Perpendicular Curb Ramp
30-9  Parallel Curb Ramp
30-10 Accessible (Type 1A/2A) Curb and Gutter Sections
30-11 Curb Ramp Clearances
30-12 Pedestrian Push Button Access
30-13 Curb Type Retaining Wall - 2' to 3'
30-14 Sidewalk Retaining Wall - 6" to 24"
30-15 Sidewalk Retaining Wall - 2' to 5'
NOTE:
TROWEL BOTH FRONT AND BACK EDGES OF THE CURB & GUTTER TO A RADIUS OF ONE-HALF (1/2) INCH.
NOTE:
VALLEY GUTTER SHALL BE PAID PER LINEAR FOOT UNDER BID ITEM "VALLEY GUTTER". MEASURE LENGTHS ALONG THE STRAIGHT FLOW LINE FROM EXPANSION JOINTS "A" AND "B" TO POINT "D" AND ON BOTH SIDES FROM EXPANSION JOINT "C" TO THE POINT "D". THE STREET SECTION SHALL BE MEASURED FROM POINT "D" TO POINT "D". IF P.C.C., THE STREET SECTION SHALL BE INCLUDED UNDER BID ITEM "VALLEY GUTTER". IF ASPHALT, THE STREET SECTION COST SHALL BE INCLUDED UNDER THE APPROPRIATE PAVING BID ITEM.
NOTE:
PAINT MEDIAN AND CURB NOSE WITH YELLOW TRAFFIC PAINT, THE PAINT SHALL BE CONSIDERED INCIDENTAL TO THE BID ITEM "CURB NOSE".
1/4" x 7" PLATE BENT TO MATCH RADIUS

P.C.C. CURB & GUTTER, AS SPECIFIED IN THE DRAWINGS

1/2" Ø STUDS @ 12" O.C.

NOTES:
1. BEND LEADING AND TRAILING EDGES 90° AND EMBED IN CONCRETE 1" MINIMUM.
2. STEEL CURB FACING SHALL BE INSTALLED ONLY IN LOCATIONS WHERE THERE IS A CURVILINEAR TRANSITION TO THE CURB FACE SUCH AS IN A CURB BULB OR TRANSIT PULL-OUT.
**TYPICAL DRIVEWAY ENTRANCE**

*(WITH ATTACHED SIDEWALK)*

<table>
<thead>
<tr>
<th>STREET RUNNING SLOPE</th>
<th>MINIMUM UPHILL RAMP LENGTH &quot;L&quot;</th>
<th>MINIMUM DOWNHILL RAMP LENGTH &quot;L&quot;</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.0% TO 0.5%</td>
<td>6.0'</td>
<td>6.0'</td>
</tr>
<tr>
<td>&gt; 0.5% TO 1.6%</td>
<td>7.0'</td>
<td>6.0'</td>
</tr>
<tr>
<td>&gt; 1.6% TO 2.4%</td>
<td>8.0'</td>
<td>5.0'</td>
</tr>
<tr>
<td>&gt; 2.4% TO 3.1%</td>
<td>9.0'</td>
<td>5.0'</td>
</tr>
<tr>
<td>&gt; 3.1% TO 3.6%</td>
<td>10.0'</td>
<td>5.0'</td>
</tr>
<tr>
<td>&gt; 3.6% TO 4.0%</td>
<td>11.0'</td>
<td>4.0'</td>
</tr>
<tr>
<td>&gt; 4.0% TO 4.4%</td>
<td>12.0'</td>
<td>4.0'</td>
</tr>
<tr>
<td>&gt; 4.4% TO 4.7%</td>
<td>13.0'</td>
<td>4.0'</td>
</tr>
<tr>
<td>&gt; 4.7% TO 5.0%</td>
<td>14.0'</td>
<td>4.0'</td>
</tr>
<tr>
<td>&gt; 5.0%</td>
<td>15.0'</td>
<td>4.0'</td>
</tr>
</tbody>
</table>

* MAXIMUM ALGEBRAIC DIFFERENCE IS 8% ON COMMERCIAL/INDUSTRIAL DRIVEWAYS.
NOTES:
1. SIZE LANDING TO MEET ADA REQUIREMENTS.
2. P.C.C. VALLEY GUTTER TO BE CONSTRUCTED ON COMMERCIAL/INDUSTRIAL STREETS, ARTERIALS, AND COLLECTORS. ASPHALT CONCRETE SWALES TO BE CONSTRUCTED ON LOCAL OR RESIDENTIAL ROADWAYS. CONTRACTOR TO DEPRESS LIPS OF GUTTER TO ENSURE ADEQUATE DRAINAGE.
LIMITS OF P.C.C. (6" THICK);
LIMITS OF PAYMENT FOR CURB RAMP;
LIMITS OF BROOM FINISH

TRANSITION BETWEEN
TYPE 1/2 AND
TYPE 1A/2A CURB
WHERE SHOWN ON
DRAWINGS

REF. PT. AT TBC,
RAMP MIDPOINT

P.C./P.T.

//

3 FEET WIDE FOR TYPE 2/2A CURB

MAX. 8.33% RUNNING GRADE;
MIN. 2.00% RUNNING GRADE;
MAX. 2.00% CROSS SLOPE;
NO LIDS OR OBSTRUCTIONS

2' BY 'RAMP WIDTH'
DETECTABLE WARNINGS
(SEE NOTE 2)

LENGTH AS REQUIRED
(SEE NOTE 3)

MAX 6"

STREET

24"

D.W.

8.33% MAX.

6" P.C.C. RAMP

LANDING/BYPASS

CURB & GUTTER
TYPE 1A OR 2A
SEE STANDARD
DETAIL 30-10

SECTION AA

NOTES:

1. CONSTRUCT PERPENDICULAR RAMPS AND FLARES WITH A BROOM FINISH
PARALLEL TO THE CURB.

2. INSTALL DETECTABLE WARNINGS (D.W.) IN ACCORDANCE WITH MANUFACTURER'S
RECOMMENDATIONS AND THE DRAWINGS. SET DETECTABLE WARNINGS SO THAT
THE FIELD AREA AT THE BASE OF THE DOMES IS FLUSH WITH SURROUNDING
CONCRETE. NO LIP IS ALLOWED AT THE EDGE OF THE DETECTABLE WARNINGS.

3. RAMP LENGTH REQUIRED FOR A TYPICAL SIDEWALK IS 8.0' FOR A TYPE 1/1A
CURB, AND 4.0' FOR A TYPE 2/2A CURB. LENGTH VARIES FOR SIDEWALKS WITH
DETACHED AND/OR INDEPENDENT GRADES. SEE DRAWINGS FOR NON-STANDARD
RAMP LENGTHS.

4. TRIM OUTSIDE EDGES AND JOINTS OF RAMP AND FLARES WITH ONE—QUARTER
INCH (1/4") RADIUS EDGING TOOL.

5. SEE DRAWINGS FOR REFERENCE POINT STATION AND OFFSET.

6. WHEN NO SIDEWALK ON CROSS STREET BY—PASS MUST CONTINUE AROUND
RAMP BEFORE TERMINATING IN SIDE STREET.
NOTES:

1. RAMP LENGTHS AND REFERENCE POINT STATION AND OFFSET SHALL BE AS SHOWN ON DRAWINGS.

2. INSTALL DETECTABLE WARNINGS (D.W.) IN ACCORDANCE WITH MANUFACTURER’S RECOMMENDATIONS AND THESE DRAWINGS. SET DETECTABLE WARNINGS SO THAT THE FIELD AREA AT THE BASE OF THE DOMES IS FLUSH WITH THE SURROUNDING CONCRETE. NO LIP IS ALLOWED AT THE EDGE OF THE DETECTABLE WARNINGS.

3. CONSTRUCT RAMPS AND LANDINGS WITH A BROOM FINISH PERPENDICULAR TO CURB.

SECTION AA

P.C.C. CURB VARIES 0-6” ± AS SHOWN ON THE DRAWINGS

TRANSMITION FROM TYPE 1A TO TYPE 2 P.C.C. C.&G. WITHIN RAMP RUN, AS REQUIRED IN SIDE STREET

LOWER LANDING:
2% CROSS SLOPE
2% MAX. RUNNING SLOPE
NO LIDS OR OBSTRUCTIONS

RAMP (TYP.) MAX. 8.33% RUNNING GRADE 2% CROSS SLOPE

LIMITS OF BROOM FINISH;
LIMITS OF P.C.C. (6” THICK);
LIMITS OF PAYMENT FOR CURB RAMP

SIDEWALK (TYPICAL)

TAPER TO GRADE

MATCH EXISTING

6” P.C.C. LANDING

TYPE II-A CLASSIFIED FILL AND BACKFILL

P.C.C. CURB VARIES 0-6” ± AS SHOWN ON THE DRAWINGS

MAX. 6”

STREET

CURB & GUTTER TYPE 1A OR 2A
SEE STANDARD DETAIL 30-10

SECTION # 30.04

DETAIL # 30-9
P.C.C. CURB AND GUTTER
TYPE 1A
(FOR USE IN CURB RAMPS)

P.C.C. CURB AND GUTTER
TYPE 2A
(FOR USE IN CURB RAMPS)

NOTES:
1. TRANSITION CURBS TO MAINTAIN CONSTANT FLOWLINE ACROSS CURB RAMP AND AROUND CURB RETURN PER THE DRAWINGS.
SURFACE REQUIREMENTS WITHIN ACCESS ROUTE:
1. 2% CROSS SLOPE MAXIMUM IN ANY DIRECTION
2. NO LIDS OR OBSTRUCTIONS
3. CONNECTED ACCESS TO PED SIGNAL BUTTON
I.A.W. STANDARD DETAIL 30–12

MINIMUM PEDESTRIAN ACCESS ROUTE
AROUND PERPENDICULAR CURB RAMPS

SURFACE REQUIREMENTS WITHIN ACCESS ROUTE:
1. 2% CROSS SLOPE MAXIMUM IN ANY DIRECTION
2. NO LIDS OR OBSTRUCTIONS
3. CONNECTED ACCESS TO PED SIGNAL BUTTON
I.A.W. STANDARD DETAIL 30–12

MINIMUM PEDESTRIAN ACCESS ROUTE
THROUGH PARALLEL RAMPS

CURB RAMP CLEARANCES
ACCESSIBLE PEDESTRIAN PUSH BUTTON APPROACH

NOTES:

1. PEDESTRIAN PUSH BUTTONS SHALL HAVE AN ACCESSIBLE APPROACH AS SHOWN.

2. APPROACH AREA SHALL JOIN OR OVERLAP THE MAIN SIDEWALK/PATHWAY, AND SHALL BE SURFACED WITH THE SAME MATERIAL.

3. APPROACH SHALL NOT EXCEED 2% SLOPE IN ANY DIRECTION.

4. APPROACH SHALL BE FREE FROM LIDS, OBSTRUCTIONS, AND LIPS.
NOTE:

PROVIDE 3/4" X 3/4" CHAMFERED CORNERS AT ALL OUTSIDE EDGES.
SIDEWALK RETAINING WALL

TRANSITION DETAIL
(PLAN VIEW)

NOTES:
1. STANDARD CURB AND GUTTER SECTION NOT INCLUDED IN UNIT BID PRICE FOR RETAINING WALL.
2. PLACE ONE CUBIC FOOT OF POROUS BACKFILL MATERIAL AROUND WEEP HOLE AS SHOWN.
3. PROVIDE 3/4" X 3/4" CHAMFERED CORNERS AT ALL OUTSIDE EDGES.
SIDEWALK RETAINING WALL

TRANSITION DETAIL
(PLAN VIEW)

NOTES:
1. STANDARD CURB AND GUTTER SECTION NOT INCLUDED IN UNIT BID PRICE FOR RETAINING WALL.
2. PLACE ONE CUBIC FOOT OF POROUS BACKFILL MATERIAL AROUND WEEP HOLE AS SHOWN.
3. PROVIDE 3/4” X 3/4” CHAMFERED CORNERS AT ALL OUTSIDE EDGES.
<table>
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<th>GENERAL</th>
<th>1</th>
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<tbody>
<tr>
<td>Article 1.1</td>
<td>Scope of Work</td>
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<tr>
<td>Article 1.2</td>
<td>Applicable Standards</td>
<td>1</td>
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<tr>
<td>Article 1.3</td>
<td>Subsurface Investigation</td>
<td>3</td>
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<tr>
<td>Article 1.4</td>
<td>Materials and Inspection</td>
<td>3</td>
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<tr>
<td>Article 1.5</td>
<td>Stripping Test for Aggregates</td>
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<td>Payment - General</td>
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<tr>
<th>SECTION 40.02</th>
<th>SEAL COAT</th>
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<tbody>
<tr>
<td>Article 2.1</td>
<td>Description</td>
<td>4</td>
</tr>
<tr>
<td>Article 2.2</td>
<td>Asphalt</td>
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<td>Cover Aggregate</td>
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<td>Construction</td>
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<td>Article 2.6</td>
<td>Measurement</td>
<td>7</td>
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<td>Article 2.7</td>
<td>Basis of Payment</td>
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<tr>
<th>SECTION 40.03</th>
<th>PRIME COAT</th>
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<td>Description</td>
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<td>Article 3.2</td>
<td>Materials</td>
<td>9</td>
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<td>Article 3.3</td>
<td>Construction</td>
<td>9</td>
</tr>
<tr>
<td>Article 3.4</td>
<td>Application of Bituminous Material</td>
<td>9</td>
</tr>
<tr>
<td>Article 3.5</td>
<td>Application of Blotter Material</td>
<td>10</td>
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<tr>
<td>Article 3.6</td>
<td>Measurement</td>
<td>10</td>
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<tr>
<td>Article 3.7</td>
<td>Basis of Payment</td>
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SECTION 40.01 GENERAL

Article 1.1 Scope of Work

The Work covered by these Specifications consists of providing all plant, labor, equipment supplies, material, transportation, handling, and storage, and performing all operations necessary to complete the construction of hot mix asphalt concrete pavement consisting of one or more courses on a previously prepared base, seal coat of asphalt cement and cover aggregate, and bituminous surface treatment in single or multiple courses.

Article 1.2 Applicable Standards

The latest revision of the following standards: American Society for Testing and Materials (ASTM), American Association of State Highway and Transportation Officials (AASHTO), and State of Alaska Department of Transportation and Public Facilities, Alaska Test Methods and Standard Practices (ATM) are hereby made a part of these Specifications. Additionally, the Western Alliance for Quality Transportation Construction (WAQTC) Test Methods and Procedures are hereby incorporated into these Specifications. The test designations listed below are those that are currently specified for use in this Division.

ADOT&PF Alaska Test Method 312

ASTM C-29 Test for Unit Weight of Aggregate
ASTM C-88 Test for Soundness of Aggregates by Use of Sodium Sulfate or Magnesium Sulfate
ASTM C-117 Test for Materials Finer than No. 200 Sieve in Mineral Aggregates
ASTM C-127 Test for Specific Gravity and Absorption of Coarse Aggregate
ASTM C-128 Test for Specific Gravity and Absorption of Fine Aggregate
ASTM C-131 Test for Resistance to Abrasion of Small Size Coarse Aggregate by Use of Los Angeles Abrasion Machine Test for Sieve or Screen Analysis of Fine and Coarse Aggregate
ASTM C-183 Sampling Hydraulic Cement
ASTM D-75 Sampling Stone, Slag, Gravel, Sand and Stone Block for Use as Highway Materials
ASTM D-140 Sampling Bituminous Materials
ASTM D-242 Specification for Mineral Filler for Bituminous Paving Mixtures
ASTM D-4125 Standard Test Method for Asphalt Content of Bituminous Mixtures by
Nuclear Method

ASTM D-4791 Flat and Elongated Particles
ASTM D-5821 Percent Fracture
AASHTO M-29 Fine Aggregate for Bituminous Paving Mix
AASHTO M-43 Standard Size of Coarse Aggregate for Highway Construction
AASHTO M-156 Requirements for Mixing Plants for Hot-Mixed, Hot-Laid Bituminous Paving Mixtures.
AASHTO M-208 Specification for Cationic Emulsified Asphalt
AASHTO M-226 Viscosity Graded Asphalt Cement - Table Three (3)
AASHTO M-320 Standard Specifications for Performance-Graded Asphalt Binder
AASHTO T-30 Test for Mechanical Analysis of Extracted Aggregate
AASHTO T-43 Test for Specific Gravity of Bituminous Materials
AASHTO T-85 Specific Gravity and Absorption of Coarse Aggregate
AASHTO T-180-D Test for Moisture-Density Relations of Soils
AASHTO T-102 Spot Test of Asphaltic Materials
AASHTO T-164 Test for Quantitative Extraction of Bitumen*
AASHTO T-195 Test for Coated Particles for Bituminous Mixtures
AASHTO T-168 Sampling Bituminous Paving Mixtures
AASHTO T-304 Uncompacted Void Content of Fine Aggregate (Fine Aggregate Angularity)
AASHTO T-308 Determining the Asphalt Content of Hot Mix Asphalt (HMA) by Ignition Method
AASHTO T-209 Theoretical Maximum Specific Gravity and Density of Bituminous Paving Mixtures
AASHTO T-166 Bulk Specific Gravity of Compacted Bituminous Mixtures Using Saturated Surface Dry Specimens
AASHTO T-275 Bulk Specific Gravity of Compacted Bituminous Mixtures Using Paraffin-Coated Specimens

* In lieu of the specified methods, the Engineer may permit the use of a nuclear asphalt gauge.

The following standards of The Asphalt Institute are also a part of these Specifications:

Construction Specifications for Asphalt Concrete, by The Asphalt Institute.

Mix Design Methods for Asphalt Concrete, by The Asphalt Institute.
The number of blows of the Compaction hammer used in the Marshall Mix Design will be fifty (50) blows per side of specimen.

**Article 1.3 Subsurface Investigation**

Information pertaining to subsurface exploration, boring, test pit locations, and other preliminary investigation may appear in the Contract Documents, or be available at selected locations for review by the Bidder. While such data will have been collected with reasonable care, there is not expressed or implied guarantee that conditions so indicated are exact or entirely representative of those actually existing. The Bidder shall make his or her own interpretation of results of such investigations and satisfy himself as to the conditions to be encountered.

**Article 1.4 Materials and Inspection**

Representative samples of all materials proposed for use under these Specifications shall be submitted to the Engineer for testing. The Contractor shall submit these materials at his own expense. Material shall not be used until it has been approved by the Engineer.

For verification of weights and measures, character of materials, and determination of temperatures used in the preparation of the paving mixes, the Engineer or his authorized representative shall at all times have access to all portions of the paving plant, aggregate plant, storage yards, and other facilities for producing and processing the material construction.

**Article 1.5 Stripping Test for Aggregates**

The use and proportions of antistrip agents shall be determined by ATM T-14 and be included in job mix design. At least seventy percent (70%) of the aggregate shall remain coated when tested in accordance with ATM T-14.

**Article 1.6 Payment - General**

Payment for all Work included in this Division shall be paid for in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described.
SECTION 40.02 SEAL COAT

Article 2.1 Description

The Work under this Section consists of the performance of all Work required for the construction of a seal coat of asphalt cement and cover aggregate on RAP, strip-paved, and gravel surfaces.

Article 2.2 Asphalt

The Contractor shall submit a certified analysis of the proposed asphalt from the refinery laboratory to the Engineer for review and approval. A copy of the certified analysis shall accompany each shipment of asphalt to the Project. The Engineer shall reserve the right to make check tests of the asphalt received on the Project site, and if the asphalt is not in accordance with the certified analysis, he may reject the material.

The asphalt required by these Specifications shall conform to the requirements of The Asphalt Institute for the type and grade shown below:

| Asphalt for Seal Coating | CRS-2 or CRS-2P |

Article 2.3 Cover Aggregate

A. General

Cover aggregate shall consist of crushed gravel and shall be sound, durable, free of adherent coatings of clay, dirt or any other objectionable matter, and shall have a percentage of wear not to exceed forty (40) after five hundred (500) revolutions, as determined by ASTM C-131. Fractured Face Count shall conform to Section 40.06, Article 6.2 - Material and Testing. The cover aggregate shall have the following gradation:

<table>
<thead>
<tr>
<th>Sieve Size</th>
<th>Percentage Passing Sieve</th>
</tr>
</thead>
<tbody>
<tr>
<td>1/2&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>90-100</td>
</tr>
<tr>
<td>#4</td>
<td>10-30</td>
</tr>
<tr>
<td>#8</td>
<td>0-8</td>
</tr>
<tr>
<td>#200</td>
<td>0-1</td>
</tr>
</tbody>
</table>

B. Stripping Test for Aggregates

Cover aggregate shall show no detrimental amount of stripping when tested as described in Section 40.01, Article 1.5 - Stripping Tests for Aggregates.
Regardless of the test results, a "non-stripping" additive shall be added to the asphalt in the amount of one-half percent (0.5%) by weight of the asphalt. Such additive material shall be of quality and grade acceptable to the Engineer.

Article 2.4 Equipment

A. General

All equipment used on this Work shall be of sufficient size and in such mechanical condition as to meet the requirements and to produce the Work to the specified quality.

B. Pressure Distributor

The bitumen distributor shall be pneumatic-tired, self-propelled, and shall have a capacity of not less than eight hundred (800) gallons. It shall be equipped with an independently-operated bitumen pump, tachometer, pressure gauges, volume metering devices, a thermometer for reading the tank temperature, and a hose attachment suitable for applying the bituminous material to spots unavoidably missed by the distributor. The independently-operated bitumen pump shall be equipped with a tachometer calibrated in revolutions per minute. The distributor shall also be equipped to agitate and circulate the bituminous material during the heating process. Spray bars shall be the circulating type with extensions, available for distributing width from eight to twenty-one feet (8’ to 21’) by one foot (1’) increments.

The nozzle shall give uniform distribution and the shutoff shall be quick and positive to prevent dripping. The distributor shall be designed and equipped to distribute the bituminous material uniformly at consistent surface speeds, at uniform temperatures with various surface widths, at known and maintained rates of five hundredth (0.05) to two (2.0) gallons per square yard within a tolerance of five (5) percent, and through pressure ranges from twenty-five (25) to seventy-five (75) pounds per square inch. Air pressure type equipment may be used only upon written approval from the Engineer.

C. Cover Aggregate Spreader

The spreader shall be self-propelled and capable of spreading the cover material uniformly for widths of eight to sixteen feet (8’ to 16’) in one foot (1’) increments, and adjustable to spread uniform layers of ten (10) to thirty (30) pounds per square yard. Revolving plate type chip spreaders will not be approved.

D. Rollers

Rollers shall be self-propelled, pneumatic-tired, weighing not less than five (5) tons and not more than eight (8) tons. Rolling shall follow closely on spreading of aggregate.
E. Hauling Equipment

The cover aggregate shall be transported from the plant to the site in trucks having tight, clean and smooth beds.

F. Miscellaneous Equipment

A power broom and all necessary hand tools thermometers, etc. shall be provided by the Contractor.

Article 2.5 Construction

A. Surface Preparation

The existing surface shall be swept clean of all dust, dirt, and other loose material with hand brooms or through the use of approved mechanical sweepers. Where existing dust and dirt cannot be satisfactorily removed by brooming, it may be necessary to flush the asphalt surface with water. If flushing is necessary, the Contractor shall furnish the necessary equipment for flushing.

B. Weather Limitations

Bituminous material shall not be placed during rainy or threatening weather, or when the moisture on the surface to be treated would prevent satisfactory bond, or when the air temperature is less than fifty degrees (50º) Fahrenheit, except by approval of the Engineer.

C. Heating and Application of Bituminous Material

The bituminous material shall be heated in such a manner as to insure even heating of the entire mass with an efficient and positive control at all times. It shall be applied at a temperature between one hundred thirty (130) and one hundred eighty degrees (180º) Fahrenheit. Necessary thermometers shall be supplied so that the temperature may be observed at all times.

The bituminous material for the surface coat shall be uniformly applied by means of a pressure distributor at the ratio of four-tenths (0.40) to fifty-five hundredths (0.55) gallons per square yard with forty-five hundredths (0.45) gallons per square yard desired. The quantity of material as measured by the volume measuring device of the distributor shall not vary from the true quantity, as herein specified, by more than five percent (5%). A strip of building paper or other suitable covering shall be used at the beginning and/or end of the spread to provide a positive cutoff at the desired limits.

Existing improvements such as curb and gutter, steps and buildings shall be protected to prevent contact with bituminous material.
D. Preparation and Application of Cover Aggregate

Cover material shall be sufficiently dried when it comes in contact with bituminous material that a satisfactory bond or coating is obtained. The moisture content shall not exceed two percent (2%) by weight.

Immediately following the application of the bituminous material, the aggregate cover material shall be uniformly spread over the surface with an approved mechanical spreader at a rate of twenty-two (22) to thirty (30) pounds per square yard. The cover material shall be applied continuously and without delay until the asphalt application is covered.

Whenever possible, successive strips shall be applied before the previous strip has cooled. Cover material shall not be spread on the six inches (6”) adjacent to an unprotected edge until the next strip of bituminous material has been applied. Rolling shall immediately follow the application of the cover material. The roller shall be pneumatic-tired and of such a weight that it does not crush the cover material.

Rolling shall continue only long enough to "set" the cover material in the bituminous material. Under no circumstances will the rolling continue until the cover material is crushed or pulverized. If the cover material is distributed or thrown off the surface by traffic, it shall be broomed back into place. Areas with a deficiency or excess of cover material shall be corrected.

E. Maintenance of Surface

After application of the cover material, the surface shall be maintained by the Contractor for two to five (2-5) days, depending on the weather. During this period the Contractor shall, at least once daily, redistribute the cover material that has become displaced by traffic, by means of brooms, a drag or other method satisfactory to the Engineer. When all possible material has been imbedded in the bituminous material to the satisfaction of the Engineer, the Contractor shall sweep the pavement surface of all excess material and remove it to the storage yard as designated, unless otherwise directed by the Engineer.

Article 2.6 Measurement

Bituminous material and cover aggregate shall be measured by weight in tons (2,000 lbs).

Legible weight tickets shall be submitted to the Engineer for all bituminous material and cover aggregate delivered to the Project site for use in the Work. All weight tickets shall contain, at a minimum, the following information:

1. Weight ticket serial number;
2. Vehicle identification number;
3. The date and time the load was weighed;
4. The tare weight of the vehicle;
5. The gross weight of the loaded vehicle, as registered on the scale; and
6. The legal gross weight of the vehicle, as permitted by Anchorage Municipal Code Sections 9.46.090 or 9.46.100.

The Owner shall not pay for that portion of any load in excess of the legal gross weight for the vehicle.

**Article 2.7 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt for Seal Coat</td>
<td>Ton</td>
</tr>
<tr>
<td>Cover Aggregate</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 40.03  PRIME COAT

Article 3.1  Description

This Work under this Section consists of the performance of all Work required for preparing and treating the base course with bituminous material, and blotter material, in conformity with the required lines.

Article 3.2  Materials

The prime coat shall be MC-30 or approved equal. The aggregate for blotter material shall conform to the gradation requirements of AASHTO M-43, Size No. 10. The aggregate shall be free from organic or other deleterious material.

Article 3.3  Construction

Weather Limitations: Bituminous material shall not be applied on a wet or frozen surface, or when the air temperature is below forty-five degrees (45º) Fahrenheit, or when weather conditions would prevent the proper construction of the prime coat.

Article 3.4  Application of Bituminous Material

The distributor shall be so designed, equipped, maintained and operated that bituminous material at even heat may be applied uniformly on variable widths of surface up to fifteen feet (15’) at readily determined and controlled rates from five hundredths (0.05) to two (2.0) gallons per square yard, with uniform pressure and with an allowable variation from any specified rate not to exceed two hundredths (0.02) gallon per square yard. Distributor equipment shall include a tachometer, pressure gauges, accurate volume measuring devices or a calibrated tank, and a thermometer for measuring temperatures of tank contents. Distributors shall be equipped with a power unit for the pump, and full circulation spray bars adjustable laterally and vertically.

When traffic is maintained, one-way traffic shall be permitted on the untreated portion of the roadbed. As soon as the bituminous material has been absorbed by the surface and will not pick up, traffic shall be transferred to the treated portion and the remaining width of the section shall be primed.
The quantities, rate of application, temperatures and areas to be treated shall be approved before application of the prime coat.

**Article 3.5 Application of Blotter Material**

If, after the application of the prime coat, the bituminous material fails to penetrate within the time specified and the roadway must be used by traffic, blotter material shall be spread in the amounts required to absorb any excess bituminous material. Blotter material will be used only at the direction of the Engineer.

**Article 3.6 Measurement**

Bituminous material will be measured by the ton and blotter material will be considered incidental to the asphalt used as prime coat.

**Article 3.7 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt Prime Coat</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 40.04  TACK COAT

Article 4.1  Description

The Work under this Section shall consist of performing all Work required for the application of bituminous material to an existing asphalt base or bituminous binder to provide bond for a superimposed asphalt wearing surface.

Article 4.2  Material

The Contractor shall submit a certified analysis from the refinery laboratory to the Engineer for review and approval. A copy of the certified analysis shall accompany each shipment of the asphalt received on the Project site, and if the asphalt is not in accordance with the certified analysis, he may reject the material. The asphalt required by these Specifications shall conform to the latest Asphalt Institute Specifications for the type and grade shown below:

<table>
<thead>
<tr>
<th>Asphalt for Tack Coat</th>
<th>STE-1 {Snap-Tack}</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>or equal</td>
</tr>
</tbody>
</table>

Article 4.3  Equipment

A. General

All equipment used on this Work shall be of sufficient size and in such mechanical condition as to meet the requirements and to produce the Work to the specified quality.

B. Pressure Distributor

The bitumen distributor shall have pneumatic tires of such width and number that the load produced on the binder course shall not exceed six hundred fifty (650) pounds per inch of tire width, and shall be so designed, equipped, maintained and operated that bituminous material at even heat may be applied uniformly on variable widths of surface at readily determined and controlled rates per square yards, with a pressure range of from twenty-five (25) to seventy-five (75) pounds per square inch, and with an allowable variation from the specified rate not to exceed five percent (5%). Distributor equipment shall include a tachometer, pressure gauges, volume measuring devices, and a thermometer for reading temperatures of tank contents.

Contractor shall be required to lay a test strip of not less than fifteen feet (15’) in length to demonstrate that the equipment is working. Location of the test strip will be on-site, as approved by the Engineer. Street closures required for testing of equipment will be the responsibility of the Contractor prior to testing.

Notification of testing will be made to the Engineer not less than 24 hours prior to making the test strip to allow the inspector and materials analysis personnel to be
present. Engineer may require reasonable adjustments to the distribution rate to meet the required or desired cured thickness.

All equipment shall meet federal and State of Alaska safety standards. The Engineer and Contractor will inspect equipment jointly. Contractor shall immediately repair or replace defective or non-working pumps, gauges, or spray bar parts.

Contractor shall re-test test strips that fail due to equipment failure or inexperienced personnel operating the equipment will be re-tested. Engineer may require additional tests that he feels is needed to verify that the equipment meets the requirements of these specifications and the Contractor has qualified personnel and supervision to complete this Work.

Supplying the application of tack coat test strips is incidental to the bid item “Tack Coat” and no separate payment shall be made.

C. Miscellaneous Equipment

A power broom equipped with blower and all necessary hand tools, thermometers, etc., shall be provided by the Contractor.

**Article 4.4 Construction**

A. Surface Preparations

Immediately before applying the tack coat, if the surface is sufficiently bonded, the full width of surface to be treated shall be swept with a power broom equipped with a blower, supplemented by hand brooms, washed down with water, or otherwise cleaned to remove all loose dirt, clay, or other loose and objectionable material.

After the operation of removing dust has been completed and prior to the application of the tack coat, the Engineer shall make an inspection of the existing pavement to determine its fitness to receive the bituminous material.

B. Weather Limitations

Tack coat shall not be applied during cold weather, after sunset, or to a wet surface. The tack coat shall be applied only when the temperature of the atmosphere is above sixty degrees (60°) Fahrenheit.

C. Heating and Application of Bituminous Material

The application of the bituminous tack material shall be made by means of a pressure distributor of approved type, and shall be in the following amounts. Contractor shall distribute undiluted amounts of bituminous material (STE-1 {Snap-Tack} or equal) so that the mixture will cure back to 0.05 to 0.10 gallons per square yard. The quality of material as measured by the volume measuring device of the distributor shall not vary from the true quantity, as herein specified, by more than
five percent (5%). The bituminous material shall be applied at a temperature between seventy-five and one hundred thirty degrees (75° and 130°) Fahrenheit. Necessary thermometers shall be supplied so that the temperature may be observed at all times.

Existing improvements such as keyboxes, manholes, cleanouts, monuments, curb and gutter, steps, and buildings shall be protected to prevent contact with bituminous material to the satisfaction of the Engineer. The surface shall be allowed to dry until it is a proper condition of tackiness to receive the AC surface course placement as is necessary to obtain this proper condition of tackiness. Until the wearing surface course is placed, the Contractor shall protect the tack coat from damage.

Article 4.5 Measurement

Bituminous material will be measured by weight in tons (2000 lbs).

Article 4.6 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt Tack Coat</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 40.05  CRACK AND JOINT SEALANT

Article 5.1  General

The Work under this Section shall consist of the performance of all plant, labor and supervision, equipment, and material for performing all operations required for the preparation and application of a hot bituminous emulsion and three-eighths inch (3/8") chip applied as a crack and joint sealant. The Work under this Section shall be performed after the asphalt pavement has been rotomilled or as directed by the Engineer.

Article 5.2  Materials

Asphalt materials shall be approved by the Engineer prior to use (a certified analysis by the refinery laboratory with each shipment of asphalt to the project may be acceptable). The Engineer shall reserve the right to make check tests of the asphalt received on the job and if the certified analysis proves to be unsatisfactory individual check tests will be required.

The asphalt required by these specifications shall conform to the requirements of the Asphalt Institute for the type and grade shown below:

Asphalt for Bituminous Emulsion Crack Sealant  STE-1 {Snap-Tack}

or approved equal

The aggregate shall consist of crushed gravel (three-eighths inch (3/8") minus and sand) evenly mixed and shall be sound, durable, and free of adherent coatings of clay, dirt, or any other objectionable matter. Not less than sixty percent (60%) by weight of crushed gravel shall consist of pieces having two or more faces being freshly fractured.

The aggregate shall have the following gradation:

<table>
<thead>
<tr>
<th>Sieve Designation</th>
<th>Percentage By Weight Passing</th>
</tr>
</thead>
<tbody>
<tr>
<td>3/8&quot;</td>
<td>100</td>
</tr>
<tr>
<td>#4</td>
<td>85-100</td>
</tr>
<tr>
<td>#8</td>
<td>0-25</td>
</tr>
<tr>
<td>#200</td>
<td>0-2</td>
</tr>
</tbody>
</table>

All aggregate equipment shall meet federal and State of Alaska safety standards and shall be sufficient in size and mechanical condition to meet the requirements and to produce the Work. The equipment will be inspected jointly by the Municipality and the Contractor and shall be approved by the Engineer before any Work is started. Any units found defective shall be immediately repaired or replaced before starting Work on this project. The Contractor shall notify the Engineer twenty-four (24) hours in advance for equipment inspection.
The bitumen distributor shall have pneumatic-tires and be self-propelled and shall have a capacity of not less than 400 gallons. It shall be equipped to agitate and circulate the bituminous material during heating, with an independently-operated bitumen pump, tachometer, pressure gauges, volume metering devices, tank thermometer, and a hand held pressurized spray bar with a single nozzle equipped with a quick and positive shut-off to prevent dripping or over-spraying.

Rollers shall be self-propelled, have pneumatic-tires, and weigh not less than five tons or more than eight tons.

The Contractor shall provide a power broom and all necessary hand tools, thermometers, etc., at no additional cost to the Municipality.

**Article 5.3 Construction**

Crack and joint sealant shall be applied to cracks or joints that have dimensions between three-eighths inch (3/8") and three-quarter inch (3/4") in width as directed by the Engineer. The sealant shall be applied in conformance with the manufacturer’s instructions and to full depth of the crack. Cracks less than three-eighths inch (3/8") in width will be filled with sealant (STE-1).

The cracks shall be cleaned and all loose material shall be blown out to the full depth of the crack. The interior of the crack shall be completely dried and existing asphalt surfaces shall be exposed to enable bonding of the sealant.

All cracks shall have a minimum depth of one inch (1”). If the crack does not have a minimum depth of one inch (1”), the crack shall be deepened to a minimum of one inch (1”) through the use of a router or similar device approved by the Engineer. The area covered with the emulsion/aggregate treatment shall be a minimum of six inches (6”) on either side of the crack.

Crack and joint sealant shall be prepared and applied in conformance with the manufacturer’s instructions. All extruded sealant shall be removed and feathered to transition to the existing pavement surface.

Crack sealant shall not be applied during cold weather, after sunset, or when the surface and crack are wet. The crack sealant shall be applied only when the temperature of the atmosphere is above sixty degrees (60º) Fahrenheit.

The application of the bituminous material shall be made by means of a pressure distributor of approved type. Undiluted amounts of bituminous material (STE-1) shall be distributed at the rate two-tenths to thirty-five hundredths (0.20 to 0.35) gallons per square yard. The quality of material as measured by the volume-measuring device shall not vary from the true quantity, as herein specified, by more than five percent (5%). The bituminous material shall be applied at a temperature between one hundred and twenty-five and one hundred and eighty-five degrees (125º and 185º) Fahrenheit.
The hot bituminous emulsion shall be applied with a hand held spray bar, with a nozzle pressure range from twenty (20) pounds per square inch (psi) to forty-five (45) pounds per square inch (psi), to distribute the bituminous material uniformly to the full depth of the crack without filling it.

Aggregate distribution shall follow the emulsion applications closely as possible. It shall be placed prior to the emulsion breaking to ensure the greatest adherence.

The loose aggregate shall be removed and all crack repairs shall be broomed and cleaned before the asphalt overlay is installed.

For wide cracks or other openings in the existing asphalt surface, which in the opinion of the Engineer are too large for crack sealing, the Engineer shall direct the Contractor to remove and replace pavement in accordance with Section 40.11 - Remove and Replace Existing Asphalt Surfacing or place an A.C. wedge course in accordance with Section 40.06 - Asphalt Concrete Pavement.

The crack and joint sealant must be in place a minimum of twenty-four (24) hours and approved by the Engineer prior to an application of the tack coat or overlay will be allowed.

**Article 5.4 Measurement**

The application of crack and joint sealant shall be measured per linear foot of sealant applied as directed. Application of aggregate shall be incidental to bid item “Crack and Joint Sealant” and no separate payment shall be made.

**Article 5.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Crack and Joint Sealant</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 40.06 ASPHALT CONCRETE PAVEMENT

Article 6.1 Description

The Work under this Section consists of the performance of all Work required for the construction of asphalt concrete pavement on a prepared base.

Article 6.2 Material and Testing

A. Asphalt:

The Contractor shall submit a certified analysis of the asphalt from the refinery laboratory to the Engineer for review and approval. A copy of the certified analysis shall accompany each shipment of asphalt to the asphalt mixing plant. The Engineer may make check acceptance tests of the asphalt binder, and, if the asphalt binder is not in accordance with the certified analysis, it shall be rejected.

1) For Class A asphalt concrete, the asphalt cement or binder required by this specification shall conform to the following AASHTO designations.

   Meets AASHTO M-320 and the following:
   - Performance Grade of Asphalt binder                              PG 64-28
   - Softening Point, minimum (AASHTO T-53)                           125°F
   - Toughness, minimum (ASTM D-5801)                                110 in-lbs
   - Tenacity, minimum (ASTM D-5801)                                 75 in-lbs

2) For other classes of asphalt concrete, the asphalt cement or binder required by these Specifications shall conform to the requirements of AASHTO M-320 and Certified Performance Grade Asphalt Binder PG 52-28.

B. Aggregates

1) Class A asphalt concrete

   Coarse aggregate is all mineral retained on the No. 4 sieve.

   The aggregate retained on the No. 4 sieve shall contain at least ninety percent (90%) by weight of particles having a minimum of two mechanically fractured surfaces.

   Coarse Aggregate: Particles retained on the No. 4 sieve. Remove all natural fines passing the No. 4 sieve before crushing coarse aggregates for the asphalt concrete grading. The grain-size distribution for the asphalt concrete shall consist entirely of aggregate produced from an aggregate crushing process. Crush only aggregate that is free from clay conglomerates, vegetative matter or other deleterious material. Crush only
aggregate which consists of sound, tough, durable rock of uniform quality and is not coated with silt or clay. Aggregates shall meet the following requirements:

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Percent Sodium Sulfate Loss</td>
<td>ASTM C-88</td>
<td>9% maximum</td>
</tr>
<tr>
<td>(5 cycles)</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Percent Fracture</td>
<td>ASTM D-5821</td>
<td>90% minimum</td>
</tr>
<tr>
<td>(Two Surfaces)</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Flat &amp; Elongated Particles</td>
<td>ASTM D-4791</td>
<td></td>
</tr>
<tr>
<td>3 to 1</td>
<td>-----</td>
<td>15% max.</td>
</tr>
<tr>
<td>5 to 1</td>
<td>-----</td>
<td>5% max.</td>
</tr>
<tr>
<td>Nordic Abrasion</td>
<td>ADOT&amp;PF Alaska Test Method 312</td>
<td>12% maximum</td>
</tr>
<tr>
<td>Absorption</td>
<td>AASHTO T-85</td>
<td>2% maximum</td>
</tr>
</tbody>
</table>

Fine Aggregate: Particles passing the No. 4 sieve. Remove all natural fines retained on the No. 4 sieve before crushing fine aggregate for the asphalt concrete grading. Crush only aggregate that is free from clay conglomerates, vegetative matter or other deleterious material and that consists of sound, tough, durable rock of uniform quality not coated with silt or clay.

The grain-size distribution for the asphalt concrete shall consist entirely of aggregate produced from an aggregate crushing process and be non-plastic as determined in accordance with AASHTO T-90 (Determining the Plastic Limit and Plasticity Index of Soils). Meet the quality requirements of AASHTO M-29, including S1.1, Sulfate Soundness and the following:

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Uncompacted Void Content of Fine Aggregate</td>
<td>AASHTO T-304</td>
<td>45% minimum</td>
</tr>
<tr>
<td>(Fine Aggregate Angularity)</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

2) Class D and E asphalt concrete

Coarse aggregate for Class D and E asphalt concrete is all mineral retained on the No. 4 sieve. The aggregate retained on a No. 4 sieve shall contain at least eighty percent (80%) by weight of crushed pieces having two or more mechanically fractured surfaces.
All coarse aggregate shall be free from coatings of clay, silt, or other objectionable matter and shall not contain clay balls or other aggregations of fine material. Coarse aggregate shall be tested for soundness in accordance with the requirements of ASTM C-88, or will have proven sound through adequate record of service.

When aggregate grading is such that the material will tend to segregate in stockpile or handling, it shall be supplied in 2 or more sizes. Each size of aggregate required to produce the combined graduation specified shall be placed in individual stockpile at the plant site and separated by bulkheads or other means. When it is necessary to blend 2 or more aggregate sizes, the blending shall be done through separate bins at the cold elevator feeders, and not in the stockpile.

Fine aggregate is composed of all mineral matter passing the No. 4 sieve. It shall consist of natural and/or manufactured material derived by crushing gravel.

The aggregate particles shall be clean, tough, durable, moderately sharp, and free from coating of clay, silt, or other objectionable matter and shall not contain clay balls or other aggregations of fine material. Fine aggregate shall be tested for soundness in accordance with the requirements of ASTM C-88, or shall have a satisfactory soundness record. When tested for soundness, the number of cycles shall be five (5), the solution shall be sodium sulphate; the maximum loss shall be nine percent (9%) by weight. Fine aggregates shall be maintained in individual stockpiles, suitably separated to prevent intermingling.

C. Mineral Filler

Mineral Filler shall conform to the requirements of ASTM D-242.

Article 6.3 Composition of Mixes

A. General Requirements

Paving mixtures prepared under these Specifications shall be composed of aggregate and paving asphalt within the limits set forth in the following table:

Asphalt paving mixtures prepared under these Specifications shall be composed of aggregate and asphalt cement within the limits set forth in the following table:
Percentages by Weight Passing Sieves

Wearing Course

<table>
<thead>
<tr>
<th>Sieve Size</th>
<th>Class A</th>
<th>Class D</th>
<th>Class E</th>
</tr>
</thead>
<tbody>
<tr>
<td>1&quot;</td>
<td>-----</td>
<td>-----</td>
<td>-----</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>100</td>
<td>-----</td>
<td>100</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>68 – 82</td>
<td>100</td>
<td>78 – 96</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>52 – 64</td>
<td>75 – 92</td>
<td>66 – 86</td>
</tr>
<tr>
<td>#4</td>
<td>36 – 46</td>
<td>50 – 68</td>
<td>46 – 66</td>
</tr>
<tr>
<td>#8</td>
<td>26 – 36</td>
<td>32 – 50</td>
<td>34 – 52</td>
</tr>
<tr>
<td>#16</td>
<td>16 – 28</td>
<td>20 – 38</td>
<td>24 – 42</td>
</tr>
<tr>
<td>#30</td>
<td>10 – 20</td>
<td>14 – 30</td>
<td>16 – 32</td>
</tr>
<tr>
<td>#50</td>
<td>6 – 16</td>
<td>10 – 24</td>
<td>10 – 24</td>
</tr>
<tr>
<td>#100</td>
<td>4 – 12</td>
<td>7 – 16</td>
<td>7 – 16</td>
</tr>
<tr>
<td>#200</td>
<td>3 – 8</td>
<td>3 – 9</td>
<td>3 – 9</td>
</tr>
</tbody>
</table>

Asphalt Cement * 5.0 – 7.0 5.0 – 7.0 5.0 – 7.0
*By weight of total mix

Target values for the gradation of the Job Mix Design shall be within the Broad Band Limits depicted in the table above.

B. Additive Materials

A "non-stripping" additive shall be added to the asphalt in the amount determined by ATM T-14 or one-fourth percent (0.25%) by weight of the asphalt, if approved by the Engineer. Such additive material shall be of quality and grade acceptable to the Engineer.

C. Job Mix

The Contractor, at his expense, shall submit to the Engineer for approval, a job mix formula within the limits specified above, for each class of mix designated by the Contract. Within each mix design the Contractor shall provide correction factor ignition points generated in accordance with AASHTO T-308. The aggregate gradation of the job-mix formula, when plotted upon an aggregate grading chart, shall closely approximate the shape of average gradations for the limits specified. For that portion of the aggregate passing No. 4 sieve, gradings which range from at or near the maximum of one (1) sieve to at or near the minimum of the next sieve will not be permitted. The Engineer may require increased asphalt content up to
one-half percent (0.5%) above that indicated by Marshall Design Criteria. Upon requiring increased asphalt content, the lower limit of percent voids and the upper limit of percent voids filled shall be waived.

D. Maximum Permissible Variations

Tolerances to the approved Job Mix Formula shall not exceed the permissible variations presented in the following table. The Job Mix Formula band shall mean the approved Job Mix Formula plus-or-minus (±) the numeric values for the maximum permissible variations.

<table>
<thead>
<tr>
<th>Sieve Size</th>
<th>Class A Asphalt</th>
<th>Class D &amp; E Asphalt</th>
</tr>
</thead>
<tbody>
<tr>
<td>3/8” and Larger</td>
<td>± 6.0</td>
<td>± 5.0</td>
</tr>
<tr>
<td>#4</td>
<td>± 5.0</td>
<td>± 5.0</td>
</tr>
<tr>
<td>#8</td>
<td>± 5.0</td>
<td>± 4.0</td>
</tr>
<tr>
<td>#16, 30 &amp; 50</td>
<td>± 4.0</td>
<td>± 4.0</td>
</tr>
<tr>
<td>#100</td>
<td>± 3.0</td>
<td>± 3.0</td>
</tr>
<tr>
<td>#200</td>
<td>± 2.0</td>
<td>± 2.0</td>
</tr>
<tr>
<td>Asphalt</td>
<td>± 0.4</td>
<td>± 0.4</td>
</tr>
</tbody>
</table>

When these permissible variations are applied to the “Class A Asphalt Concrete” Job Mix formula, the broad band limits in Subsection A, above, may be exceeded only as follows:

1. The three-quarter inch (3/4”) and No. 200 sieves shall not exceed the broad band limits in SubArticle 6.3.A - General Requirements;

2. All other sieves may exceed the broad band limits in SubArticle 6.3.A - General Requirements for the respective sieve sizes in the above table provided that the Job Mix Formula band is not exceeded.

When these permissible variations are applied to the “Class D or Class E Asphalt Concrete” Job Mix formulas, the individual sieve shall not exceed the Broad Band limits in SubArticle 6.3.A – General Requirements, above.

Maximum temperature shall not vary more than twenty-five degrees (25º) Fahrenheit from the approved Job Mix Formula design.
E. Test Methods

The job-mix shall be determined according to the Marshall Method, as set forth in The Asphalt Institute Manual.

Upon compaction and testing of the job-mix specimens, the mixture shall conform to the aforementioned specifications within the following limits:

<table>
<thead>
<tr>
<th></th>
<th>Class A</th>
<th>Class D &amp; E</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt Stability (Marshall) Pounds Minimum</td>
<td>1200</td>
<td>1200</td>
</tr>
<tr>
<td>Asphalt Flow (Marshall) Maximum</td>
<td>8 to 16</td>
<td>8 to 16</td>
</tr>
<tr>
<td>Asphalt Percent Voids</td>
<td>2.5 to 4.5</td>
<td>3 to 5</td>
</tr>
<tr>
<td>Asphalt Percent Voids Filled with Asphalt</td>
<td>70 to 80</td>
<td>75 to 85</td>
</tr>
</tbody>
</table>

Article 6.4 Equipment

A. General

All equipment furnished by the Contractor shall be maintained in a sound mechanical condition. Equipment shall be serviced and lubricated away from the paving site; units that drip fuel, oil and/or grease shall be removed from the Project until such leakage is corrected to the satisfaction of the Engineer.

B. Asphalt Mixing Plant

All plants, used by the Contractor, shall be designed, coordinated and operated to produce a mix uniformly within the job-mix tolerances as listed herein and in accordance with AASHTO M-156. The plant may be either a weightbatch type or a volumetric proportioning, continuous/drum mixing type, provided the equipment has demonstrated that it is suitable for producing finished mixtures complying with the job-mix formula specified herein.

The plant shall be equipped with the necessary equipment for storing, handling, drying, heating and mixing the aggregate and asphalt. Satisfactory means shall be provided for aggregate and asphalt control as to quantity and temperature. Adequate safety measures shall be provided on stairs, gears, pulley, chains, sprockets, and all other dangerous moving parts.

Contractor shall calibrate the asphalt plant not more than thirty (30) days in advance of production and furnish copies of the data to the Engineer at least one day prior to asphalt concrete production. Aggregate and asphalt cement sampling locations meetings OSHA safety requirements shall be provided. Proportioning (batch) scales shall not be used for weighing material for payment. Weight scales used in conjunction with a storage silo may be used to weight the final product for payment, provided the scales are certified by the State of Alaska. The asphalt plant shall maintain a current Air Quality Permit issued by the State of Alaska.
C. Pavers

Asphalt pavers shall be self-propelled units provided with a heated vibratory screed. Grade and cross slope shall be controlled through the use of automatic grade and slope control devices. The paver screed control system shall be automatically actuated by the use of an erected string-line or a mobile string-line (ski) at least thirty feet (30’) in length on the high side of the paver. Grade control shall be used on either (a) both the high and low sides, or (b) grade control on the high side and slope control on the low side.

The Contractor may request a waiver for the screed control system (string-line or ski) if he or she believes the paving grade poses an unreasonable obstacle in the form of extreme horizontal or vertical curves or unusual cul-de-sac and/or street configuration.

For trails, pavers shall be capable of placing the required thickness in one lift with a minimum paving width of five feet (5’), truck-towed spreader-type equipment will be permitted, providing the width and depth requirement can be met.

The paver shall be equipped with a receiving hopper having sufficient capacity for a uniform spreading operation. The hopper shall be equipped with a distribution system to place the asphalt concrete mixture uniformly in front of the screed without segregation and/or tearing.

The term “screed” includes any strike-off device operated by cutting, crowding, or other action which is effective on mixes at workable temperatures, without tearing, shoving, or gouging, and which produces a finished surface of an even and uniform texture. The screed shall be adjustable as to level and section and shall have provisions for vibration and heat.

The screed assembly shall produce a finished surface of the required smoothness, thickness, and texture without tearing, shoving, displacing or segregating the asphalt concrete mixture. Screed extensions used for paving a constant width shall be heated and vibrated. Auger extensions shall be within one and one-half feet (1.5’) of the screed extension on both sides.

The paver shall be capable of placing courses in thicknesses of from one-half inch (1/2”) to at least three inches (3”), and, in width, be adjustable in increments of six inches (6”) and one foot (1’).

The use of a pick-up machine to transfer the asphalt concrete mixture from a windrow to the paver hopper will be permitted, provided the pick-up machine is capable of collection of the windrowed material without damage to the underlying course. The Engineer will not allow the continued use of the pick-up machine if segregation, excessive temperature loss, or any detrimental effects are observed.

Paver shall be equipped with a means of preventing the segregation of the coarse aggregate particles from the remainder of the bituminous concrete mixture while
being carried from the paver hopper over the slat-conveyor to the auger chamber. The mechanism to accomplish this must be approved in writing by the paver manufacturer and may consist of chain curtains, deflector plates, or other devices and may be any combination of these.

The following specific requirements apply to the following identified bituminous pavers:

1. Blaw-Knox bituminous paver shall be equipped with the Blaw-Knox Materials Management Kit (MMK).
2. Cedarapids bituminous paver must have been manufactured in 1989 or later.
3. Caterpillar bituminous pavers shall be equipped with the following deflector plate models: 6630, 6631, or 6640.

Contractor shall provide a Certificate of Compliance that verifies the required mechanism has been installed to prevent bituminous paver segregation.

The Engineer shall approve all mechanisms proposed by Contractor for preventing paver segregation of coarse aggregate prior to the bituminous paver’s use on the project.

D. Rollers

Rollers shall be self-propelled, reversible, and equipped to maintain clean and straight contact surfaces. Heat shall be maintained on pneumatic tires by skirting or other approved devices.

The number, weight, and type of rollers furnished shall be sufficient to obtain the required density and surface requirements while the mix is in a workable condition. One pneumatic and a minimum of one vibratory roller shall be furnished and operated in a workmanlike manner by the Contractor. There shall be at least one operator for each roller.

Pneumatic Tired Rollers:

Pneumatic tired roller shall ride on not less than seven uniformly sized and uniformly inflated smooth tires mounted on wheel rims of twenty inch (20") minimum diameter. The rear group of tires shall align behind and cover the spaces between the forward group of tires. Tires shall be inflated, and the roller ballasted, to provide a uniform (plus or minus five [5] pounds per square inch) minimum ground contact weight of seventy (70) pounds per square inch, unless a lower weight is requested in writing by the Engineer. If a pneumatic roller experiences a pick-up problem, the Contractor shall be required to add an effect release agent to the tire watering tank.
Steel-Drum Rollers: Steel-wheel roller may be of two (2) types:

- Two-axle static drum rollers, 8 to 22 tons in weight.
- Two-axle vibratory drum rollers, 8 to 22 tons in weight.

All rollers shall be equipped with power units of not less than four (4) cylinders and under working conditions shall develop a compression in the rear wheels of two hundred fifty (250) to three hundred fifty (350) pounds per inch of roller width. Rollers shall be in good working condition and be free from backlash, faulty steering mechanism, or worn parts. Rollers shall be equipped with adjustable scrapers to keep the drums clean and with efficient means of keeping the drums/wheels wet to prevent mixes from sticking to the drums. Rollers/Drums shall be free of flat areas, openings or projections which will mar the surface of the pavement.

E. Haul Trucks

Vehicles used for the transportation of hot-mix asphalt from the plant to the Project shall have tight metal bottoms and shall be free from dust, screenings, petroleum oils, volatiles, and other mineral spirits which may effect the mix being hauled. The truck beds shall be cleaned as often as required, but at least once a day. After this operation the truck bed shall be elevated and thoroughly drained; no excess solution shall be permitted.

When requested by the Engineer, trucks shall be equipped with covers of canvas, insulated boxes, or other suitable material, and be of sufficient size and weight to protect the load from adverse weather conditions and to maintain the required mix temperatures.

F. Truck Scales

Hot mix asphalt shall be weighed on platform scales furnished by the Contractor or on public scales at the Contractor's expense. The scales shall be satisfactory to the Engineer and shall comply with all State Laws governing the use of scales. The scales shall be tested and sealed by an authorized public official, at the expense of the Contractor, as often as the Engineer may deem necessary to ensure their accuracy. Batch plant proportioning scales may be used in lieu of truck scales only with the written approval of the Engineer.

G. Hand Tools

Only lutes or asphalt rakes shall be used during the spreading operation and when finishing by hand.

Tamping irons shall weigh not less than twenty-five (25) pounds and shall have a bearing area not exceeding forty-eight (48) square inches. Mechanical compaction equipment, satisfactory to the Engineer, may be used instead of tamping irons.
H. Straightedges

Straightedges ten (10') and sixteen feet (16’) in length, to test the finished surface, shall be provided by the Contractor. The sixteen foot (16’) straightedge shall be used on straight sections and the ten foot (10’) straightedge on vertical curves or crown.

Article 6.5 Construction

A. Weather Limitations

Asphalt concrete mixture shall not be placed when it is raining or when rain is imminent, on a saturated surface, on an unstable/yielding roadbed, when the base material is frozen, or when weather conditions prevent proper handling or finishing of the mixture. Asphalt concrete mixture shall not be placed unless the surface temperature is forty-five degrees (45º) Fahrenheit or warmer and the ambient air is at least thirty-two degrees (32º) Fahrenheit and not descending. Air temperature shall be measured in the shade away from heat sources at the paving site.

B. Preparation of Area to be Paved

The area to be paved shall be true to line and grade, having a smooth dry, compacted surface prior to the start of paving operations. The area to be paved shall be free from all loose asphalt and foreign material.

Contractor shall notify the Engineer, a minimum of twenty-four (24) hours prior to paving, that the newly constructed, rotomill planed, or existing surface, has been prepared in conformance with the Drawings and Specifications and are ready to be paved. Engineer or his representatives shall inspect the grade through the use of string line, straightedge, levels, or any other means necessary. Upon determining the grade that has been proposed for paving is in conformance with the Drawings and Specifications, Engineer will provide written authorization for the Contractor to proceed with the paving. The Contractor shall not initiate paving prior to receiving written authorization to proceed.

The surface of the Leveling Course, when finished, shall not demonstrate any deviation in excess of three-eighths inch in ten feet (3/8” in 10’) parallel with, and at right angles to, the centerline, or more than five-eighths inch (5/8”) total from centerline to face of curb of the area to be paved. Any deviation in excess of this amount shall be corrected by loosening, adding, or removing material and reshaping and compacting to satisfy the above requirement.

Existing paved surfaces shall be cleaned of loose material by sweeping with a power broom, supplemented by hand sweeping, if determined necessary by the Engineer.

After rotomilling of a section of the roadway has been completed, that section shall be inspected by the Engineer for areas of distress or failure. Areas requiring repair
shall have the remaining pavement removed, and the distressed area shall be excavated to the depth and limits directed by the Engineer. The excavated area shall be backfilled, as directed by the Engineer, with crushed aggregate Leveling Course material and/or Asphalt Concrete leveling course in conformance with the Drawings and Specifications. Pavement surface irregularities, remaining from the rotomilling effort, that extend more than three-quarters inch (3/4") below the milling indentations shall be pre-leveled and brought into conformance with the tolerances established in Article 6.6 - Density and Surface Requirements. Pre-leveling shall be completed with an approved Class D asphalt concrete in accordance with this Section and include the furnishing, hauling, placing, and compaction of the asphalt concrete.

Contact surfaces of curbing, gutters, manholes, and other structures shall be painted with a thin, uniform coating of asphaltic cement or approved equal material prior to the mixture being placed against them. Butt joints on previously placed cooled pavement shall be saw cut and tack coated prior to continuing the paving operation.

Contractor shall not pave against newly placed concrete curbing until said curbing has cured for a minimum five (5) days. For the purpose of paving operations only, curb curing time may be reduced to seventy-two (72) hours only upon receipt of Contractor’s written certification that Type III Portland High-Early-Strength cement concrete was used in, properly placed, and appropriate curing compounds were applied to the adjacent curb and gutter.

C. Preparation of Paving Asphalt

The asphalt shall be heated at the paving plant to a temperature at which it can be properly handled through the pumping system, but at no time shall the temperature of the asphalts exceed that recommended by the asphalt supplier or manufacturer, or be greater than three hundred twenty-five degrees (325º) Fahrenheit or less than two hundred fifty degrees (250º) Fahrenheit.

D. Preparation and Handling of Aggregates

The aggregate for the asphalt concrete mixture shall be heated and dried to a temperature compatible with the mix requirements specified. The burner on the dryer shall be properly adjusted to avoid damage to the aggregate and to avoid the presence of unburned fuel on the aggregate. Any asphalt concrete mixture in which soot or fuel is present shall be wasted and no payment made.

Drying operations shall reduce the aggregate moisture content so that the moisture content of the asphalt concrete mixture, sampled at the point of acceptance for asphalt cement content, shall be no more than one-half percent (0.5%) (by total weight of mix), as determined by ATM T-25. Adequate dry storage shall be provided for the mineral filler.
Aggregates shall be stored at the plant in such a manner that the separate sizes will not become intermixed. Cold aggregate shall be carefully fed to the plant in such proportions that surplus and shortages in the hot bins will not cause breaks in the continuous operations.

Stockpiles and bins shall be sampled for gradation analysis, dust coating, and for other purposes, at the option of the Engineer.

When requested by the Engineer, the Contractor shall provide representative samples from each of the hot bins. Samples shall be used to determine compliance with these Specifications.

1. Drying:

   The aggregate shall be thoroughly dried and heated to provide a paving mix within a tolerance specified herein. The moisture content of the heated and dried aggregate shall not exceed one-half percent (0.5%).

   Dust collected during the drying operation may be fed uniformly back into the hot aggregate prior to screening, provided a position mechanical feed is used which will control the feed back to the quantity specified by the Engineer.

2. Screening:

   Aggregates shall be screened into sizes that may be recombined into a gradation meeting the requirements of the job-mix formula. Screens shall have normal capacities slightly in excess of the production capacity of the mixer and rated capacity of the dryer.

3. Hot Aggregate Storage:

   Hot screened aggregate shall be stored in such a manner as to minimize segregation and loss of temperature.

E. Mixing Plants and Controls

All plants shall be equipped with a positive means to govern the time of mixing. Mixing time shall not be altered unless requested by the Engineer.

Frequent gradation analysis of the hot aggregates of the completed mix shall be made to be certain that the materials being used and produced are within the tolerances of the job-mix formula and the specifications of the mix being used. If the mix is found to be outside the hot-mix formula tolerances or outside the specification limits, corrections shall be made in quantities measured from the hot bins and suitable changes made at the cold bin feeders. It shall be the responsibility of the Contractor to furnish a finished product in accordance with the Contract Documents. Tests conducted by the Engineer are for quality acceptance purposes only and are not authorized for use in plant calibration. Plant metering
systems and scales shall be calibrated to the accuracy specified in AASHTO M-156.

Batch Type Plant: When the mix is produced in a batch type plant, the aggregate shall be accurately weighed in the proper proportions to provide the batch weight.

The asphalt shall be heated to provide a material sufficiently fluid to produce a uniform coating on every particle of aggregate within the specified mixing time. The temperature of the aggregates and asphalt immediately prior to mixing shall be approximately that of the completed batch. In no case shall the temperature of the asphalt and aggregate vary more than twenty-five degrees (25º) Fahrenheit when placed in the mixing chamber.

A dry mixing period of not less than ten (10) seconds shall precede the addition of the asphalt to the mix. Excess wet mixing shall be avoided. Wet mixing shall continue as long as is necessary to obtain a thoroughly blended mix. The minimum percent of coated particles used to establish the mixing time interval shall be ninety-five percent (95%) as determined by AASHTO T-195.

Continuous Type Plant: Continuous mix and drum plants shall in general be controlled in the same manner as batch plants.

The determination of mixing time shall be by weight method under the following formula unless otherwise approved:

\[
\text{Mixing time in seconds} = \frac{\text{Pugmill Dead Capacity in Pounds}}{\text{Pugmill Output in Pounds Per Second}}
\]

The weights used for computing mixing time shall be determined for the job, from tests made by the Contractor and shall conform to the recommendations of the manufacturer. Mixing temperature shall not exceed that recommended by the asphalt cement manufacturer without the written approval of the Engineer. To aid in determining the proper temperature of the completed batch, current viscosity data shall be available at the plant at all times.

F. Transportation of Mix

The dispatching of the hauling vehicles shall be so scheduled that all material delivered may be placed and rolled in daylight. When variations in size of loads, speed of trucks, length of haul, and conditions of trucks interfere with orderly continuous operations, the Engineer may order suitable corrections to be made.

G. Mechanical Spreading

Contractor shall submit a Paving Plan for the Engineer’s review a minimum of five (5) working days prior to initiating paving operations. The plan shall consist of at least the following items:

1. Paving schedule to include sequence of operations.
2. Operational details to include:
   
a. Plant operating capacity and target production rate. Process control testing frequency for gradation, moisture, asphalt cement content, and compaction.

b. Number and capacity of trucks, cycle time, and delivery rate.

c. The manufacturer and model of the paver and pick-up machine to include information on grade followers, sensors, operating speed, and production rate of the pavers.

d. Number, type, weight, and operating speed of rollers, including replacement roller.

e. Location and method of constructing longitudinal and transverse joints.

f. Construction plan for paving intersections and driveways.

3. The asphalt concrete shall be placed on the road surface at a temperature not less than two hundred fifty degrees (250°F) Fahrenheit or greater than three hundred degrees (300°F) Fahrenheit. Additionally, the maximum temperature to which the asphalt concrete is heated shall not exceed the supplier’s recommendation. The asphalt concrete temperature shall be measured directly behind the paver screed at the time of placement.

The asphalt concrete mixture shall be laid upon a surface approved in writing by the Engineer, spread and struck-off and compacted to the thickness specified in the Drawings and specifications. Asphalt pavers shall be used to distribute the asphalt concrete mixture in lanes of such widths as to hold to a practical minimum the number of longitudinal joints required.

Longitudinal joints and edges shall be constructed to true line markings. Lines shall be established parallel to the center line for the paver to follow in placing individual lanes. The paver shall be operated and positioned to closely follow the established line. When backing trucks to the finisher, care shall be taken not to jar the paver.

The texture of the unrolled surface shall be checked to determine its uniformity. The adjustment of the screed, tamping, feed screws, hopper feed, etc., shall be checked frequently to assure uniform spreading of the mix. Segregation of the material shall not be permitted. If segregation occurs, the spreading operation shall be immediately suspended until the cause is determined and corrected.

Any irregularities left by the paver shall be corrected by trimming directly behind the machine by use of lutes or covered rakes. Immediately after trimming, the edges of the course shall be thoroughly compacted by tamping. Distortion of the pavement during this operation shall be avoided.
Edges against which additional pavement is to be placed shall be vertically formed to true line. A lute or covered rake shall be used immediately behind the finisher, when required to obtain a true line and vertical edge. Any irregularities in the surface of the pavement course shall be corrected directly behind the paver. Excess material forming high spots shall be removed by a shovel or lute. Indented areas shall be filled with hot-mix and smoothed with the back of a shovel pulled over the surface. Fanning of material over such areas shall not be permitted.

On longitudinal joints, the paver shall be positioned so that in spreading, the material overlaps the edge of the lane previously placed by one or two inches (1” or 2”) and is sufficiently high to allow for compaction. The coarse aggregate in the material overlapping the joint shall all be raked out into the cold lane as soon as possible behind the paver and broomed up and wasted. In no case shall scattered rocks be rolled into the surface of either lane.

Asphalt concrete mixture which is contaminated or segregated will be rejected.

When multiple lifts are specified in the Contract, the final lift shall not be placed until all lower lifts throughout that section, as defined by the Paving Plan, have been placed and accepted. Paving shall not begin until all adjacent curb has been poured for at least seven (7) days when Type I/II cement is used or three (3) days when Type III cement is used.

H. Hand Spreading

On areas where irregularities or unavoidable obstacles make the use of mechanical spreading and finishing equipment impracticable, the asphalt concrete mixture shall be spread, raked, and luted by hand tools. For such areas, the asphalt concrete mixture shall be placed to the required compacted thickness and density.

I. Compaction

Immediately after the asphalt mixture has been spread, struck off and surface irregularities adjusted, it shall be thoroughly and uniformly compacted by rolling.

The surface shall be rolled when the mixture is in the proper condition and when the rolling does not cause undue displacement, cracking, or shoving.

Initial rolling shall be done with a steel-drum roller with the drive roll operating toward the paver, and/or a suitable pneumatic tired roller. Initial rolling shall be completed while the bituminous mat temperature is above two hundred twenty-five degrees (225º) Fahrenheit.

Following the initial rolling at least three coverages of the pavement shall be completed with a pneumatic tired roller, while the mat temperature is above one hundred seventy-five degrees (175º) Fahrenheit.
Final rolling shall be completed with a steel–drum roller and shall continue until roller marks and further compression are not evident in the pavement and specified density has been achieved.

Unless otherwise directed, rolling shall begin at the sides and proceed longitudinally parallel to the road center line, each trip overlapping one-half the roller width, gradually progressing to the crown of the road. When paving in echelon or abutting a previously placed lane, the longitudinal joint should be rolled first followed by the regular rolling procedure. On superelevated curves the rolling shall begin at the low side and progress to the high side by overlapping of longitudinal trips parallel to the centerline.

Any displacement occurring as result of the reversing of the direction of a roller, or from other causes, shall be corrected at once by the use of rakes and addition of fresh mixture when required. Care shall be exercised in rolling not to displace the line and grade of the edges of the asphalt mixture.

To prevent adhesion of the mixture to the rollers, the wheels shall be kept properly moistened with water or water mixed with very small quantities of detergent or other approved material. Excess liquid will not be permitted.

Along forms, curbs, headers, walls, and other places not accessible to the rollers, the mixture shall be thoroughly compacted with hot hand tampers, smoothing irons, or with mechanical tampers. On depressed areas, a trench roller may be used or cleated compression strips may be used under the roller to transmit compression to the depressed area.

Rollers or other vehicles shall not be parked or left standing on pavement that has not cooled sufficiently to prevent indentation by wheels.

J. Joints

The Contractor shall not construct longitudinal joints in the driving wheel paths. The Contractor shall align the joints of the top layer of pavement to either the centerline of the road or to lane lines. The Contractor shall offset the longitudinal joint in the top layer of pavement not more than six inches (6") from centerline of edge of stripe. Joints shall be constructed to ensure a continuous bond between old and new sections of the course. All joints shall present the same texture and smoothness as other sections of the course. The Contractor shall offset the longitudinal joints in the top layer from the joint in the layer immediately below by at least four inches (4").

When joining existing pavement and new pavement, the old pavement shall be cut in a neat line with a power driven saw.

Improperly formed joints resulting in surface irregularities shall be removed full depth, replaced with fresh asphalt concrete mixture, and thoroughly compacted. Rolling of joints after the material has cooled below one hundred seventy degrees
(170º) Fahrenheit shall not be allowed. All pavement removal shall be precut to a neat line with a power-driven saw.

A tack coat of asphalt cement or asphalt emulsion shall be applied on all cold joints and allowed to break prior to placing fresh asphalt concrete mixture against the joint. This Work shall be completed by Contractor just prior to paving.

Transverse joints shall be formed by saw cutting back on the previous run to expose the full depth of the course or by using a removable bulkhead. Transverse joints shall not be perpendicular to centerline, but shall be skewed between fifteen and twenty-five degrees (15º and 25º).

K. Repair and Replacement

Asphalt concrete mixture that becomes contaminated with foreign material or is in any way defective as determined by the Engineer shall be removed. Skin patching will not be permitted. Defective materials shall be removed for the full thickness of the course. The pavement shall be cut so that all edges are vertical, the sides are parallel to the direction of traffic, and the ends are skewed between fifteen and twenty-five degrees (15º and 25º). Edges shall be coated with a thin tack coat of material. Fresh asphalt concrete mixture shall be placed in sufficient quantity so that the finished surface will conform to grade and smoothness requirements. The asphalt concrete mixture shall be compacted to the density specified. Any area determined to have an excess or deficiency of asphalt concrete shall be corrected by full depth removal and replacement. No payment shall be made for material replacing defective material. All costs associated with the patching of defective areas shall be borne by Contractor.

L. Vehicular Traffic

Contractor shall not allow vehicular traffic on the asphalt mat surface until the mat surface has cooled to below one hundred twenty degrees (120º) Fahrenheit. Any portion of the asphalt concrete mixture that becomes loose and broken, rutted, or damaged in any way due to vehicular traffic on the asphalt mat surface prior to it cooling to below one hundred twenty degrees (120º) Fahrenheit, shall be removed and replaced with fresh hot asphalt concrete, which shall be compacted to conform with the surrounding area at the specified density.

Article 6.6 Density and Surface Requirements

The complete pavement shall have a density equal to or greater than ninety-six (96) percent of Maximum Density (Marshall Method), except for trail pavement which shall have a density equal to or greater than ninety percent (90%). Maximum Density shall be determined in accordance with the test procedures specified in Section 40.01, Article 1.2 - Applicable Standards. The compacted specimens on which the Maximum Density is determined, shall be produced from a laboratory specimen made from the same days mix, and as close to the lay down temperature as practicable.
When requested by the Engineer, the Contractor shall, without charge, provide the Engineer with test samples of asphalt concrete cored from the completed pavement. All cores shall be at least four inches (4") in diameter and the core holes will be patched by the Contractor within seventy-two (72) hours.

The final surface shall be of a uniform texture conforming to true grade, and cross sections in accordance with the Contract Documents. The thickness of the course shall be in accordance with the Drawings and Specifications. Where curb and gutter is present the compacted pavement surface shall be one-eighth inch plus or minus one-eighth inch (1/8"±1/8") above the top front edge of curb.

Prior to the delivery of the first load of asphalt to the Project, the Contractor shall furnish straightedges to the Inspector for checking surface uniformity. Irregularities in the finished pavement surface shall not exceed three-sixteenths of an inch (3/16") within ten feet (10'), or five-sixteenths of an inch (5/16") within sixteen feet (16'). Non-conforming surfaces shall be subject to rejection by the Engineer. Irregularities which develop before the completion of rolling shall be remedied by loosening the surface mix, removing or adding material as may be required, and rerolling.

For trails, a ten foot (10') straightedge, supplied by the Contractor, shall be used to check the paving surface. Surface irregularities shall not exceed one inch in ten feet (1" in 10'). Non-conforming surfaces shall be subject to rejection by the Engineer. Irregularities which develop before completion of rolling shall be remedied by loosening the surface mix, removing or adding material as may be required, and rerolling.

Article 6.7 Measurement

Asphaltic concrete will be paid for by one of the methods as defined in the paragraph below and as designated in the Bid Schedule.

A. Measurement by the Ton

Measurement of hot-mix asphaltic paving materials, unless otherwise provided, shall be weighed on truck scales in accordance with Article 6.4, SubArticle F – Truck Scales. Asphalt concrete pavement shall be measured per ton (2,000 lbs) based on the amount of hot mix asphaltic material actually used in the completed and accepted work modified as follows: the quantity paid for shall not exceed one hundred and five percent (105%) of tonnage determined on the basis of the average core density, the specified neat line thickness, and the completed area of asphaltic concrete pavement. In addition, the Owner will not pay for that portion of any load in excess of the legal gross weight for the vehicle delivering the load.

B. Measurement by the Square Yard

Measurement of hot-mix asphaltic paving materials, unless otherwise provided, shall be measured by the completed and accepted work. The area measured will be that which is shown on the Drawings plus any additional areas as authorized by the Engineer in writing.
The tolerance for thickness of asphaltic concrete under square yard measurement shall be plus or minus one-fourth inch (1/4”) from design mat thickness, as shown on the typical section. This one-fourth inch (1/4”) variance shall be the exception only with the average variance for the job being plus or minus one-eighth inch (±1/8”) from the design mat thickness. All asphaltic concrete placed outside the variables allowed will be corrected by the Contractor at his expense.

C. Measurement by the Linear Foot

Measurement of hot-mix asphaltic paving materials for bike trails, unless otherwise provided, shall be per linear foot along the centerline of the constructed trail. The thickness of asphalt shall not be less than the thickness shown in the typical section as noted on the Drawings.

Article 6.8 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>A.C. Pavement (Class)</td>
<td>Ton</td>
</tr>
<tr>
<td>A.C. Pavement (Class, Thickness)</td>
<td>Square Yard</td>
</tr>
<tr>
<td>A.C. Pavement (Class, Thickness)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 40.07 STONE MASTIC ASPHALT CONCRETE PAVEMENT

Article 7.1 General

The Work under this Section consists of performing all operations pertaining to furnishing and mixing of aggregate, asphalt cement, and additives at a mixing plant; and the hauling, spreading, and compaction of the Stone Mastic Asphalt (S.M.A.) Concrete mixture on a previously prepared surface as specified in the Contract and in conformance with the lines, grades, and thicknesses shown on the Drawings.

Article 7.2 Material and Testing

A. Asphalt

Contractor shall submit a certified analysis of the asphalt from the refinery laboratory to the Engineer for review and approval. A copy of the certified analysis shall accompany each shipment of asphalt to the asphalt mixing plant. The Engineer reserves the right to make check tests of the asphalt received on the Project site; and if the asphalt is not in accordance with the certified analysis, he may reject the materials.

The asphalt required by these specifications shall conform to the requirements of The Asphalt Institute for the type and grade. For SMA Class of asphalt concrete, the asphalt cement or binding shall conform to the requirements of AASHTO M-320 and be certified Performance Grade Asphalt Binder PS 64-28.

B. Aggregate for Stone Mastic Asphalt Pavement

1. Coarse Aggregate

Coarse aggregate (that material retained on the No. 4 Sieve) shall be crushed stone or crushed gravel and shall consist of sound, tough, durable rock of uniform quality. All material shall be free from clay balls, vegetative matter, or other deleterious matters. Coarse aggregate shall not be coated with dirt or other finely divided mineral matter. In addition, coarse aggregate shall meet the following requirements:
Property Test Method Requirement
Percent of Wear AASHTO T-96 45 max
Degradation Value ATM T-13 30 min
Percent Sodium Sulfate Loss ASTM C-88 9 max (5 cycles)
Percent Fracture Double Face ASTM D-5821 90 min
Flat and Elongated Particles ASTM D-4791
3 to 1 15% max
5 to 1 5% max
Absorption AASHTO T-85 2% max
Nordic Abrasion ADOT&PF Alaska Test Method 312 12% max

2. Fine Aggregate

Fine aggregate (passing the No. 4 Sieve) shall meet the quality requirements of AASHTO M-29, and shall consist of one hundred percent (100%) crushed aggregate and shall be nonplastic (AASHTO T-90). Additionally, the uncompacted Voids shall be a minimum of forty-five percent (45%) when tested by AASHTO T-304 method A.

3. Mineral Filler

Mineral filler shall conform to the requirements of AASHTO M-17. The mineral filler shall be free of organic impurities and have a plastic index not greater than four. The mineral filler shall be sufficiently dry to flow freely and be free from agglomerations. The mineral filler shall not have less than forty percent (40%) by weight smaller in size than 0.02 mm.

**Article 7.3 Composition of Mixes**

A. General Requirements

1. Blended Aggregate

The several aggregate fractions for the mixture shall be sized, graded, and combined in such proportions that the resulting composite blend conforms to the following grading requirements:
STONE MASTIC ASPHALT CONCRETE
AGGREGATE BROAD BAND
Percent Passing by Weight

<table>
<thead>
<tr>
<th>Sieve Designation</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>3/4&quot;</td>
<td>100</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>82-88</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>60-70</td>
</tr>
<tr>
<td># 4</td>
<td>25-29</td>
</tr>
<tr>
<td># 8</td>
<td>18-24</td>
</tr>
<tr>
<td># 16</td>
<td>15-20</td>
</tr>
<tr>
<td># 30</td>
<td>12-18</td>
</tr>
<tr>
<td># 50</td>
<td>11-15</td>
</tr>
<tr>
<td># 100</td>
<td>10-14</td>
</tr>
<tr>
<td># 200</td>
<td>7-10</td>
</tr>
<tr>
<td>* Asphalt Cement</td>
<td>6.4-7.5</td>
</tr>
</tbody>
</table>

* By Weight of Total Mix

B. Additive Materials

1. Anti-strip Additives

Anti-strip agents shall be used in the proportions determined by the (ATM) T-14 and shall be included in the approved Job-Mix Design. At least seventy percent (70%) of the aggregate shall remain coated when tested in accordance with ATM T-14.

2. Stabilizing Additives

Cellulose stabilizing additive shall be used. The dosage rate for the cellulose is three-tenths percent (0.3%) by weight of the total mix. The allowable tolerance per ton shall not exceed plus or minus ten percent (±10%) of the required weight of the stabilizing additive.

The cellulose-stabilizing additive shall conform to the following properties:
CELLULOSE STABILIZING ADDITIVE

Property Requirement

Sieve Analysis:

Method A - Alpine Sieve Analysis (1)

<table>
<thead>
<tr>
<th>Fiber Length</th>
<th>0.25&quot; (max.)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Percent Passing # 100 Sieve</td>
<td>60-80</td>
</tr>
</tbody>
</table>

Method B - Mesh Screen Analysis (2)

<table>
<thead>
<tr>
<th>Fiber Length</th>
<th>0.035&quot;-0.06&quot;(avg.)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Percent Passing # 20 Sieve</td>
<td>75-95</td>
</tr>
<tr>
<td>Percent Passing # 40 Sieve</td>
<td>55-75</td>
</tr>
<tr>
<td>Percent Passing # 140 Sieve</td>
<td>10-30</td>
</tr>
</tbody>
</table>

Ash Content (3) 20 percent non-volatiles (max.)

pH (4) 7.5±1.0

Oil Absorption (5) 5.0±1.0 times fiber weight

Moisture Content (6) <5 percent

Bulk Density (7) 1.25-2.50 lbs/ft³

Notes:

1. **Method A - Alpine Sieve Analysis.** This test is performed using an Alpine Air Jet Sieve (Type 200 LS). A representative five-gram sample of fiber is sieved for fourteen (14) minutes at a controlled vacuum of twenty-two inches plus or minus three inches (22" ±3") of water. The portion remaining on the screen is weighed.

2. **Method B - Mesh Screen Analysis.** This test is performed using standard No. 20, 40, 60, 80, 100, and 140 sieves, nylon brushes, and a shaker. A representative ten (10) gram sample of fiber is sieved using a shaker and two nylon brushes on each screen. The amount retained on each sieve is weighed and the percentage passing calculated.

3. **Ash Content.** A representative two to three (2 to 3) gram sample of fiber is placed in a tared crucible and heated between eleven hundred and twelve hundred degrees (1100º and 1200º) Fahrenheit for not less than two hours. The crucible and ash are cooled in a desiccator and reweighed.
4. **pH Test.** Five grams of fiber is added to one hundred (100) ml of distilled water, stirred, and let sit for thirty (30) minutes. The pH is determined with a probe calibrated with pH 7.0 buffer.

5. **Oil Absorption Test.** Five grams of fiber is accurately weighed and suspended in an excess of mineral spirits for not less than five minutes to ensure total saturation. It is then placed in a screen mesh strainer (approximately one-half (0.5) square millimeter hole size) and shaken on a wrist action shaker for ten (10) minutes (approximately one and one-quarter inch [1 1/4"] motion at two hundred and forty [240] shakes/minute). The shaken mass is then transferred, without touching, to a tared container and weighed. Results are reported as the amount (number of times its own weight) the fibers are able to absorb.

6. **Moisture Content.** Ten grams of fiber is weighed and placed in a two hundred and fifty degree (250º) Fahrenheit forced-air oven for two hours. The sample is then reweighed immediately upon removal from the oven.

7. **Bulk Density.** Fluff fiber with air or Hobart Mixer, weigh out twenty-five (25) grams of fiber, place in 100-ml cylinder, tap cylinder, and measure volume.

C. **Job-Mix**

Contractor, at his expense, shall submit to the Engineer for approval a job-mix formula within the limits specified in this Article. The aggregate gradation of the job-mix formula, when plotted upon an aggregate grading chart, shall closely approximate the shape of average gradations for the limits specified. For that portion of the aggregate passing # 4 Sieve, gradings which range from at or near the maximum of one sieve to at or near the minimum of the next sieve will not be permitted. The Engineer may require increased asphalt content up to one-half percent (0.5%) above that indicated by the Marshall Design Criteria. Upon requiring increased asphalt content, the lower limit of percent voids and the upper limit of percent voids filled shall be waived.
D. Maximum Permissible Variations

<table>
<thead>
<tr>
<th>Measured Characteristics</th>
<th>Permissible Variation Percent by Weight of Total Mix</th>
</tr>
</thead>
<tbody>
<tr>
<td>3/4&quot;</td>
<td>+/-6.0</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>+/-6.0</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>+/-6.0</td>
</tr>
<tr>
<td># 4</td>
<td>+/-6.0</td>
</tr>
<tr>
<td># 8</td>
<td>+/-6.0</td>
</tr>
<tr>
<td># 16</td>
<td>+/-5.0</td>
</tr>
<tr>
<td># 30</td>
<td>+/-4.0</td>
</tr>
<tr>
<td># 50</td>
<td>+/-4.0</td>
</tr>
<tr>
<td># 100</td>
<td>+/-3.0</td>
</tr>
<tr>
<td># 200</td>
<td>+/-2.0</td>
</tr>
<tr>
<td>Asphalt Cement</td>
<td>+/-0.4</td>
</tr>
<tr>
<td>Density</td>
<td>93 Min. - 98 Max. of MSG</td>
</tr>
</tbody>
</table>

E. Test Methods

The job-mix shall be determined according to the Marshall Method, as set forth in State of Alaska, Department of Transportation and Public Facilities, Alaska Test Methods (ATM) T-17, except that the optimum voids total mix shall be adjusted from four percent (4.0%) voids to three percent (3.0%) voids when determining optimum asphalt content.

Upon compaction and testing of the job-mix specimens, the mixture shall conform to the aforementioned specifications within the following limits:

STONE MASTIC ASPHALT CONCRETE
### MIX DESIGN REQUIREMENTS

<table>
<thead>
<tr>
<th>Design Parameters</th>
<th>Requirements</th>
</tr>
</thead>
<tbody>
<tr>
<td>Stability, lbs., min.</td>
<td>1,000</td>
</tr>
<tr>
<td>Flow, 0.01 inch (0.25)</td>
<td>8-16</td>
</tr>
<tr>
<td>Voids in Total Mix, Percent</td>
<td>2-4</td>
</tr>
<tr>
<td>Compaction, Number of Blows Each Side of Test Specimen</td>
<td>50</td>
</tr>
<tr>
<td>Dust-Asphalt Ratio (The Percent of Material Passing the No. 200 Sieve Divided by the Percent of Asphalt, by Weight of Mix)</td>
<td>0.6-1.4</td>
</tr>
<tr>
<td>Voids in the Mineral Aggregate (VMA), min.</td>
<td>15.0</td>
</tr>
<tr>
<td>Voids in Coarse Aggregate (VCA) of Asphalt Concrete</td>
<td>Less than VCA of the Coarse Aggr. as determined by AASHTO T-19</td>
</tr>
<tr>
<td>Asphalt Cement Content, %</td>
<td>6.2 to 7.4</td>
</tr>
</tbody>
</table>

Approved Job-Mix Designs and aggregates will have the full tolerances, as listed above, applied and will not be limited to the broad band as referenced in SubArticle 7.3.A.1 – Blended Aggregate. Tolerances will not be applied to the largest sieve specified.

Contractor shall provide a Stone Mastic Asphalt Concrete mixture that meets the Stone Mastic Asphalt Concrete Job-Mix formula submitted for approval.

At least fifteen (15) calendar days prior to the production of Stone Mastic Asphalt Concrete mixture, Contractor shall submit the following to the Engineer:

1. A letter stating the location, size, and type of mixing plant, the proposed gradation for the Job-Mix Design, gradations for individual stockpiles with supporting process control information, and the blend ratio of each aggregate stockpile. The proposed gradation must meet the requirements of the table for, SubArticle 7.3.A.1 – Blended Aggregate Stone Mastic Asphalt Concrete Aggregate Broad Band. Stone Mastic Asphalt Concrete mixtures produced from different plants shall not be mixed.

2. Representative samples of each of the aggregates to be blended. Sample sizes: one hundred (100) pounds of each intermediate and/or coarse aggregate, two hundred (200) pounds of fine aggregate, twenty-five (25) pounds of blend sand and/or mineral filler.

3. A minimum of three one (1) gallon samples of the asphalt cement proposed for use in the mixture, including name of product, manufacturer, test results,
manufacturer’s certificate of compliance, and a temperature/viscosity curve for the asphalt cement.

4. A one-half (1/2) pint sample of the anti-strip additive proposed, including name of product, manufacturer, manufacturer’s data sheet, and current Materials Safety Data Sheet (MSDS).

5. A minimum five-pound sample of the cellulose-stabilizing additive proposed for use in the mixture, including name of product, manufacturer, test results, and manufacturer’s certificate of compliance.

Article 7.4 Equipment

A. General

All equipment shall be in good working order and free of asphalt concrete mix buildup. All equipment shall be available for inspection and demonstration of operation twenty-four (24) hours prior to placement of Stone Mastic Asphalt Concrete mix.

B. Bituminous Mixing Plants

Contractor shall use an asphalt plant designed to dry aggregates, maintain accurate temperature control, and accurately proportion asphalt cement and aggregates. Contractor shall calibrate the asphalt plant not more than thirty (30) days in advance of production and furnish copies of the data to the Engineer at least one day prior to Stone Mastic Asphalt Concrete production.

The asphalt plant shall have a scalping screen to prevent oversize material or debris from being incorporated into the Stone Mastic Asphalt Concrete mixture. Aggregate and asphalt cement sampling locations meeting OSHA safety requirements shall be provided.

Proportioning (batch) scales shall not be used for weighing material for payment. Weight scales used in conjunction with a storage silo may be used to weigh the final product for payment, provided the scales are certified. The asphalt plant shall maintain a current Air Quality Permit issued by the State of Alaska.

C. Hauling Equipment

Trucks used for hauling asphalt mixtures shall have tight, clean, smooth metal beds which have been thinly coated with a minimum amount of paraffin oil, lime water solution, or other manufactured asphalt release agent. Diesel fuel or fuel oil shall not be used as an asphalt release agent.

Each truck shall have a watertight canvas cover of such size as to extend at least one foot (1’) over the sides and end of the truck bed and be adequately secured to protect the Stone Mastic Asphalt Concrete mixture. The use of the canvas cover shall be at the Engineer’s direction.
D. Asphalt Pavers

Asphalt pavers shall be self-propelled units provided with a heated vibratory screed. Grade and cross slope shall be controlled through the use of automatic grade and slope control devices. The paver screed control system shall be automatically actuated by the use of an erected string line or a mobile string-line (ski) at least thirty feet (30’) in length on the high side of the paver. Grade control shall be used on either a) both the high and low sides or b) grade control on the high side and slope control on the low side.

The paver shall be equipped with a receiving hopper having sufficient capacity for a uniform spreading operation. The hopper shall be equipped with a distribution system to place the Stone Mastic Asphalt Concrete mixture uniformly in front of the screed.

The screed assembly shall produce a finished surface of the required smoothness, thickness, and texture without tearing, shoving, or displacing the Stone Mastic Asphalt Concrete mixture. Screed extensions used for paving a constant width shall be heated and vibrated. Auger extensions shall be within one and one-half feet (1.5’) of the screed extension on both sides.

The use of a pick-up machine to transfer the Stone Mastic Asphalt Concrete mixture from a windrow to the paver hopper will be permitted, provided the pick-up machine is capable of collection of the windrowed material without damage to the underlying course. The Engineer will not allow the continued use of the pick-up machine if segregation, excessive temperature loss, or any detrimental effects are observed.

E. Rollers

Contractor shall supply a minimum of three rollers weighing a minimum of twelve (12) tons each to compact the mixture to the required density while maintaining the pace of the paving operations. Rollers shall be of the static steel wheel or vibratory steel wheel type. The roller drums shall be a minimum of seven feet (7’) wide. The rollers shall be self-propelled and capable of reversing without backlash. They shall be specifically designed to compact hot asphalt concrete mixtures. The use of equipment which results in crushing of the aggregate, or pick-up of the Stone Mastic Asphalt Concrete mixture will not be permitted.

F. Truck Scales

Hot mix asphalt shall be weighed on platform scales furnished by Contractor or on public scales at Contractor’s expense. The scales shall be satisfactory to the Engineer and shall comply with all state laws governing the use of scales. The scales shall be tested and sealed by an authorized public official, at the expense of Contractor, as often as the Engineer may deem necessary to ensure their accuracy.
Article 7.5 Construction

A. Weather Limitation

Stone Mastic Asphalt Concrete mixture shall not be placed when it is raining or when rain is imminent, on a wet surface, on an unstable/yielding roadbed, when the base material is frozen, or when weather conditions prevent proper handling or finishing of the mixture. No Stone Mastic Asphalt Concrete mixture shall be placed unless the surface temperature is forty-five degree (45º) Fahrenheit or warmer and the ambient air is at least thirty-five degrees (35º) Fahrenheit and not descending.

B. Preparation of Area to be Paved

Existing surfaces shall be prepared in conformance with the Drawings and Specifications. Existing paved surfaces shall be cleaned of loose material by sweeping with a power broom, supplemented by hand sweeping, if necessary.

After planing of a section of the roadway has been completed, that section shall be inspected by the Engineer for areas of distress or failure. Areas requiring repair shall have the remaining pavement removed, and the distressed area shall be excavated to the depth and limits directed by the Engineer. The excavated area shall be backfilled, as directed by the Engineer, with crushed aggregate leveling course material in conformance with the Drawings and Specifications, asphalt concrete leveling course, or with Stone Mastic Asphalt Concrete. Pavement surface irregularities remaining from the planing effort that extend more than three-quarter inch (3/4") below milling indentation shall be pre-leveled and brought into conformance with the tolerances established in Article 7.6 Density and Surface Requirements. Pre-leveling shall be completed with an approved Class B Asphalt Concrete in accordance with Section 40.06 - Asphalt Concrete Pavement and include the furnishing, hauling, placing, and compaction of the asphalt concrete.

Contact surfaces of curbing, gutters, manholes, and other structures shall be painted with a thin, uniform coating of asphaltic cement or approved equal material prior to the mixture being placed against them. Butt joints on previously placed cooled pavement shall be tack-coated prior to continuing the paving operation.

C. Preparation of Paving Asphalt

The asphalt shall be heated at the paving plant to a temperature at which it can be properly handled through the pumping system, but at no time shall the temperature of the asphalt exceed that recommended by the asphalt supplier or manufacturer, or be greater than three hundred and twenty-five degrees (325º) Fahrenheit or less than two hundred and fifty degrees (250º) Fahrenheit.

D. Preparation and Handling of Aggregates

The aggregate for the Stone Mastic Asphalt Concrete mixture shall be heated and dried to a temperature compatible with the mix requirements specified. The burner
on the dryer shall be properly adjusted to avoid damage to the aggregate and to avoid the presence of unburned fuel on the aggregate. Any Stone Mastic Asphalt Concrete mixture in which soot or fuel is present shall be wasted and no payment made.

Drying operations shall reduce the aggregate moisture content so that the moisture content of the Stone Mastic Asphalt Concrete mixture, sampled at the point of acceptance for asphalt cement content, shall be no more than one-half percent (0.5%) (by total weight of mix), as determined by WAQTC TM 6. Adequate dry storage shall be provided for the mineral filler and the stabilizing additive.

E. Mixing Plants and Controls

The aggregate, asphalt cement, additives, and stabilizing additives shall be combined uniformly in the amounts required by the Job-Mix Design. The materials shall be mixed such that a complete and uniform coating of the aggregate is obtained.

1. Batch Plants

Stabilizing additive shall be added through a separate inlet directly into the weigh hopper above the pugmill. The addition of the stabilizing additive shall be timed to occur during the hot aggregate charging of the hopper. Adequate dry mixing time is required to ensure proper blending of the aggregate and fiber stabilizer.

2. Drum Mix Plants

Cellulose stabilizing additive shall be introduced into the drum mixer at the same point the asphalt cement is added. Operation of the drum mixer will be such to ensure complete blending of the stabilizing additive into the mix.

The dry mixing and wet mixing times for Stone Mastic Asphalt Concrete with cellulose stabilizing additive shall be a minimum of fifteen (15) seconds and forty (40) seconds, respectively. For batch plants, dry aggregate shall be placed in motion immediately prior to the addition of asphalt cement. Wet mixing time shall be adequate to obtain ninety-eight percent (98%) coated particles when tested in accordance with AASHTO T-195.

The temperature of the Stone Mastic Asphalt Concrete mixture at the time of the mixing shall be as determined by the Job-Mix Design.

Provisions shall be made for proportioning the mineral filler into the mixture uniformly and in the desired quantities. Special attention shall be directed to providing appropriate equipment for accurately proportioning the relative large amounts of mineral filler required for a Stone Mastic Asphalt Concrete mixture.
F. Temporary Storage of Asphalt Concrete Mixture

Temporary storage or holding of Stone Mastic Asphalt Concrete mixture in silo type storage bins will be permitted.

All of the Stone Mastic Asphalt Concrete mixture drawn from the silo type storage bins shall conform to all of the requirements for Stone Mastic Asphalt Concrete mixtures as if loaded directly into hauling equipment from the mixing plant. Signs of visible segregation, heat loss, changes from the Job-Mix Design, changes in the characteristics of asphalt cement, lumpiness, or stiffness of the mixture will be cause for rejection.

Rejected Stone Mastic Asphalt Concrete mixture shall be disposed of by Contractor at no cost to the Owner.

G. Mechanical Spreading

Contractor shall submit a Paving Plan for the Engineer’s review a minimum of five (5) working days prior to initiating paving operations. The plan shall consist of at least the following items:

1. Paving schedule to include sequence of operations.

2. Operational details to include:
   a. Plant operating capacity and target production rate. Process control testing frequency for gradation, moisture, asphalt cement content, and compaction.
   b. Number and capacity of trucks, cycle time, and delivery rate.
   c. The manufacturer and model of the paver and pick-up machine, to include information on grade followers, sensors, operating speed, and production rate of the pavers.
   d. Number, type, weight, and operating speed of rollers, including replacement roller.
   e. Location and method of constructing longitudinal and transverse joints.
   f. Construction plan for paving intersections and driveways.

3. The Stone Mastic Asphalt Concrete shall be placed on the road surface at a temperature not less than two hundred and sixty degrees (260°F) Fahrenheit or greater than three hundred degrees (300°F) Fahrenheit. Additionally, the maximum temperature to which the Stone Mastic Asphalt Concrete is heated shall not exceed the supplier’s recommendation. The Stone Mastic
Asphalt Concrete temperature shall be measured directly behind the paver screed at the time of placement.

The Stone Mastic Asphalt Concrete mixture shall be laid upon a surface approved by the Engineer, spread and struck-off and compacted to the required compacted thickness. The mixture shall be spread and compacted in one layer with a total thickness of two inches (2”). Asphalt pavers shall be used to distribute the Stone Mastic Asphalt Concrete mixture in lanes of such widths as to hold to a practical minimum the number of longitudinal joints required.

On areas where irregularities or unavoidable obstacles make the use of mechanical spreading and finishing equipment impracticable, the Stone Mastic Asphalt Concrete mixture shall be spread, raked, and luted by hand tools. For such areas the Stone Mastic Asphalt Concrete mixture shall be placed to the required compacted thickness.

Any Stone Mastic Asphalt Concrete mixture which is contaminated or segregated will be rejected.

When multiple lifts are specified in the Contract, the final lift shall not be placed until all lower lifts throughout that section, as defined by the Paving Plan, have been placed and accepted. Paving shall not begin until all adjacent curb has been poured and cured for at least seventy-two (72) hours.

H. Compaction

Immediately after the Stone Mastic Asphalt Concrete mixture has been spread, struck-off, and surface irregularities adjusted, it shall be thoroughly and uniformly compacted by rolling.

Due to the nature of Stone Mastic Asphalt Concrete mixtures, the surface shall be rolled immediately. Rolling shall be accomplished with a minimum of three steel wheel rollers of a minimum weight of twelve (12) tons each. Rolling procedures shall be adjusted to provide the specified pavement density. Rollers shall move at a uniform speed to obtain no less than ten to twelve (10-12) impacts per foot with the drive roller nearest the paver. The rollers vibratory eccentric’s shall be set for high frequency and low amplitude. Rolling shall continue until all roller marks are eliminated and the minimum density has been obtained.

To prevent adhesion of the mixture to the rollers, it shall be necessary to keep the wheels properly moistened with water mixed with detergent or other approved material. Rollers showing evidence of excessive pick-up will be replaced.

Once sufficient in-place density has been achieved, rolling operations shall cease. If migration of asphalt cement and filler to the compacted pavement surface has
occurred, Contractor shall remove the defective area and replace it with fresh Stone Mastic Asphalt Concrete mixture at his expense.

The surface will be tested after final rolling at selected locations using a ten foot (10') straightedge. The variation of the surface from the testing edge of the straightedge between any two contacts with the surface shall not exceed three-sixteenth inch (3/16"). The Stone Mastic Asphalt Concrete mixture in all defective areas shall be removed and replaced. All costs associated with removal and replacement of Stone Mastic Asphalt Concrete mixture in the defective areas shall be borne by Contractor.

Areas not accessible to the rollers shall be graded with rakes and lutes and compacted with mechanical tampers. For depressed areas a trench roller may be used to achieve the required compaction.

Stone Mastic Asphalt Concrete mixture that becomes loose and broken, segregated, mixed with dirt, or is any other way defective shall be removed and replaced with fresh Stone Mastic Asphalt Concrete mixture, which shall be compacted to conform with the surrounding area. Areas where asphalt cement, stabilizing additive and/or filler has migrated to the compacted pavement surface and produced “flushing” or segregated coarse aggregate has formed “rock pockets” shall be removed and replaced with fresh Stone Mastic Asphalt, by the contractor without compensation.

The Stone Mastic Asphalt Pavement shall be compacted to a minimum of ninety-three (93%) and maximum of ninety-eight percent (98%) of maximum specific density (MSD) as determined in accordance with AASHTO T-209. The compacted specimens, on which the maximum density is determined, shall be calculated from a laboratory specimen produced from a field sample of the same days mix. Acceptance testing for field density shall be determined in accordance with AASHTO T-166/T-275.

Rollers or other vehicles shall not be parked or left standing on pavement that has not cooled sufficiently to prevent indentation by wheels.

I. Joints

Joints shall be constructed to ensure a continuous bond between old and new sections of the course. All joints shall present the same texture and smoothness as other sections of the course.

When joining existing pavement and new pavement, the old pavement shall be cut in a neat line with a power driven saw.

Improperly formed joints resulting in surface irregularities shall be removed full depth, replaced with fresh Stone Mastic Asphalt Concrete mixture, and thoroughly compacted. Rolling of joints after the material has cooled below one hundred and
seventy degrees (170º) Fahrenheit shall not be allowed. All pavement removal shall be precut to a neat line with a power driven saw.

A tack coat of asphalt cement or asphalt emulsion shall be applied on all cold joints and allowed to break prior to placing fresh Stone Mastic Asphalt Concrete mixture against the joint. Contractor shall complete this Work immediately prior to paving.

Transverse joints shall be formed by saw cutting back on the previous run to expose the full depth of the course or by using a removable bulkhead. Transverse joints shall not be perpendicular to centerline, but shall be skewed between fifteen and twenty-five degrees (15º and 25º).

The longitudinal joints in one layer shall offset those in the layer immediately below by at least six inches (6”). Longitudinal joints shall be at lane lines. Where preformed marking tape striping is required, the longitudinal joint in the top layer shall be offset not more than six inches (6”) from the edge of the stripe.

J. Repair and Replacement

Stone Mastic Asphalt Concrete mixture that becomes contaminated with foreign material or is in any way defective as determined by the Engineer shall be removed. Skin patching is not permitted. Defective materials shall be removed for the full thickness of the course. The pavement shall be cut so that all edges are vertical, the sides are parallel to the direction of traffic, and the ends are skewed between fifteen and twenty-five degrees (15º and 25º). Edges shall be coated with a thin tack coat of material. Fresh Stone Mastic Asphalt Concrete mixture shall be placed in sufficient quantity so that the finished surface will conform to grade and smoothness requirements. The Stone Mastic Asphalt Concrete mixture shall be compacted to the density specified. No payment shall be made for material replacing defective material. All costs associated with the patching of defective areas shall be borne by Contractor.

K. Vehicular Traffic

Contractor shall not allow vehicular traffic on the asphaltic mat surface until the mat surface has cooled to below one hundred and twenty degrees (120º) Fahrenheit. Any portion of the Stone Mastic Asphalt Concrete mixture that becomes loose and broken, rutted, or damaged in any way due to vehicular traffic on the asphaltic mat surface prior to it cooling to below one hundred and twenty degrees (120º) Fahrenheit, shall be removed and replaced with fresh hot Stone Mastic Asphalt Concrete, which shall be compacted to conform with the surrounding area at the specified density.

L. Test Section

A minimum five (5) days prior to proceeding with Stone Mastic Asphalt Concrete paving, Contractor shall place a minimum of three hundred (300) tons of Stone Mastic Asphalt Concrete mixture in a test section at a location approved by the
Engineer. The test section will be used to adjust the Job-Mix Design asphalt cement content and to also familiarize Contractor’s personnel with the laydown and compaction of Stone Mastic Asphalt Concrete mixture.

Stone Mastic Asphalt Concrete mixture placed in the test strip that meets specifications will be measured and paid for at Contract prices. No payment will be made for mixture that does not meet specifications. If the test strip is placed within the project limits and does not meet specifications it shall be removed at Contractor’s expense.

**Article 7.6 Density and Surface Requirements**

Stone Mastic Asphalt Concrete placed on this project shall have a density equal to or greater than ninety-three percent (93%) but shall not exceed ninety-eight percent (98%) of the maximum specific gravity (MSG) as determined in accordance with AASHTO T-209. For the first day’s production of Stone Mastic Asphalt Concrete, the MSG will be determined by the Job-Mix Design. For each subsequent day’s production, the MSG will be determined by a sample of Stone Mastic Asphalt Concrete from the respective day’s production. Acceptance testing for field density will be determined in accordance with ATM T-18, except that a minimum six inch (6”) diameter core is required.

When requested by the Engineer, Contractor shall, without charge, provide the Engineer with test samples of Stone Mastic Asphalt Concrete cored from the completed pavement. All cores shall be at least six inches (6”) in diameter and the Contractor shall patch core holes within seventy-two (72) hours.

The final surface shall be of a uniform texture conforming to true grade and cross sections in accordance with the Contract Documents. The Engineer shall determine the thickness of the course.

Prior to the delivery of the first load of asphalt to the project, Contractor shall furnish straightedges to the Inspector for checking pavement surface uniformity. Surface irregularities of the newly placed Stone Mastic Asphalt Concrete Pavement shall not exceed three-sixteenth inch (3/16") within ten feet (10'), or five-sixteenth inch (5/16") within sixteen feet (16’). Stone Mastic Concrete Pavement with non-conforming surfaces shall be subject to rejection by the Engineer. Irregularities which develop before the completion of rolling shall be remedied by loosening the surface mix, removing or adding material as may be required, and rerolling.

**Article 7.7 Measurement**

Stone Mastic Asphalt Concrete will be paid for as defined in the following paragraphs and as designated in the bid schedule.

A. Measurement by the Ton

Measurement of Stone Mastic Asphalt Concrete, unless otherwise provided, shall be weighed on truck scales in accordance with Article 7.4, SubArticle F – Truck
Scales. Stone Mastic Asphalt Concrete pavement shall be measured per ton of two thousand (2,000) pounds based on the amount of hot mix Stone Mastic Asphaltic material actually used in the completed and accepted Work modified as follows: the quantity paid for shall not exceed one hundred and five percent (105%) of tonnage determined on the basis of the average core density, the specified neat line thickness, and the completed area of Stone Mastic Asphaltic Concrete pavement. In addition, the Owner shall not pay for that portion of any load in excess of the legal gross weight for the vehicle delivering the load.

B. Sampling and Testing

Samples taken for the determination of acceptance will be taken from behind the screed prior to initial compaction. Samples may be taken from the windrow, if belly-dumps are used, with the prior approval of the Engineer. Asphalt cement content will be determined by ATM T-23, AASHTO T164-94, or AASHTO T-308, with the exception that the moisture content will be determined by ATM T-25.

For Stone Mastic Asphalt Concrete mixture samples, the gradation will be determined in accordance with AASHTO T-30 from the aggregate remaining after the ignition oven (AASHTO T-308) has burned off the asphalt cement, or from aggregate extracted in accordance with AASHTO T164-94. If the asphalt cement content is determined by AASHTO T-308, the percent of asphalt shall be reduced by three-tenths percent (0.3%), by weight of total mix, for Stone Mastic Asphalt containing cellulose.

Article 7.8 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>S.M.A. Concrete Pavement</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 40.08  RECYCLED ASPHALT PAVING (RAP)

Article 8.1 General

The work under this Section consists of furnishing all materials and performing all operations necessary to complete placement and construction of a recycled asphalt paving (RAP) surface on an existing prepared subbase.

Article 8.2 Material

The RAP shall consist of crushed gravel, rock, sand, or other approved material. The aggregate shall be derived from recovered AC pavement and be free from lumps, balls of clay, or other objectionable matter, and shall be durable and sound. The portion of the material retained on a No. 4 sieve shall be known as coarse aggregate. Maximum RAP conglomerate (prior to extraction) shall pass the one inch (1”) sieve.

RAP is to be delivered to the jobsite in an unheated condition. Delivery of heated material to the jobsite will be rejected unless previously approved by the Engineer.

A. Coarse Aggregate

The coarse aggregate material conforming to the requirement specified above shall have a percentage of wear not to exceed fifty (50) after five hundred (500) revolutions, as determined by the current requirements of ASTM C-131. It shall consist of angular fragments, reasonably uniform in density and quality, and reasonably free from thin elongated pieces, dirt, and other objectionable material. At least fifty percent (50%) of the coarse aggregate particles shall have at least two (2) mechanically fractured faces. Asphalt extraction and sieve analysis shall be performed in accordance with ASTM D-2172-A or B, AASHTO T-164-A or B, and AASHTO T-30.

B. Fine Aggregate

The fine aggregate shall consist of material free of organic or other objectionable matter. The fine aggregate, either naturally combined with the coarse aggregate or separately obtained and mixed therewith, shall be of such character that the composite material will conform to the gradation and other requirement specified.
C. Gradation

The composite mixture of coarse aggregate and fine aggregate, processed as hereinafter specified, shall conform to the following gradation limits:

<table>
<thead>
<tr>
<th>Sieve Size</th>
<th>Percentage Passing By Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>1&quot;</td>
<td>100</td>
</tr>
<tr>
<td>3/4&quot;</td>
<td>70-100</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>50-85</td>
</tr>
<tr>
<td># 4</td>
<td>35-65</td>
</tr>
<tr>
<td># 10</td>
<td>25-50</td>
</tr>
<tr>
<td># 40</td>
<td>10-30</td>
</tr>
<tr>
<td># 80</td>
<td>5-20</td>
</tr>
<tr>
<td># 200</td>
<td>2-10</td>
</tr>
</tbody>
</table>

Asphalt Content: 2.7% - 4.7%

Moisture Content: 3.5% Max

The asphalt content of RAP delivered to the project shall be determined on the individual extraction test results and not an average of extractions conducted.

Article 8.3 Construction

The RAP shall be placed to the lines, grades, and thicknesses shown on the Drawings and shall consist of the materials specified. The RAP shall provide a smooth stabilized paved surface on which vehicular traffic can drive.

A. Preparation of Subbase

Subbase preparation shall be completed by others in accordance with Division 20, Section 20.11 - Grading Existing Surfaces with the compaction density modified to ninety-five percent (95%).

B. Placing

The approved RAP material shall be deposited and spread uniformly on the prepared subbase in one uniform layer to the required contour and grades and to such loose depth that when compacted to the density required will achieve the specified thickness. Portions of the layer which become segregated in spreading shall be remixed to the required gradation.
C. Compacting

The RAP shall be compacted to at least ninety-five percent (95%) of maximum density as per AASHTO T-180D. In all places not accessible to the rolling equipment, the mixture shall be compacted with tamping equipment capable of attaining the specified density. Blading, rolling and tamping shall continue until the surface is smooth and free from waves and inequalities. If at any time the mixture is determined to be above or below optimum moisture, it shall be aerated by means of blade graders, harrows or other approved equipment or moisture added until the moisture content is such that the surface can be recompacted and finished as above. In place compaction shall be accomplished with a double-drum vibratory asphalt compactor with a minimum of fifteen thousand (15,000) pounds of dynamic force per drum. All requests for equipment substitution shall require a current certification test, identifying the capability of the equipment to meet the required specifications.

D. Smoothness Test

The surface of the RAP, when finished, shall not show any deviation in excess of three-eighths inch (3/8”) when tested with a ten foot (10’) straightedge applied parallel with and at right angles to the centerline of the area to be paved. Any deviation in excess of this amount shall be corrected by loosening, adding, or removing material and reshaping and compacting to satisfy the above requirement.

The Contractor shall furnish a ten foot (10’) long straightedge and shall, in the presence of the Engineer, straightedge test the entire surface.

E. Seal Coat

The surface of the RAP shall be seal coated in accordance with the provisions in Section 40.02 – Seal Coat. The sealant shall be considered incidental to the cost of RAP construction.

Article 8.4 Measurement

The RAP shall be measured in tons of materials delivered and placed in accordance with these Specifications and adjusted for excess moisture as provided. Said measurement may include moisture up to a maximum of three and one-half percent (3.5%) of dry weight of the material.

When tests by the Engineer indicate that moisture contents in excess of three and one-half percent (3.5%) may be occurring consistently, the frequency of testing will be increased as necessary and the results averaged over a period of one week. When this average is greater than three and one-half percent (3.5%), the tonnage as measured over the above period shall be reduced by the difference. No credit will be due the Contractor when moisture content is less than three and one-half percent (3.5%). Testing will be done in accordance with AASHTO T-255.
Article 8.5  Basis of Payment

Payment for this work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish &amp; Install RAP</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 40.09   BITUMINOUS SURFACE TREATMENT

Article 9.1   Description

The Work under this Section consists of the performance of Work required for the construction of a seal coat of asphalt cement and cover aggregate on an asphalt street surface.

Article 9.2  Asphalt

The Contractor shall submit a certified analysis from the refinery laboratory to the Engineer for review and approval. A copy of the certified analysis shall accompany each shipment of asphalt to the Project. The Engineer shall reserve the right to make check tests of the asphalt received on the Project site, and if the asphalt is not in accordance with the certified analysis, he may reject the material.

The asphalt required by these Specifications shall conform to the requirements of The Asphalt Institute for the type and grade shown below:

Asphalt for Bituminous Surface Treatment CRS-2

Article 9.3  Aggregates

Aggregate shall consist of crushed gravel and shall be sound, durable, free of adherent coatings of clay, dirt, dust or any other objectionable matter, and shall have a percentage of wear not to exceed forty (40) after five hundred (500) revolutions, as determined by the ASTM C-131. Not less than sixty percent (60%) by weight of crushed gravel shall consist of crushed pieces having two (2) or more faces having freshly fractured face.

Aggregate material shall have the following gradation:

A.  Cover - 1st Course

<table>
<thead>
<tr>
<th>Sieve Designation</th>
<th>Percentage By Weight Passing</th>
</tr>
</thead>
<tbody>
<tr>
<td>3/4&quot;</td>
<td>100</td>
</tr>
<tr>
<td>1/2&quot;</td>
<td>90-100</td>
</tr>
<tr>
<td>3/8&quot;</td>
<td>40-75</td>
</tr>
<tr>
<td># 4</td>
<td>0-15</td>
</tr>
<tr>
<td># 8</td>
<td>0-5</td>
</tr>
<tr>
<td># 200</td>
<td>0-1</td>
</tr>
</tbody>
</table>
B. Cover Aggregate - 2nd Course

<table>
<thead>
<tr>
<th>Sieve Designation</th>
<th>Percentage By Weight Passing</th>
</tr>
</thead>
<tbody>
<tr>
<td>3/8&quot;</td>
<td>100</td>
</tr>
<tr>
<td># 4</td>
<td>85-100</td>
</tr>
<tr>
<td># 8</td>
<td>0-25</td>
</tr>
<tr>
<td># 200</td>
<td>0-2</td>
</tr>
</tbody>
</table>

**Article 9.4 Equipment**

A. General

All equipment used on this Work shall be of sufficient size and in such mechanical condition as to meet the requirements and to produce the Work to the specified quality.

B. Pressure Distributor

The bitumen distributor shall be pneumatic-tired self-propelled, and shall have a capacity of not less than eight hundred (800) gallons. It shall be equipped with an independently-operated bitumen pump, tachometer, pressure gauges, volume metering devices, a thermometer for reading the tank temperature and a hose attachment suitable for applying the bituminous material to spots unavoidably missed by the distributor. The independently-operated bitumen pump shall be equipped with a tachometer calibrated in revolutions per minute. The distributor shall also be equipped to agitate and circulate the bituminous material during the heating process.

Spray bars shall be the circulating type with extensions, available for distributing width from eight to twenty-one feet (8' to 21') by one foot (1') increments. The nozzles shall give uniform distribution and shutoff shall be quick and positive so as to prevent dripping. The distributor shall be designed and equipped to distribute the bituminous material uniformly at consistent surface speeds, at uniform temperatures with various surface widths, at known and maintained rates of five-hundredths (0.05) to two (2.0) gallons per square yard within a tolerance of five percent (5%), and through a pressure range from twenty-five (25) to seventy-five (75) pounds per square inch. Air pressure type equipment may be used only upon written approval from the Engineer.

C. Cover Aggregate Spreader

The spreader shall be self-propelled and capable of spreading the cover material uniformly for widths of eight to sixteen feet (8' to 16') in one foot (1') increments, and adjustable to spread uniform layers of ten (10) to fifty (50) pounds per square yard. Revolving plate type chip spreaders will not be approved.
D. Rollers

Rollers shall be self-propelled, pneumatic-tire, weighing not less than five (5) tons or more than eight (8) tons. Rolling shall follow closely on spreading of aggregate.

E. Hauling Equipment

The cover aggregate shall be transported from the plant to the site in trucks having tight, clean smooth beds.

F. Miscellaneous Equipment

A power broom and all necessary hand tools, thermometers, etc., shall be provided by the Contractor.

Article 9.5 Construction

A. Surface Preparation

The existing road surface shall be graded smooth to the satisfaction of the Engineer. There shall be no visible ruts, holes, or large rocks protruding from the surface to be treated. Existing soft spots in the road base shall be compacted or excavated and backfilled with a suitable base course material. The area to be treated in any one operation shall be as indicated on the Drawings.

B. Weather Limitations

Bituminous material shall not be placed during rainy or threatening weather, or when the moisture on the surface to be treated would prevent satisfactory bond. The surface coats shall not be applied when the air temperature is below fifty degrees (50º) Fahrenheit, unless otherwise approved by the Engineer.

C. Heating and Application of Bituminous Material

Bituminous material shall be heated in such a manner as to insure heating of the entire mass with an efficient and positive control at all times. A curing period of twenty-four (24) hours may be required between the application of the prime coat and the next application of bituminous material, during which time the surface shall be kept in repair. The asphalt for the surface treatment shall be applied at a temperature between one hundred thirty and one hundred eighty degrees (130º and 180º) Fahrenheit and at a rate of 0.30 to 0.45 gallons per square yard for the first coat and 0.25 to 0.40 gallons per square yard for the second coat. Thermometers shall be provided by the Contractor so that temperatures can be observed at all times.

Bituminous material shall be uniformly applied by means of a pressure distributor in the amount per square yard specified. The quantity of material as measured by the volume measuring device of the distributor shall not vary from the true quantity as herein specified by more than five percent (5%). Bituminous material shall not be
applied until sufficient cover aggregate is on hand to cover the area to be applied at a particular application. If the aggregate spreader is delayed by breakdown or operational difficulties, application of bituminous material shall cease until the Engineer is satisfied that delays will not recur.

D. Preparation and Application of Cover Aggregate

Cover material shall be sufficiently dried when it comes in contact with bituminous material so that a satisfactory bond or coating is obtained.

When the prime coat has sufficiently cured, the asphalt for the surface coat shall be applied to the surface. Immediately following this application of bituminous material, the first coat of cover aggregate shall be uniformly spread over the surface with an approved mechanical spreader at a rate of thirty (30) to forty (40) pounds per square yard. A minimum curing period of twelve (12) hours will be required before the second coat of asphalt and cover aggregate is applied. The rate of application of the second coat of cover aggregate shall be twenty (20) to thirty (30) pounds per square yard. Cover material shall be applied continuously and without delay until the particular application is covered. Whenever possible successive strips shall be applied before the previous strip has cooled. Cover material shall not be spread on the six inches (6”) adjacent to an unprotected edge until the next strip of bituminous material has been applied. Rolling shall immediately follow the application of each coat of cover aggregate and shall continue until the surface is accepted as determined by the Engineer.

E. Maintenance of Surface

After application of the second coat of cover material, the surface shall be maintained by the Contractor, at his expense, for a period of three (3) days. During this period the Contractor shall, at least once daily, redistribute the cover material that has become displaced by traffic, by means of brooms, a drag or other method satisfactory to the Engineer.

When all possible aggregate has been imbedded in the bituminous material on each course to the satisfaction of the Engineer, the Contractor shall sweep each course surface of all excess material and remove it to a designated area, unless otherwise directed by the Engineer.

Article 9.6 Measurement

Bituminous material and cover aggregate course will be measured by weight in tons (2,000 lbs).

Legible weight tickets shall be submitted to the Engineer for all bituminous material and cover aggregate delivered to the Project site for use in the Work. All weight tickets shall contain, at a minimum, the following information:

1. Weight ticket serial number;
2. Vehicle identification number;
3. The date and time the load was weighed;
4. The tare weight of the vehicle;
5. The gross weight of the loaded vehicle, as registered on the scale; and
6. The legal gross weight of the vehicle, as permitted by Anchorage Municipal Code Sections 9.46.090 or 9.46.100.

The Owner shall not pay for that portion of any load in excess of the legal gross weight for the vehicle.

**Article 9.7 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>B.S.T. Asphalt for Prime Coat (Type &amp; Grade)</td>
<td>Ton</td>
</tr>
<tr>
<td>B.S.T. Asphalt (Type &amp; Grade)</td>
<td>Ton</td>
</tr>
<tr>
<td>B.S.T. Cover Aggregate (1st Coat)</td>
<td>Ton</td>
</tr>
<tr>
<td>B.S.T. Cover Aggregate (2nd Coat)</td>
<td>Ton</td>
</tr>
</tbody>
</table>
SECTION 40.10 PAVEMENT ROTOMILLING

Article 10.1 General

The Work under this Section consists of furnishing all plant, labor, supervision, equipment, and material for performing all operations necessary for the removal and disposal of pavement to a depth designated on the Drawings with a power-operated profile machine designed for this specific purpose.

Article 10.2 Construction

Pavement shall be removed by rotomilling, planning, or grinding to a final surface smooth enough for temporary traffic and repaving with no additional preparation other than sweeping and application of a tack coat. Additional rotomilling, grinding, or milling by the power-operated profile machine may be necessary around manhole covers, valve boxes, survey monument cases, etc. The depth of asphalt removal under this Section may vary.

The Contractor shall dispose of the removed pavement tailings at a location designated by the Municipal Street Maintenance Department. The Contractor shall coordinate the exact location with the Street Maintenance Control Center. If the removed pavement material under this Section contains objectionable material within it, as identified by the Engineer, then Contractor shall dispose of this material in accordance with Division 10, Section 10.04, Article 4.9 - Disposal Sites.

Article 10.3 Measurement

Pavement removed by rotomilling, planning or grinding shall be measured by the square yard of pavement designated for and actually removed. No additional payment will be made regardless of depth or additional rotomilling around utility appurtenances.

Article 10.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Pavement Rotomilling</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 40.11 REMOVE AND REPLACE ASPHALT SURFACING

Article 11.1 Description

The Work under this Section consists of the performance of all Work required for removing, disposing of, and replacing existing asphaltic surfacing or cold mix asphalt, including leveling course and existing traffic markings, as indicated on the Drawings.

Article 11.2 Materials

All materials used shall conform to the requirements of these Specifications and other agencies (if any) having jurisdiction over the pavement being replaced.

Cold mix asphalt material shall be that located at the Kloep Maintenance Station.

Article 11.3 Construction

All construction practices, tests and other controls shall conform to the Division 20 - Earthwork, and this Division.

Contractor shall remove existing asphalt surfacing and leveling course, regardless of thickness. Replacement asphalt surfacing and leveling course thickness shall be shown on the Drawings.

Contractor shall neatly and cleanly saw cut existing asphalt surfacing to be removed prior to removal. Saw cuts shall be a minimum of two inches (2") deep and shall be straight and parallel. If any portion of the remaining asphalt surfacing is undercut by trenching operations, the Contractor shall saw cut, remove, and replace the affected area at his own expense.

Contractor shall apply tack coat to all contact surfaces including curbing, gutters, manholes, and other structures with a thin, uniform coating prior to placing asphalt. Butt joints on previously placed cooled pavement shall be tack coated prior to continuing the paving operation.

Asphaltic concrete paving replacement will be performed by utilizing a mechanical spreader and will be compacted by a mechanical roller weighing not less than ten (10) tons, except that where the area of the asphalt replacement patch is less than one hundred (100) square feet, a mechanical spreader need not be employed.

Small areas inaccessible to roller shall be tamped to produce a compression and surface texture equivalent to that produced by the specified rolling. Hand tampers shall have a maximum tamping face of fifty (50) square inches and a minimum weight of twenty-five (25) pounds.

The Contractor shall replace traffic markings, in kind, in accordance with Division 70, Section 70.10 – Traffic Markings.
Prior to winter shutdown, Contractor shall haul, place, and compact cold mix asphalt on areas where asphalt has been removed. Rejuvenated hot mix asphalt will be supplied by the Municipality of Anchorage at a rate of approximately two (2) tons per hour. The rejuvenated hot mix will be located at the Kloep Maintenance Station, 5701 Northwood Drive. Contractor shall coordinate with Maintenance & Operations Department, Street Maintenance Division, at 343-8277, when the patch can be obtained. Contractor shall haul from the Kloep Maintenance Station to the project site.

Contractor shall keep all asphalt surfacing designated for removal free from objectionable material (concrete, steel, etc.). Contractor shall dispose all removed asphalt surfacing at the Kloep Maintenance Station, 5701 Northwood Street. Contractor shall coordinate exact location and time of delivery with the Maintenance & Operations Department, Street Maintenance Division, at 343-8102. If the removed asphalt surfacing, under this Section, contains objectionable material, as identified by the Engineer, Contractor shall dispose of this asphalt surfacing in accordance with Division 10, Section 10.04, Article 4.9 – Disposal Sites.

**Article 11.4  Measurement**

Removing, disposing of, and replacing existing asphalt surfacing and cold mix asphalt, including leveling course, is measured per square yard, complete in place.

**Article 11.5  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Asphalt, Replace with Municipal-Supplied Hot Mix Asphalt</td>
<td>Square Yard</td>
</tr>
<tr>
<td>Remove Asphalt, Replace with Asphalt Surfacing (Class)</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
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MISCELLANEOUS
DIVISION 40
INDEX OF STANDARD DETAILS

40-1  Typical Resurfacing Detail - Gravel
40-2  Typical Resurfacing Detail - Non-Gravel Surfaces
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1. CONTRACTOR SHALL CONSTRUCT A 1% CROWN WITH THE PEAK CENTERED OVER THE CENTERLINE OF THE EXCAVATION.
NOTES:

1. ENGINEER OR PERMITTING AGENCY MAY DIRECT ADDITIONAL AMOUNTS OF SURFACE REPLACEMENT MATERIALS AND/OR TYPE II CLASSIFIED FILL & BACKFILL, BASED UPON FIELD CONDITIONS.

2. AFTER TRENCH BACKFILL HAS BEEN COMPACTED, CONTRACTOR SHALL SAW CUT (REF. SECTION 40.02.5.J) AND REMOVE AN ADDITIONAL 12" FROM EACH EDGE OF THE ORIGINAL CUT. ENGINEER MAY REQUIRE ADDITIONAL REMOVAL, IF THE EXISTING SURFACING HAS BEEN LIFTED IN THE REMOVAL PROCESS OR IF THE JOINT DOES NOT OCCUR ON UNDISTURBED MATERIAL. TRIM AND SQUARE THE EDGES OF EXISTING SURFACING, AND REMOVE LOOSE MATERIALS BEFORE PLACING PAVEMENT. CONTRACTOR SHALL PAINT SURFACES AND EDGES OF EXISTING PAVEMENT WITH HOT ASPHALT CEMENT AS SPECIFIED IN THE CONTRACT DOCUMENTS OR AS APPROVED BY THE ENGINEER.

3. MAXIMUM LIFT THICKNESS IS 2’ UNLESS OTHERWISE SPECIFIED IN THE DRAWINGS OR APPROVED BY THE ENGINEER.

4. THIS DETAIL APPLIES TO ALL NON–GRAVEL SURFACES INCLUDING, BUT NOT LIMITED TO, PAVEMENT, RECYCLED ASPHALT PAVEMENT (RAP), AND BITUMINOUS SURFACE TREATMENT, ALSO KNOWN AS CHIP SEAL.
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FOR SANITARY SEWERS
DIVISION 50
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SECTION 50.01 GENERAL

Article 1.1 Scope of Work

The Work covered by these Specifications consists of providing all plant, labor, equipment, supplies, material, transportation, handling and storage, and performing all operations necessary to complete the construction for pipe laying, jointing, and testing of sanitary sewers.

Requirements for earthwork including trench excavating and backfill are specified in Division 20 - Earthwork.

Article 1.2 Applicable Standards

The latest revision of the following standards of the American Society of Testing and Materials (ASTM), the American Association for State Highway and Transportation Officials (AASHTO), the American Standards Association (ASA), and the American Water Works Association (AWWA) are hereby made a part of these Specifications.

ASTM A48 Specifications for Gray Iron Castings
ASTM 438 Traverse Testing of Gray Cast Iron
ASTM A746 Specification for Ductile Iron Gravity Sewer
ASTM C14 or ASTM C14M [Metric] Specification for Concrete Sewer, Storm Drain and Culvert Pipe
ASTM C76 or ASTM C76M [Metric] Specification for Reinforced Concrete Culvert, Storm Drain and Sewer Pipe
ASTM C150 Specification for Portland Cement
ASTM C206 Specification for Finishing Hydrated Lime
ASTM C443 or ASTM C443M [Metric] Specification for Joints for Circular Concrete Sewer & Culvert Pipe, Using Rubber Gaskets
ASTM C478 or ASTM C478M [Metric] Specification for Precast Reinforced Concrete Manhole Sections
ASTM D256 Test Methods for D-C Resistance of Plastics and Electrical Insulating Materials
ASTM D2321  Recommended Practice for Underground Installation of Thermoplastic Sewer Pipe
ASTM D3034  Specification for Type of PSM Poly (Vinyl Chloride) (PVC) Sewer Pipe and Fittings
ASTM D3035  Specification for Polyethylene (PE) Plastic Pipe (SDR-PR) Based on Controlled Outside Diameter
ASTM D3350  Specification for Polyethylene Plastic Pipe and Fittings Materials
AASHTO M45  Sand for Cement Mortar
AWWA C104/ ANSI A21.4  Cement-Mortar Lining for Ductile-Iron Pipe and Fittings for Water
AWWA C105/ ANSI A21.5  Polyethylene Encasement for Ductile Iron Piping for Water and Other Liquids
AWWA C111/ ANSI A21.11  Rubber-Gasket Joints for Ductile-Iron and Gray-Iron Pressure Pipe and Fittings
AWWA C151/ ANSI A21.51  Ductile-Iron Pipe, Centrifugally Cast in Metal Molds or Sand-Lined Molds, for Water or Other Liquids
AWWA C901  Standard for Polyethylene (PE) Pressure Pipe and Tubing, 1/2 in. Through 3 in., for Water Service
UPC  Uniform Plumbing Code, latest edition adopted by the Municipality, and current local amendments

Article 1.3  Surveys

Survey shall be performed by the Contractor per Division 65 - Construction Survey.

Article 1.4  Concrete and Mortar

A.  Miscellaneous Concrete

All concrete used in the construction of sanitary sewer systems with the exception of precast manholes, manhole risers, cones, and reinforced concrete pipe shall be Class A-3. Concrete Work shall conform to Division 30 - Portland Cement Concrete.

B.  Mortar

Cement for mortar used in the construction of sanitary sewer systems shall conform with the requirements of ASTM C-150, Type II. Sand shall conform with the requirements of AASHTO M-45. The mortar shall be composed of one (1) part cement and three (3) parts sand. The addition of lime is not permitted. The use of five (5) minute or fast-cure mortar is prohibited.
Article 1.5 Insulation

Rigid board insulation required for frost protection of sanitary sewer mains and services shall be high density extruded polystyrene, Minimum 60 PSI, equivalent to R-20 per four inch (4") thick insulation. The design engineer may request authorization from the AWWU plan reviewer, prior to plan approval, to substitute expanded polystyrene rigid board insulation if the geotechnical report reflects suitable soil conditions. The designer shall submit analysis and justification demonstrating the insulation will not be exposed to seasonal ground water influxes or a high soil moisture content. Subsequent to AWWU’s review and approval, all Drawing references to substitute expanded for extruded insulation shall reflect “Expanded Polystyrene Rigid Board Insulation meeting ASTM D2842”.

Article 1.6 Payment - General

Payment for all Work included in this Division shall be paid for in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described.
SECTION 50.02   FURNISH AND INSTALL PIPE

Article 2.1   Description

The Work under this Section consists of the performance of all operations pertaining to furnishing and installing pipe for sanitary sewer systems.

In the case of Owner-furnished pipe, the Owner will allot to the Project pipe to accomplish the project in amounts, exactly matching the Contractor's pay quantities for pipe. Any surplus pipe left over from this allotment at the end of the Project shall be returned from the Contractor's job sites to the Owner's designated pipe yard. If the Contractor withdraws from the Owner's pipe yard more than the amount required to match the payment quantities, the Contractor shall pay the Owner on the basis of the Owner's invoice price for pipe (including freight), plus a ten percent (10%) overhead to reimburse the Owner's invoice price for pipe (including freight), plus a ten percent (10%) overhead to reimburse the Owner for handling, warehouse, inspection, and administration.

Article 2.2   Materials

A. General

All piping shall be in accordance with the Contract Documents conforming to the size and class shown and specified. Changes in class shall be made within one-half of a pipe length of the station indicated on the Drawings. The use of pipe containing asbestos materials shall be prohibited.

B. Ductile Iron Pipe

Ductile iron pipe shall conform to requirements of ASTM A-746 (AWWA C-151) and Cement Mortar shall conform to the requirements of AWWA C-104. Class 50 pipe shall be used, unless otherwise required by the Contract Documents. Fittings shall be cast iron or ductile iron and all bells conforming to AWWA C-104 except that so-called "short body" fittings, otherwise meeting AWWA Specifications may be used. Rubber gasket joints for ductile iron pipe fittings shall conform to the requirements of AWWA C-111.

C. Cast Iron Pipe

All cast iron pipe and fittings shall be hub and spigot service weight soil pipe meeting the requirements of ASTM A74. Gaskets shall meet the requirements of ASTM C564.

D. Concrete Pipe and Fittings

Reinforced concrete pipe and fittings shall conform to the requirements of ASTM C-76. Non-reinforced concrete sanitary sewer pipe shall conform to the requirements of ASTM C-14.
E. Concrete Pipe Joints

Joints for concrete pipe shall conform to the requirements of ASTM C-14 and ASTM C-443. Joints shall be of the "O" Ring type and shall be subject to the approval of the Engineer as to configuration. All repair clamps shall be approved stainless steel clamps.

F. High Density Polyethylene Pipe (HDPE)

The pipe and fitting material shall have a cell classification of 445574C in accordance with ASTM D3350. In addition, the material must exceed 1000 hours when tested in accordance with the Ring Environmental Stress Crack Resistance Test (Radar Ring Test) with fewer than twenty percent (20%) failures. Also, the extruded pipe shall have impact strength greater than three (3) cubic feet per inch when tested in accordance with the ASTM D256 (Izod Pendulum Impact Test).

The pipe shall be homogeneous throughout and free of visible cracks, holes, foreign inclusions or other injurious defects. It shall be uniform in color, opacity, density and other physical properties.

Butt fusion of the pipe and fittings shall be performed in accordance with the pipe manufacturer's recommendations as to equipment and technique. The fusion operation shall be performed by an individual who has demonstrated the ability to fuse polyethylene pipe in the manner recommended by the pipe supplier. The pipe supplier shall supply a representative to instruct the Contractor's crew on Butt Fusion and installation and witness the first twenty joints.

Alternate coupling methods for HDPE pipe shall not be used unless accepted by the Engineer in conformance with the requirements of Division 10, Section 10.05, Article 5.7 - Materials. Any request to consider an alternate coupling method in the Work and/or approval of its use, should it be accepted, shall not cause an increase in the cost of the Work to the Owner.

Article 2.3 Construction

A. Excavation and Backfill

Excavation and backfill for furnishing and installation of sanitary sewer pipe shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

The Contractor shall remove and dispose of all sewage-saturated soils encountered within the trench area. All sewage-saturated soils shall be considered unsuitable material. Sewage-saturated soils may not be used as fill material anywhere within the Municipality and shall be disposed of at the Municipal Landfill. There shall be no separate payment for removal and disposal of sewage-saturated soils. Removal and disposal of sewage-saturated soils shall be considered incidental to the pay item: Furnish and Install Pipe
B. Pipe Grade and Alignment

Variance of individual pipe sections from established line and grade shall not be greater than those listed in the table below, providing that such variance does not result in a level or reverse sloping invert.

<table>
<thead>
<tr>
<th>Diameter (Inches)</th>
<th>Allowance Tolerance (Feet)</th>
</tr>
</thead>
<tbody>
<tr>
<td>8</td>
<td>0.03</td>
</tr>
<tr>
<td>10</td>
<td>0.03</td>
</tr>
<tr>
<td>12</td>
<td>0.03</td>
</tr>
<tr>
<td>14</td>
<td>0.04</td>
</tr>
<tr>
<td>16</td>
<td>0.04</td>
</tr>
<tr>
<td>18*</td>
<td>0.05</td>
</tr>
</tbody>
</table>

*Note: For all pipe sizes over eighteen inches (18") in diameter, variance shall not exceed five hundredths feet (0.05').

During the progress of the Work, the Contractor shall provide instruments such as transits, levels, laser devices, and other facilities for transferring grades from offset hubs or for setting of batter boards or other construction guides from the control points and bench marks provided by the Contractor. The Contractor shall provide qualified personnel to use such instruments and who shall have the duty and responsibility for placing and maintaining such construction guides. The Contractor shall notify the Engineer 48 hours prior to taking measurements on newly installed section of line and/or appurtenances for Record Documents.

If the method of transferring grades from the offset hubs to the pipe require batter boards, they shall be at least one by six inches (1" x 6") supported on two by four inch (2" x 4") stakes or approved metal rods and shall be placed every twenty-five feet (25'). At least three boards must be in place at any given time to facilitate checking of line and grade. Both line and grade shall be checked and recorded in a field book for each piece of pipe laid, except at tunnels where methods acceptable to the Engineer shall be used to carry forward line and grade.

The practice of pushing in uncompacted backfill over a section of pipe to provide a platform for transit and level alignment and grade observations shall be subject to the approval of the Engineer. If intermittent backfilling is allowed, backfilling shall be accomplished in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.
C. Pipe Laying

All pipe shall be laid with Type II-A Classified Fill and Backfill as bedding unless otherwise required by the Contract Documents or directed by the Engineer.

Pipe laying shall in all cases proceed upgrade with the spigot ends of the pipe pointing in the direction of the flow. Each pipe shall be laid true to line and grade and in such a manner as to form a close concentric joint with the adjoining pipe. The alignment of the installed pipe shall appear straight to visual observation and shall be such that a full circle of light can be seen between manholes, etc., when sighting along all points of the pipe circumference. Each section of pipe shall be handled carefully and placed accurately; each pipe shall be joined in accordance with the pipe manufacturer’s recommended standards. Each section of pipe shall be properly supported to ensure true alignment and an invert which is smooth and free from roughness or irregularity.

The Contractor shall stagger the joints for sanitary sewer pipe such that no sewer pipeline joint shall be closer than nine feet (9’) measured horizontally (outside of pipe to outside of pipe) from its intersection with either water mains or water services encountered in the Work.

The Contractor shall take every precaution to preclude foreign debris from entering the sanitary sewer system. Temporary screening techniques of the downstream manholes proposed for use by the Contractor shall first be reviewed and approved by the Engineer prior to their use in the Work. Contractor shall be responsible for removing and cleaning any foreign debris that enters the sanitary sewer system. All costs associated with the removal of foreign debris from the sanitary sewer system resulting from the Contractor’s activities shall be considered incidental to the Contract.

At all times, when Work is not in progress, open ends of pipe and fittings shall be securely and satisfactorily closed so that no undesirable substance will enter the pipe or fittings.

Where a project outfalls into an existing sanitary sewer, construction of physical connection to the existing line shall be delayed until all upstream underground construction, including exfiltration testing, is complete and accepted unless special permission is granted by the Owner. Care shall be exercised during construction, flushing, and testing operations of the connecting link to assure that water or any foreign debris is not diverted into any portion of a sanitary sewer line in service or a sanitary sewer line which is not a portion of the construction project for which the Contractor is responsible.

Pipe shall not be laid when the bottom of the ditch or the sides to one foot (1’) above the pipe are frozen. Backfill material shall not contain frozen material. The trench shall not be left open during freezing weather so that the temperature of the material near the pipe goes below freezing.
All ductile iron pipe shall be encased in one layer of polyethylene encasement in accordance with Section 50.13 - Polyethylene Encasement.

D. Bedding of Ductile Iron Pipe for Sanitary Sewer Main

Sanitary sewer pipe and sanitary sewer service connections shall be bedded to the spring line in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill, and Standard Detail 20-8. Native materials may be used where bedding material above the spring line is eliminated and as approved by the Engineer; however, it must be compacted to ninety-five percent (95%) maximum density.

E. Laying Instructions for Concrete Pipe with "O" Ring Bell End Joint

To allow a watertight joint and to insure an installation which will allow the pipe to perform as designed, the following recommendations of the pipe manufacturer shall be observed.

1. Spigot groove and bell surface shall be clean and free of foreign material.
2. Apply joint lubricant freely to the bell including the tapered surface and completely coat the rubber gasket.
3. After placing gasket in groove, run a small tool completely around between gasket and groove to equalize gasket stretch.
4. Exercise care at first contact of the pipe. Avoid bumping which may damage spigot. Stop any swaying motion before contact is made.
5. To couple pipe, insert spigot slowly and carefully straight into bell, to allow the gasket to cushion the initial contact and center the spigot as it enters the tapered portion of the bell.
6. Complete joint should have spigot against inside bell shoulder. Inside joint space should not exceed one-half inch (1/2") for straight runs. Pulled joint deflections for alignment change shall comply with pipe manufacturer's recommended deflection limits.
7. Check all around pipe for rolled or "fishmouthed" gaskets after coupling.
8. Do not pick up and drop coupled pipe to adjust grade.
9. Ensure that the pipe is not supported only at the bell nor is the pipe barrel resting on a high spot. The bottom quarter of the pipe shall be uniformly supported through its length in order for the pipe to resist the design loads.
F. Laying Instructions for Other Pipe

All other pipe shall be laid in accordance with the manufacturer's published recommendations.

Article 2.4 Testing

A. General

The Contractor shall clean and flush all sanitary sewer pipe installed prior to testing and substantial completion inspection. Water and sewer main and service trenches shall be substantially backfilled and compacted.

All sanitary sewer pipe installed shall be subject to either an infiltration test or an exfiltration test. In those areas where, in the opinion of the Engineer, the water table is high enough to subject the pipe to a satisfactory infiltration test, it is not anticipated that an exfiltration test shall be required. In checking leakage, there will be no allowance made for external hydrostatic head.

Where in the opinion of the Engineer, the water table is not high enough to provide a satisfactory infiltration test, an exfiltration test shall be required.

The type of test (either infiltration or exfiltration) shall be determined by the Engineer. The Contractor shall have the option of choosing only one method (air or water) of testing for each section tested.

All wyes, tees, or ends of side sanitary sewer stubs and service connections shall be plugged or capped and the plug or cap shall be securely fastened to withstand the internal test pressures. Such plugs or caps shall be readily removable and their removal shall provide a socket suitable for extending the lateral connection.

All testing shall be considered a subsidiary obligation under Furnish and Install Pipe and is considered incidental to the Contract.

The Contractor shall take precaution to prevent sewage from entering the new sanitary sewer pipeline until it has been inspected, tested and accepted for operation by the Engineer. The Contractor may request inspection, testing and acceptance of incremental segments of the Work. An incremental segment shall be considered a mainline sanitary sewer with a completed manhole or cleanout at each end.

B. Exfiltration Test (Using Water)

On completion of a section of sanitary sewer between manholes or otherwise, the Engineer shall require that the ends of all pipe be plugged, including service connections, and the pipe subjected to a hydrostatic pressure. Generally all testing is to be conducted after backfilling, prior to resurfacing and after service connections are made.
A minimum head of six feet (6') of water above the crown at the upper end of the test section shall be maintained for a period of four (4) hours during which time it will be presumed that full absorption of the pipe body has taken place and thereafter for a further period of one (1) hour for the actual test of leakage. During this one-hour period, the measured loss shall not exceed the rate of fifty (50) gallons per inch diameter per mile per twenty-four (24) hours.

The above listed leakage rate shall also be applied to infiltration from ground water and infiltration or exfiltration in greater amounts will be cause for rejection of the sanitary sewer and all repairs necessary to meet these requirements and retesting shall be at the expense of the Contractor.

The maximum length of sanitary sewer for the above allowable leakage test shall be one thousand feet (1,000'). If it is not apparent that leakage test results between any two (2) manholes is satisfactory, then the Engineer may require subsequent tests to establish the more exact location of the leakage areas. Any section of sanitary sewer between any two (2) manholes that does not meet the above requirements shall be rejected and the Contractor, at his expense, shall make the necessary repairs to the sanitary sewer to meet the requirements, and shall make subsequent tests after repairs to assure compliance with the Specifications.

C. Exfiltration Test (Using Air)

The Contractor shall furnish all facilities and personnel for conducting the test under the observation of the Engineer. The equipment and personnel shall be subject to the approval of the Engineer. Joints only may be tested in pipe thirty-six inches (36") in diameter, or larger at the option of the Contractor.

The Contractor may desire to make an air test prior to backfilling for his own purpose. However, the acceptance air test shall be made after backfilling has been completed, and compacted.

Immediately following the pipe cleaning, the pipe installation shall be tested with low-pressure air. Air shall be slowly supplied to the plugged pipe installation until the internal air pressure reaches four (4.0) pounds per square inch greater than the greatest back pressure of any ground water in contact with the pipe. At least two (2) minutes shall be allowed for temperature stabilization before proceeding further.

The pipeline shall be considered acceptable when tested at an average pressure of four (4.0) pounds per square inch greater than the greatest back pressure of any ground water in contact with the pipe, if:

The total rate of air loss from any section tested in its entirety between manholes or between manholes and cleanout structures does not exceed two (2.0) cubic feet per minute, or the following table may be utilized as a guideline for a satisfactory test by air for pipe sizes shown:
### Pipe Diameter

<table>
<thead>
<tr>
<th>Pipe Diameter</th>
<th>Allowable Pressure Drop in 10 Minutes</th>
</tr>
</thead>
<tbody>
<tr>
<td>8&quot;</td>
<td>2.7 PSI</td>
</tr>
<tr>
<td>10&quot;</td>
<td>2.1 PSI</td>
</tr>
<tr>
<td>12&quot;</td>
<td>1.8 PSI</td>
</tr>
<tr>
<td>15&quot;</td>
<td>1.4 PSI</td>
</tr>
<tr>
<td>18&quot;</td>
<td>1.2 PSI</td>
</tr>
<tr>
<td>24&quot;</td>
<td>0.9 PSI</td>
</tr>
</tbody>
</table>

Pressure gauges shall be incremented in not more than 1/2 pound increments for accurate tests.

If the pipe installation fails to meet test requirements, the Contractor shall determine at his own expense the source or sources of leakage, and he shall repair (if the extent and type of repairs proposed by the Contractor are acceptable to the Engineer), or replace all defective materials or Workmanship. The completed pipe installation shall meet the requirements of this test or the alternative water exfiltration test before being considered acceptable.

Safety braces shall be required to hold plugs in place and to prevent the sudden release of the compressed air. Due to the large forces that could be exerted by an escaping plug during the testing of the pipe, workmen shall not be allowed in the manholes in which plugs have been placed while tests are being conducted. The Contractor's testing equipment shall be arranged in such a manner that a pressure relief device will prohibit the pressure in the pipeline from exceeding 10 PSI.

### D. Infiltration Test

Infiltration testing may be allowed at the Engineer's option when the natural ground water table is six feet (6') above the crown of the higher end of the test section. The maximum allowable limit for infiltration shall not exceed the rate of fifty (50) gallon per inch diameter per mile per twenty-four (24) hours.

The Contractor shall furnish all tools, equipment, and labor necessary to complete the tests and shall verify from his own observations, or preliminary tests, that each line conforms with this Specification before requesting the Engineer to observe and record the actual leakage.

The Engineer may require the Contractor to repair obvious leaks even though the total length of the test section falls within the maximum allowable leakage for the test used.
E.  Check of Line and Grade

After backfilling and cleaning, but before final acceptance, all sections of installed line may be checked for line and grade. Excluding service connections, all size sanitary sewer mains thirty inches (30") and smaller in diameter may be checked for line and grade by closed circuit television. A full circle of light must be seen and no pipe misplaced in line or grade. A physical inspection of the interior of all sanitary sewer line thirty inches (30") in diameter and above will be made before acceptance. Any excess deviation in line and grade shall be corrected by the Contractor prior to Final Acceptance of the Project.

Article 2.5  Measurement

Measurement for all sizes of pipe shall be based on the horizontal distances and will be from center to center of manholes or from center of manholes to center of cleanout bend.

Unless specifically identified for payment under a separate bid item, the unit price bid for Furnish and Install Pipe (size) (shape) (type) (material) (class and/or gage) shall include all labor, equipment and materials to furnish and install a functional sanitary sewer system including, but not limited to, the following incidental items: asphalt surfacing removal and replacement; concrete sidewalk, curb, and or gutter removal and replacement; clearing and grubbing; trench excavation and backfill; excavation dewatering; trench support system; furnishing and installing Type II-A Classified Fill and Backfill as bedding; compaction; installation of pipe, fittings, adapters, or other necessary appurtenances; polyethylene encasement; surveying; testing; disposal of unusable or surplus material; protection, bracing and or shoring of existing utilities; restoration of existing drainage patterns; removal and replacement of existing culverts, fences, landscaping, and other public or private improvements or natural features impacted by the Work; finish grading; and cleanup.

Article 2.6  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Where the Work includes disconnecting existing sanitary sewer services from an existing sewer main and reconnecting them to a new sewer main, the disconnection and reconnection of those existing sewer services will be considered incidental to the installation of the new sewer main.

Unit cost payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install Pipe (Size, Shape, Type Material, Class and/or Gauge)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 50.03 SANITARY SEWER MANHOLES

Article 3.1 General

The Work under this Section consists of the performance of all operations pertaining to the construction and installation of sanitary sewer manholes complete with frames and covers.

Article 3.2 Material

Materials used in the construction of manholes shall conform to the requirements of ASTM C-478 (AASHTO-199) and the Standard Details. Cones shall be Type (b), eccentric, unless otherwise approved.

Cement for mortar used in the construction of manholes shall conform to the requirements of ASTM C-150, Type II. Sand shall conform with AASHTO Specification M-45. The mortar shall be composed of one (1) part cement and three (3) parts sand. The joints shall be constructed to produce a smooth, regular watertight surface. Only enough water shall be added to provide plasticity in placing the mortar.

The tensile strength of the gray cast iron for manhole frames, pavement-adjusting rings and covers shall be 30,000 PSI minimum conforming with the requirements of ASTM A-48. The requirement for transverse breaking load shall be 2,000 pounds, conforming with the requirements of ASTM A-438. Frames and covers shall conform to the Standard Details. Where lockable manhole covers are specified, the Contractor shall submit Shop Drawings of the locking device for approval of the Engineer.

Gray iron castings shall have appropriate certifications and be individually marked in accordance with the requirements of AASHTO M-306. Castings which do not possess appropriate AASHTO M-306 certifications and markings shall be replaced by the Contractor at no expense to the Owner.

Each precast concrete barrel section, precast concrete eccentric cone section, concrete adjusting ring and manhole cover/frame shall be set and sealed by use of a plastic gasket joint sealer, as manufactured by Henry Company, Inc., Ram-Nek Sealant Division, or an approved equal.

All manhole joints shall be sealed with MacWrap external joint sealant, manufactured by MarMac Manufacturing Company, or approved equal. Seals shall be applied per manufacturer's published recommendations.

All exterior manhole concrete surfaces shall be coated for waterproofing with TUFF-N-DRI® brush grade foundation coating, or approved equal, applied per manufacturer’s recommendations.

Manholes shall be installed with no less than three (3) layers of 8-mil polyethylene encasement on the outside of the manhole.
Refer to Division 30, Section 30.01, Article 1.6 - Mix Requirements for Classes of Concrete, for specifications pertaining to Class A-3 concrete as required in forming manhole inverts. The use of Transite or Asbestos Cement (AC) pipe to form manhole inverts is prohibited.

Rubber waterstops used in pipe-to-manhole joints, shall be rings of resilient material that will fit snugly over a pipe. The resilient material shall be held firmly against pipe surface by means of a stainless steel mechanical take-up device which, when tightened, will compress the resilient material or, by a stretch, fit. The rubber waterstop shall be designed and installed so that leakage between pipe and manhole is eliminated. Material and manufacture of waterstops shall conform to applicable provision of the ASTM Standard Specifications for Resilient Connectors between Reinforced Concrete Manhole Structure and Pipes, ASTM C923. Waterstops at manhole pipe penetrations shall be sealed with “Z-lok” and “A-lok” manhole pipe connections per the Standard Details, or approved equal.

Reinforcement steel shall conform to the requirements of ASTM A-185, ASTM A-615, Grade 60 steel, or better, and the Standard Details.

**Article 3.3 Construction**

**A. General**

Excavation and backfill for furnishing and installing sanitary sewer manholes shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

The manhole frames and covers shall be brought to the grades shown on the Drawings. Manhole grade rings shall be set in and made secure by use of butyltight (or equal). (In paved streets, manhole grade rings and frames shall be placed on a full bed of mortar to prevent settlement.) Each manhole must have a minimum of one (1) six inch (6”) grade ring and a two inch (2”) pavement adjusting ring.

Use of, and installation of, a plastic gasket joint sealer (“Ram-Nek” or equal) for manhole construction shall be strictly in accordance with the manufacturer’s printed instructions. Gaskets shall be trimmed on the inside of the manhole to prevent the excess gasket material from entering the sanitary sewer lines.

All portions of precast manholes must be approved by the Engineer prior to installation in the sanitary sewer systems. The precast manhole manufacturer shall provide timely notice (at least two working days in advance) to allow time for the Engineer to arrange for necessary inspections. Installation, of manhole sections will not be allowed prior to the Engineers written approval. This approval does not relieve the Contractor of the responsibility for protection of manholes against damage during handling and installation.
Manholes shall be installed at the locations shown on the Drawings such that primary leads enter radially at the invert elevations specified. The base section shall be set plumb on a prepared surface.

Prior to backfilling, the Contractor shall apply TUFF-N-DRI® waterproofing to the exterior of the manhole, MacWrap (or equal) at all manhole joints and three (3) layers of 8-mil polyethylene encasement on the outside of the manhole.

In the case of poured-in-place manhole construction, if the Contractor elects to accomplish the manhole construction utilizing more than one continuous concrete pour, a keyed construction joint shall be used. These manholes shall have poured-in-place bases.

B. Sanitary Sewer Manhole Invert Construction

The invert channels shall be smooth and semicircular in shape conforming to the inside of the connecting sanitary sewer section. Changes in directions of flow shall be made by forming a smooth radius sized to allow adequate access of a T.V. camera and/or maintenance equipment into the served sanitary sewer pipe. Changes in size and grades of the channels shall be made gradually and evenly. The invert channels may be formed directly in the concrete of the manhole base, or may be formed and poured in place, or may be constructed by laying a full section of sanitary sewer pipe through the manhole and breaking out the top half after the surrounding concrete has hardened. The floor of the manhole outside the channels shall be smooth and shall slope towards the channels at a grade of one inch (1”/ft) per foot.

C. Additional Depth for Manholes

This item consists of the construction of additional depth to manholes over and above the standard depth specified below. Additional depth to manholes shall be constructed as per Standard Detail and designated as to type:

<table>
<thead>
<tr>
<th>TYPE</th>
<th>DEPTH</th>
</tr>
</thead>
<tbody>
<tr>
<td>&quot;A,&quot; &quot;B&quot; &amp; &quot;C&quot;</td>
<td>12 feet</td>
</tr>
</tbody>
</table>

D. Component Part Replacements

The Contractor shall take due care not to destroy or damage existing component parts of manholes that are to remain or be reset in place.

The Contractor shall furnish and install barrel sections and grade rings to adjust the top of sanitary sewer manholes to grade in accordance with Sections 50.18 – Adjust Sanitary Sewer Manhole Cone to Finish Grade and 50.19 - Adjust Sanitary Sewer Manhole Ring to Finish Grade, as shown in Standard Details 50-24 and 50-25. All materials used in the adjustment of sanitary sewer manhole cones
including mortar, steps barrel sections, block, etc., shall conform to the requirements for sanitary sewer manholes as outlined in Article 3.2 - Materials.

Installation of new sections shall be constructed to produce a smooth, regular, watertight surface.

E. Removal of Existing Manhole Component Parts

Upon removal of manhole component parts, the Contractor shall clean and prepare existing component parts prior to installation of replacement parts. This will include, but not be limited to, removing existing grout and Ramnek-type sealant from remaining and connecting component parts.

Materials that can be reused (manhole covers, frames, etc.) shall be salvaged and removed in a workmanlike manner and delivered to AWWU at 325 East 94th Court. The Contractor shall provide a disposal site for non-salvageable materials.

Article 3.4 Measurement

Manholes shall be measured as units complete in place. Depth of manholes will be based upon a measurement to the nearest foot, for payment purposes, from top of casting to the top of the base slab. All depths over the specified standard depth will be paid under "Additional Depth to Manholes."

Article 3.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Component parts of existing or new manholes shall be included in the unit price for the bid item being constructed, reset, or replaced, and shall be paid for by a cumulative total of each unit constructed.

Any excavation required in the removal or upgrade of sanitary sewer manholes shall be considered incidental to the bid item under construction.

Adjustments to grade in accordance with Sections 50.18 – Adjust Sanitary Sewer Manhole Cone to Finish Grade and 50.19 - Adjust Sanitary Sewer Manhole Ring to Finish Grade shall be incidental to the bid item under construction and no separate payment shall be made.

Related component parts to the bid items under construction (including steps, etc.) as shown in the Standard Details shall be incidental to that bid item.

If, in the opinion of the Engineer, the Contractor was negligent in damaging component parts of existing manholes to remain or be reset in place, the Contractor shall replace them in kind at his expense. If in the opinion of the Engineer, the damage was unavoidable,
replacement component parts may be furnished by AWWU and the Work paid for at the bid item price.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Sanitary Sewer Manhole (Type, Standard Depth)</td>
<td>Each</td>
</tr>
<tr>
<td>Additional Depth to Manhole (Type)</td>
<td>Vertical Foot</td>
</tr>
<tr>
<td>R&amp;R Manhole Frame and Cover</td>
<td>Each</td>
</tr>
<tr>
<td>R&amp;R Manhole Frame and Cover and Rings</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.04 WATERTIGHT MANHOLE FRAMES AND COVER

Article 4.1 General

The Work under this Section consists of providing all operations pertaining to the furnishing and installation of watertight manhole frames and covers.

Article 4.2 Material

Watertight frames and covers for manholes and similar appurtenances shall be of cast iron and conform to the dimension shown in the applicable Standard Details. The requirement for tensile strength of the gray cast iron shall be 30,000 PSI minimum in accordance with the requirements of ASTM A-48 and the requirement for transverse breaking load shall be 2,000 pounds in accordance with the requirements of ASTM A-438. Contact surfaces between frames and covers shall be machined to provide a uniform contact surface. Manhole covers shall have identification letters as shown on the Standard Details.

Article 4.3 Construction

Installation shall be performed in accordance with the manufacturer's written instructions and the Standard Details.

Article 4.4 Measurement

Watertight manhole frames and covers shall be measured as complete units in place.

Article 4.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment is made only for the additional cost of furnishing and installing the watertight frame and cover which exceeds the cost of the standard frame and cover included in the completed manhole unit price.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Watertight Manhole Frame and Cover</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.05  CONNECTIONS TO EXISTING SANITARY SEWER MANHOLES

Article 5.1  General

The Work under this Section consists of providing all operations pertaining to the Work required for connections to existing manholes.

Article 5.2  Construction

Excavation and backfill for connections to existing manholes shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

Connections to existing manholes shall be made by core drilling the new penetration into the manhole and providing an NPC Kor-N-Seal or approved equal pipe to manhole connector to produce a water tight seal. The use of impact tools to form new penetrations is prohibited.

Connection to existing manholes shall be made in a workmanlike manner, shall be water tight and have smooth flow surfaces and curves. The invert shall be brought into the existing manhole at the elevation shown on the Drawings. The downstream pipe in manholes shall be screened to prevent entry of mortar or other debris from entering the system.

Where a connection is made to an existing sanitary sewer manhole, the base shall be broken out if necessary to form a smooth channel in accordance with the construction requirements of a new manhole. Connections to existing sanitary sewer manholes will be allowed only after all portions of the Contractor's Work tributary to the connection point has been cleaned and flushed, inspected and tested. Under certain conditions, connections prior to the completion of the system may be permitted subject to the Engineer's prior written approval and the provision of suitable and adequate debris and sand traps and sumps upstream from the connection. If the connection to existing manhole occurs near the existing ladder rungs of the existing manhole, the Contractor shall remove the existing ladder rungs and install new ladder rungs so that the ladder rungs are not above a pipe penetration. (The Contractor may rotate the barrel sections and cone section of the manhole rather than removing old ladder rungs and installing new ladder rungs.)

Article 5.3  Measurement

Connection to existing manholes shall be measured as complete units in place.
Article 5.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section. Where the connection is made to a pipe stubbed out of the existing manhole, payment will not be allowed for the connection.

Payment shall be made on the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Connect to Existing Sanitary Sewer Manhole</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.06 CONSTRUCT SANITARY SEWER DROP CONNECTION

Article 6.1 General

The Work under this Section consists of providing all operations pertaining to furnishing and installing drop sanitary sewer connections to manholes.

Article 6.2 Materials

Pipe and fittings used in the construction of drop connections for sanitary sewers shall conform to the requirements of AWWA C-151/ANSI A21.51 for Class 50 pipe, and AWWA C104/ANSI A21.4 for fittings, and the Standard Details. Pipe penetrations into the manhole shall comply with Section 50.03, Article 3.2 – Material and Section 50.05, Article 5.2 – Construction. Pipe and fittings shall be restrained through the use of EBAA Iron MEGALUG® fittings or equal on all mechanical joints and U.S. Pipe FIELD LOK® gaskets or equal on all push-on joints.

Article 6.3 Construction

Excavation and Backfill for furnishing and installing drop sanitary sewer connections shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill. Installation of drop sanitary sewer connections shall be in accordance with the Standard Details.

Over-excavation under drop connection shall require compaction of not less than ninety-five percent (95%) of the maximum density prior to installation of the pipe and fittings, or the concrete cradle.

Refer to Division 30, Section 30.01 - General for requirements pertaining to Class A-3 concrete.

Article 6.4 Measurement

Drop sanitary sewer connections shall be measured as units, complete in place.

Article 6.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Sanitary Sewer Drop Connection</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.07   CONSTRUCT BEAVER SLIDE

Article 7.1   General

The Work under this Section consists of providing all operations pertaining to the construction and installation of beaver slides in a manhole.

Article 7.2   Material

Refer to Division 30, Section 30.01 - General, for requirements pertaining to Class A-3 concrete as required in forming beaver slide inverts.

Article 7.3   Construction

Beaver slides shall be constructed to provide a smooth and continuous channel directed into and with the flow of the receiving sanitary sewer and in accordance with the Standard Details.

Beaver slides are required where the invert of the connecting sanitary sewer is above the crown of the receiving sewer and the drop in the manhole does not exceed the maximum height shown on the Standard Details.

Article 7.4   Measurement

Beaver slides shall be measured as units complete in place.

Article 7.5   Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Beaver Slide</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.08 LATERAL CONNECTION TO EXISTING PIPE

Article 8.1 General

The Work under this Section consists of providing all operations pertaining to lateral connections to trunk and interceptor mains, as approved by the Utility.

Article 8.2 Construction

Lateral connections to existing sanitary sewer pipe shall be water tight and have smooth flow surfaces. The lateral shall be brought into the existing pipe in accordance with the Contract Documents, unless otherwise approved by the Engineer. The connection shall be made in a top quadrant of the pipe, and the lateral shall not intrude past the inside wall of the existing pipe.

Taps to reinforced concrete sanitary sewer pipe shall be made by use of an approved mechanical hole cutter. A tapping saddle shall be installed centered over the hole. Breaking into the pipes by use of a chipping gun, jackhammer, or other similar method will not be allowed.

Article 8.3 Measurement

Lateral connections will be paid for as a complete unit in place which includes all pipe and fittings from the manhole to the existing main.

Article 8.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Lateral Connection</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.09  DEEP SANITARY SEWER SERVICE RISERS

Article 9.1  General

The Work under this Section consists of providing all materials and operations pertaining to deep sanitary sewer service risers. Deep service risers shall be installed where the sanitary sewer is in excess of twelve feet (12’) deep and eight feet (8’) of cover can be maintained over the service. Deep service risers shall be fully restrained ductile iron pipe to the edge of right-of-way or easement. No more than two sanitary sewer service connections shall be installed on a single deep service riser.

Article 9.2  Material

All deep sanitary sewer service riser connections shall be constructed with the following materials:

   A. Ductile iron pipe with Tyton® joints and Mechanical Joint fittings.

   B. EBAA Iron MEGALUG®, U.S. Pipe Field LOK® Gasket, or approved equal.

Article 9.3  Construction

Excavation and backfill for furnishing and installing deep sanitary sewer service risers shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill. Contractor shall construct deep sanitary sewer service risers in accordance with the Standard Detail.

Article 9.4  Measurement

Service risers for deep sanitary sewer connections shall be measured as complete units in place.

Article 9.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Deep Sanitary Sewer Service Riser (Size)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.10 SANITARY SEWER SERVICE CONNECTIONS

Article 10.1 General

The Work under this Section consists of providing all materials and operations pertaining to the construction required for sanitary sewer service connections.

The Contractor shall notify the Engineer and property owners seventy-two (72) hours prior to any interruption of sanitary sewer service. The Contractor shall provide temporary service during the period of interruption.

If construction activities are to occur in areas other than existing easements and temporary construction permit areas, the Contractor shall secure a written Access Permit from the property owner prior to beginning construction. Such permission shall hold the Municipality of Anchorage harmless from any damage and claims sustained by the Contractor's operations within the permit area.

Article 10.2 Material

All gravity sanitary sewer service connects shall be constructed with class 50 ductile iron Tyton® and joint pipe.

All one and one-half inch (1.5”) and two inch (2”) force main sanitary sewers to be constructed with Type K copper tubing or HDPE SDR 11 (eleven) per the Standard Detail for this Work. All one and one-half inch (1.5”) and two inch (2”) force main sanitary sewer connections shall use service clamp per the Standard Detail.

All gravity services with less than five and one-half feet (5.5’) of cover shall be insulated with four inches (4”) of rigid board insulation in conformance with Section 50.01, Article 1.5 - Insulation.

All one and one-half inch (1.5”) and two inch (2”) force main sanitary sewers to be installed with minimum of ten feet (10’) of bury.

For gravity sewer services, connection to main shall be made with a Romac style CB Sewer saddle.

Article 10.3 Construction

Excavation and backfill for furnishing and installing sanitary sewer service connections shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

The service connections shall be bedded with non-frost susceptible material, with a fine granular texture and containing no material larger than one-half inch (1/2"). For gravity and force main sewers, bedding shall be placed the full extent of ditch and six inches (6”) above the pipe.
Construction shall be in accordance with the Standard Details. Multiple connections shall not be made any closer together than three feet (3'). The terminus of the house connection shall be sealed with a suitable stopper. Taps, where allowed for installation of saddles on to sanitary sewer pipes, shall be made with a mechanical hole cutter as manufactured by the Pilot Manufacturing Company or equal. Tee and wye saddles will be allowed on mains twelve inches (12”) and larger, wye saddles will be the only saddles allowed on mains smaller than twelve inches (12”). All gravity service connections to sanitary sewer mains shall be approved ductile iron pipe. All one and one-half inch (1.5”) and two inch (2”) force main sanitary service connections to sanitary sewer mains shall be approved Type K copper or HDPE SDR 11 (eleven) pipe.

Saddles for gravity sewer connections shall be placed over a circular hole sawed one-eighth inch (1/8”) larger than the inside diameter of the saddle. The strap(s) shall be tightened in accordance with the manufacturer’s instructions and centered over the hole sawed in the pipe being tapped. The hole shall be made above the spring line of the main being tapped.

All ductile iron pipe and/or cast iron pipe shall be encased in one layer of polyethylene encasement in accordance with Section 50.13 - Polyethylene Encasement. Sanitary sewer service connections shall be installed to the edge of right-of-way or edge of sanitary sewer easement of the lot being served and shall be permanently marked by means of a Carsonite (or equal) marker extending three feet (3’) above grade, painted green.

Record drawings shall include the pipe station of service connection at the main, service length, service invert elevations at the main and property line and distance to nearest property corner.

Minimum slopes for gravity sewer service connections shall be as follows:

<table>
<thead>
<tr>
<th>Pipe Diameter</th>
<th>Slope</th>
<th>Feet per foot</th>
</tr>
</thead>
<tbody>
<tr>
<td>4”</td>
<td>2.08%</td>
<td>0.0208</td>
</tr>
<tr>
<td>6”</td>
<td>1.00%</td>
<td>0.0100</td>
</tr>
<tr>
<td>8”</td>
<td>0.40%</td>
<td>0.0040</td>
</tr>
<tr>
<td>10”</td>
<td>0.28%</td>
<td>0.0028</td>
</tr>
<tr>
<td>12”</td>
<td>0.22%</td>
<td>0.0022</td>
</tr>
</tbody>
</table>

Upon exposing a stub-out, the Contractor is required to insure that the line is free and clear of obstructions prior to connection with the service extension.

If the service line is found to be either plugged or if a gravity sewer service is found to have reverse grade, the Contractor is required to notify the AWWU immediately or be liable for correcting the misalignment or unplugging the line at his expense. At the point of tie-in if No-hub pipe is exposed, a "Romac repair clamp" SC-1-450 or equal shall be used to connect to the on-property service line. If a "Ty-seal" hub is utilized, the use of a "Romac
repair clamp" or equal is not required. When using a bend at the point of tie-in, two (2) "Romac repair clamps" shall be used.

An Inspector for the AWWU shall be present when initial connection or service line extension is made to the Utility line, without exception.

AWWU will not approve any installation which is not in accordance with the Uniform Plumbing Code, these Specifications, and the AWWU Design Criteria. The Contractor shall not start the excavation for main line tap or on site service until a permit is obtained. All permits must be posted on the job at the time of the inspection.

**Article 10.4 Measurement**

Sanitary sewer service connections shall be measured as completed units in place. This item will include all materials, excavation, installation, compaction, backfill, and installation of bedding material.

Unless specifically identified for payment under a separate bid item, the unit price bid for Sanitary Sewer Service Connect (size) shall include all labor, equipment and materials to furnish and install a functional sanitary sewer service connection including but not limited to the following incidental items: location and verification of customers’ existing service locations, disconnection and reconnection of customer’s existing services where the Work includes replacement of existing services, clearing and grubbing; trench excavation and backfill; excavation dewatering; trench support system; furnishing and installing Type II-A Classified Fill and Backfill as bedding; compaction; installation of pipe, fittings, adapters, or other necessary appurtenances; sanitary sewer service insulation; polyethylene encasement; when applicable, connection to existing service at edge of right-of-way; disposal of unusable or surplus material; protection of existing utilities; restoration of existing drainage patterns; removal and replacement of existing culverts, fences, landscaping, and other public or private improvements; finish grading; and cleanup.

Where the Work includes disconnecting existing sanitary sewer services from an existing sewer main and reconnecting them to a new sewer main, the disconnection and reconnection of those existing sewer services will be considered incidental to the installation of the new sewer main.
**Article 10.5  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sanitary Sewer Service Connect (Size)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.11  REMOVE AND DISPOSE OF EXISTING CESSPOOLS OR SEPTIC TANKS AND CONNECT EXISTING SERVICE

Article 11.1  General

The Work under this Section consists of providing all operations pertaining to removal and disposing of existing cesspools or septic tanks and connection of existing service. If cesspools or septic tanks are encountered during construction, the Contractor shall either defer construction of the sewer main through the cesspools until such time as all downstream construction has been completed, tested and accepted or the Contractor may proceed with construction provided that the waste from the house service connection is accommodated continuously until satisfactory connection to the sanitary sewer main can be made. Such accommodations shall be in a manner approved by the Municipality of Anchorage Department of Health and Human Services.

Article 11.2  Construction

Where the Contractor must remove cesspools or septic tanks from the trench area, the following procedures shall apply:

1. The liquid and sludge from the existing structure shall be pumped into a watertight container, and transported to and disposed of at an approved sanitary sewer dump station to be designated by the Engineer. Care shall be exercised in transporting cesspool liquid and sludge so that no spillage occurs during transport and disposal.

2. The Contractor shall then remove the remaining sludge, septic tank, cesspool or privy pit, logs or cribbing, and any saturated gravel remaining in the trench area, and shall dispose of this material at a Contractor provided disposal area approved by the Municipality of Anchorage Department of Health and Human Services and the Engineer.

3. The Contractor shall then fill the void created by removal of the cesspool with Type III material in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

4. As soon as the downstream portion of the new sanitary sewer has been tested and accepted the Contractor shall replace the existing service to the property line and connect the existing house service to the main. Connection shall be made in a workmanlike manner and at a uniform grade to accommodate the existing service.

Article 11.3  Measurement

Removal of existing cesspool or septic tank, replacing the existing service to the property line and connecting existing house service to the new sanitary sewer is to be measured as two pay items as indicated in Article 11.4 – Basis of Payment. Disposal of logs, cribbing,
tanks and saturated gravel shall be measured as unsuitable material. Gravel necessary to fill the void after removal of structure shall be measured as Type III per Division 20, Section 20.21 – Classified Fill and Backfill.

**Article 11.4  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07, - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Cesspool or Septic Tank</td>
<td>Each</td>
</tr>
<tr>
<td>Connect Existing Service</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.12 CONSTRUCT SANITARY SEWER CLEANOUT

Article 12.1 General

The Work under this Section consists of providing all materials and operations pertaining to construction and installation of sanitary sewer cleanouts.

Article 12.2 Material

Material used in the construction of sanitary sewer cleanouts shall conform to the requirements of AWWA C-151, for Class 50 ductile iron pipe and AWWA C104/ANSI A21.4 fittings and as shown on the Standard Detail. Fittings to be restrained joint pipe and shall be EBAA Iron MEGALUG®, Romac Industries RomaGrip, U.S. Pipe Field LOK® Gasket, or approved equal.

Article 12.3 Construction

Excavation and backfill for the construction of sanitary sewer cleanouts shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

Over-excavation under cleanouts shall require thorough compaction prior to installation of the pipe and fittings.

The cleanout assembly shall be restrained throughout by use of EBAA Iron MEGALUG®, Romac Industries RomaGrip, U.S. Pipe Field LOK® Gasket, or approved equal, and shall be installed in accordance with Standard Detail 50-19.

Article 12.4 Measurement

Cleanouts will be measured as units, complete in place.

Article 12.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Sanitary Sewer Cleanout</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.13 POLYETHYLENE ENCASEMENT

Article 13.1 General

The Work under this Section consists of providing all labor, materials, and equipment to furnish and install one layer of polyethylene encasement on all ductile and cast iron pipe and fittings.

Polyethylene encasement shall be furnished on all ductile iron and cast iron water mains, sanitary sewer mains, sanitary sewer laterals, sanitary sewer connections, water services, valve boxes, and sanitary sewer cleanouts.

Article 13.2 Material

The polyethylene encasement material for pipe shall conform to the most current edition of AWWA C105/ANSI A21.5.

Article 13.3 Construction


Bedding and backfill material around pipelines with polyethylene encasement shall be placed using protective measures such as shields, guards, coating systems, and/or other methods as needed to protect the polyethylene encasement from becoming torn, punctured or otherwise damaged during the Work. Damage to the integrity of the polyethylene encasement shall be either repaired or the pipeline removed and the polyethylene encasement replaced as directed by the Engineer. Costs for repair and/or replacement of damaged polyethylene encasement shall be considered incidental to the installation of the polyethylene encasement and/or the installation of the pipeline protected by the encasement.

Article 13.4 Measurement

Measurement on all sizes of polyethylene encasement for pipe shall be the same as the measurement of the pipe installed.
Article 13.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Polyethylene Encasement</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>

SECTION 50.14  BYPASS PUMPING SANITARY SEWAGE FLOWS

Article 14.1  General

The Work under this Section consists of providing all planning, coordination, and operations pertaining to the bypass pumping of sewage flows around those portions of the sewage facilities to be rehabilitated. The existing flows include those from any upstream collection system components that contribute to the subject sanitary sewer mains or manhole facilities. Also, this Work shall include the installation and upgrade of portable sanitary sewer facilities (portable toilets) for use by the affected residents.

Article 14.2  Construction

The sewage flows shall be bypassed around sections of pipe designated for rehabilitation on an as-required basis. The Contractor shall ensure the pumps and bypass lines are of appropriate capacity and size to accommodate the anticipated sewage flows during the duration of all operations requiring such bypass.

The Contractor shall notify the occupants of any affected structure served by AWWU sanitary sewer service, in writing, at least 48-hours in advance, of any scheduled sanitary sewer service interruption. Services affected shall not be interrupted more than eight (8) consecutive hours or more than once in a twenty-four (24) hour period. Under no circumstances shall the Contractor allow the discharge of sewage into the existing storm drain system or onto the ground. No excavation will be permitted to facilitate this Work, except as indicated on the Drawings.

Prior to construction, the Contractor shall submit to the Engineer a plan detailing the scheduled deployment of pumps, hoses and other equipment necessary to maintain sewage flows during construction. The pumping system shall be such that the hydraulic gradient both upstream and downstream of the piping being bypassed will not reach elevations that will cause damage to the properties being served. This will require close attention to the elevation of the upstream head needed to actuate the pumping cycle and the rate of discharge flow from the pumps. The Contractor shall be liable for all damages which result from sewage flows not properly maintained during the progress of the Work, including all damages to private property which occur as a direct or indirect result of inadequate control of the sewage flow while the sewage bypass operation is ongoing. The Contractor is reminded that after-hours pumping may require a permit to exceed the allowable noise levels. Should such permit not be available for certain locations, such lack of availability shall not cause for claim for additional compensation or time extension.

The installation and maintenance of portable sanitary sewer facilities (portable toilets) shall be one (1) unit per four (4) residences (housing units) and at least one (1) unit on each side of the street. These units shall include hand sanitation devices, shall be properly vented, stocked, cleaned as necessary, or as determined by the Engineer, and overall maintained in a clean working order.
Article 14.3 Measurement

Bypass pumping of existing sewage flows will be paid for on a lump sum basis.

Article 14.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Pumping and Bypassing Existing Sanitary Sewage Flows</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 50.15  ON-PROPERTY SANITARY SEWER SERVICE

Article 15.1  Description

The Work under this Section consists of furnishing all material, labor, and equipment necessary for connecting the new sanitary sewer service at property line to the existing sanitary sewer service on property at, or near, the existing sanitary sewer main located on private property.

The exact location, type, and size of existing service connections are unknown. All information provided in the Drawings and Specifications has been taken from maintenance records, record drawings, or field surveys, and represents AWWU’s best indication of the service’s location and size. The Contractor shall locate the existing service line prior to installing the new service on property.

The Contractor shall notify the homeowner forty-eight (48) hours in advance of actual construction of the Work.

Article 15.2  Material

All material shall be in accordance with Section 50.02 - Furnish and Install Pipe. The Contractor shall supply all necessary fittings, adapters and other appurtenances to make a complete working system.

Article 15.3  Construction

The Contractor shall perform required trench excavation and backfill and compact to specified density; provide Type II-A Classified Fill and Backfill as bedding; flush and test system; protect/restore existing utilities, driveways, trees, utility markers, survey monuments, fences, retaining walls, buildings, walkways, gardens, landscaping and other private improvements damaged by the Contractor; and provide general cleanup. Prior to beginning Work, the Contractor shall submit to the Engineer in writing for approval, the service line routing, method of construction and schedule for performing the Work. The Contractor shall use appropriately sized construction equipment to minimize the impact to on-lot improvements and vegetation.

All Work shall be done in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill, and this Division.

Article 15.4  Measurement

Locating, furnishing and installing on-property sanitary sewer service lines shall be measured as units, complete in place.

The unit price for furnishing and placing sanitary sewer service connections on private property shall constitute full compensation for all labor, material and equipment required to provide a complete functioning sanitary sewer service connection from the property line to
an acceptable connection point on the existing sanitary sewer service, as determined by the Engineer, and installation of appurtenances.

All excavation, pipe bedding material, pipe fittings, and appurtenances, insulation, backfill, backfill gravel, topsoil and seeding, resetting fences, reconstructing retaining walls, walkways and restoration of property shall be included in this bid item. Asphalt or concrete driveways (where required) shall be paid for under the appropriate bid items for this Work.

Any conflicts with the homeowner concerning the installation of the on-lot sanitary sewer service connection and restoration of the property after construction shall be resolved by the Contractor at no additional cost to the Owner.

**Article 15.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install (Size) On-Property Sanitary Sewer Service</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.16 MANHOLE REHABILITATION

Article 16.1 Description

The Work under this Section consists of rehabilitation of existing manholes or adjusting their grade. The Contractor is to provide all labor, materials, and supervision required to furnish and install new manhole components needed to rehabilitate existing manholes.

Rehabilitation of existing manholes can include the following items of Work:

- Removal and replacement of manhole covers and frames.
- Removal and replacement of grade rings.
- Removal and replacement of manhole ladder rungs.
- Removal and replacement of cone section.
- Removal and replacement of barrel ring/riser section.
- Removal and replacement of base section.
- Removal and replacement of entire manhole assembly.

The manhole components to be removed and replaced for a specific manhole are identified in the Drawings. The Contractor is to reuse those manhole components that are not to be replaced in assembly of the rehabilitated manhole.

Article 16.2 Material

The replacement component materials of construction to be deployed in the Work are to comply with the requirements of this Division and the Standard Details. The Contractor shall furnish new, unused materials for those manhole components identified in the Drawings to be replaced.

Article 16.3 Construction

A. Work Plan

The Contractor shall prepare and submit a Work Plan to the Engineer that identifies how the manhole rehabilitation effort will proceed without interruption of existing sanitary sewer service. The Plan shall also address maintenance of vehicular traffic and pedestrian traffic. Manhole rehabilitation efforts shall not proceed without the Engineer's approval.
B. Temporary Services

The Contractor shall maintain sanitary sewer service during the execution of the Work. Any sewage pumping, temporary bypass piping, and/or temporary sanitary sewer service required to complete the Work will be considered incidental to the manhole rehabilitation effort and will not be paid for separately.

C. Earthwork

The Contractor shall excavate around the manhole as needed to access the Work. All excavation, shoring, dewatering, backfill and compaction efforts required to access the Work shall be per Division 20 – Earthwork. All importation of fill and/or disposal of unsuitable material, excavation, and backfill efforts shall be considered incidental to the manhole rehabilitation effort and will not be paid for separately.

D. Restoration

Upon completion of the manhole assembly effort, the Contractor shall restore the existing grades and surrounding area to preconstruction conditions. Any pavement, sidewalk, curb and gutter, landscaping, and/or other improvements or natural features disturbed and/or damaged by the manhole rehabilitation effort shall be restored by the Contractor to preconstruction conditions. Restoration of these conditions shall be considered incidental to the manhole rehabilitation effort and will not be paid for separately.

E. Manhole Rehabilitation

The Contractor shall remove and replace those manhole components identified in the Drawings. The rehabilitated manhole shall be configured according to the requirements of this Division and the Standard Details.

The Contractor shall use care in protecting those component parts of the existing manhole that are to be reused in the rehabilitated manhole.

Where the Work requires disassembly and reuse of components that are assembled with grout and/or mastic/sealant/gasket materials, the Contractor shall completely remove these materials from the components and replace them with new materials approved by AWWU for manhole construction in the reassembly of the rehabilitated manhole.

Where the Work requires the removal and replacement of existing ladder rungs, all ladder rungs within the existing manhole shall be removed and replaced. The Contractor shall cut off existing rungs and grind smooth against the interior wall of manhole. New ladder rungs shall be installed per the Standard Details.

Where the Work requires the removal and replacement of manhole cone section, manhole barrel ring/riser sections, and/or removal and replacement of manhole base section, the completed rehabilitated manhole shall be tested for leakage prior
to backfilling. With the excavation still dewatered, the Contractor shall demonstrate the integrity of the completed rehabilitated manhole using the methods described in Section 50.02, Article 2.4 - Testing. The infrastructure to be tested shall include all components of the rehabilitated manhole and connecting pipes disturbed or otherwise altered in the execution of the Work. If the rehabilitated manhole does not pass the leakage test due to visible defects in the new components and/or materials furnished by the Contractor, the defects shall be corrected and the assembly retested as often as required to pass the leakage test. If the failure of the test is determined by the Engineer to be a result of defects in the manhole components reused in the Work, the Engineer may direct the Contractor to take additional corrective measures on a time and materials basis.

The Contractor shall dispose of manhole components not used in the manhole rehabilitation effort and all other unsuitable or waste materials created in the execution of the Work. Disposal of these components shall be considered incidental to the manhole rehabilitation effort.

Article 16.4 Measurement

Rehabilitated manholes shall be measured as units complete in place with the components identified in the Drawings replaced, tested and accepted by the Engineer.

All effort required to complete the Work, including development of a Work Plan acceptable to the Engineer, temporary bypass piping, temporary sanitary sewer service, excavation, shoring, dewatering, backfilling, integrity testing, restoration of Work area to existing preconstruction conditions, and/or other items of Work needed to complete the manhole rehabilitation effort shall be considered incidental to the completion of the Work and shall not be paid separately.
Article 16.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove and Replace Manhole Cover and Frame</td>
<td>Each</td>
</tr>
<tr>
<td>(Manhole #)</td>
<td></td>
</tr>
<tr>
<td>Remove and Replace Manhole Grade Rings</td>
<td>Each</td>
</tr>
<tr>
<td>(Manhole #, Number &amp; Height of Grade Rings)</td>
<td></td>
</tr>
<tr>
<td>Remove and Replace Manhole Ladder Rungs</td>
<td>Each</td>
</tr>
<tr>
<td>(Identify Manhole No., Number of Ladder Rungs)</td>
<td></td>
</tr>
<tr>
<td>Remove and Replace Manhole Cone Section</td>
<td>Each</td>
</tr>
<tr>
<td>(Manhole #, Type)</td>
<td></td>
</tr>
<tr>
<td>Remove and Replace Manhole Barrel Ring/Riser Section</td>
<td>Each</td>
</tr>
<tr>
<td>(Manhole #, Type, Number of Rings Replaced, Depth Below Grade measured to bottom of lowest ring to be replaced)</td>
<td></td>
</tr>
<tr>
<td>Remove and Replace Manhole Base Section</td>
<td>Each</td>
</tr>
<tr>
<td>(Manhole #, Type, Depth of Base Section Below Grade measured to the bottom of base section)</td>
<td></td>
</tr>
</tbody>
</table>
SECTION 50.17  RAISE OR LOWER SEWER SERVICE

Article 17.1 General

The Work under this Section consists of all operations pertaining to raising or lowering of existing sanitary sewer services when the grade(s) of such services interfere with a utility under construction. Every effort has been made in the preparation of the Drawings to avoid conflict in grades with existing sewers; however, there may be some locations where conflict occurs.

Article 17.2 Construction

Where a conflict in grade occurs, the Contractor shall be required to excavate the sewer service from the point of interception sufficient distance to raise or lower the sewer service such that the grade conflict will be eliminated. Minimum grade of the sewer service shall be maintained in accordance with Section 50.10, Article 10.3 - Construction. In no case will the length of raising or lowering of the sanitary sewer service exceed fifty feet (50'). All excavation, backfill, and pipe laying shall be performed in accordance with the provisions of this Division and Division 20 - Earthwork.

Article 17.3 Measurement

Raising or lowering sewer services will be measured as units, complete in place.

Article 17.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section unless otherwise noted.

Any materials needed to complete the raising or lowering of a sewer service shall be provided by the Contractor and considered incidental to the price bid for this item. Compaction, where required, will also be considered incidental to the Contract.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Raise or Lower Sewer Service</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.18 ADJUST SANITARY SEWER MANHOLE CONE TO FINISH GRADE

Article 18.1 General

The Work under this Section consists of providing all operations pertaining to the adjustment of existing manhole cones to finish grade. All broken and/or missing manhole components are to be replaced with new materials furnished and installed by the Contractor in accordance with these Specifications.

Article 18.2 Material

All materials used in the adjustment of manhole cones including mortar, steps, barrel sections, premolded plastic gaskets, etc., shall conform to the requirements for manholes as outlined in Section 50.03 - Sanitary Sewer Manholes. Radial concrete manhole blocks may be used for upward adjustments in certain cases if approved by the Engineer.

Article 18.3 Construction

The Contractor shall remove the existing cone and add to or remove portions of the barrel of each manhole requiring a cone adjustment. Each precast concrete barrel and cone section shall be set upon and sealed with a premolded plastic gasket which shall meet AASHTO M-198, ASTM C990, or Federal Specification SS-SS-210. Any damage to manholes resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor’s expense. All inverts, benchwalls, and/or catch areas shall be left clean and free from any foreign materials.

Contractor shall adjust the manhole cone to finish grade prior to placement of pavement. Cutting of new asphalt for adjustments is not acceptable. Any adjustment(s) requiring cutting of new asphalt shall not be paid and shall be deducted from the plan quantity.

Article 18.4 Measurement

Manhole cone adjustments shall be measured as units, complete in place.
Article 18.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for cone adjustments shall include compensation for changes in height per the applicable Standard Details, unless otherwise directed by the Engineer. In no case will payment for both ring and cone adjustments be made for the same manhole.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Sanitary Sewer Manhole Cone</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.19   ADJUST SANITARY SEWER MANHOLE RING TO FINISH GRADE

Article 19.1   General

The Work under this Section consists of providing all operations pertaining to the adjustment of existing manhole rings to finish grade. All broken and/or missing manhole components are to be replaced with new materials furnished and installed by the Contractor in accordance with these Specifications.

Article 19.2   Material

All materials used in the adjustment of manhole rings shall conform to the requirements for manholes as outlined in Section 50.03 - Sanitary Sewer Manholes.

The Contractor may utilize Neenah R-1979 Series Manhole Adjusting Rings, or an approved equal, for adjusting the manhole to finished grade.

Article 19.3   Construction

The Contractor shall adjust the manhole rings in accordance with the applicable Standard Details. The Contractor shall set the adjusting rings in a bed of premolded plastic gasket material that meets AASHTO M-198, ASTM C990, or Federal Specification SS-S-210. The casting can be set in a bed of mortar with steel adjusting shims in the event the grade will not allow the premolded plastic gasket material. The steel shims shall be placed in four locations as a minimum and must be approved by the Engineer. Any damage to manholes resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor's expense.

Milling is an approved method of lowering the manhole grade. A horizontal milling process would as the casting is milled to lower the top to meet the finish grade of the street. This method must be submitted to the Engineer for approval.

Contractor shall adjust the manhole cone to finish grade prior to placement of pavement. Cutting of new asphalt for adjustments is not acceptable. Any adjustment(s) requiring cutting of new asphalt shall not be paid and shall be deducted from the plan quantity.

Article 19.4   Measurement

Manhole ring adjustments shall be measured as units, complete in place.
Article 19.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 – Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for ring adjustment shall include full compensation for changes in height. In no case will payment for both ring and cone adjustments be made for the same manhole.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Sanitary Sewer Manhole Ring</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.20 REMOVE EXISTING SANITARY SEWER MANHOLE

Article 20.1 General

The Work under this Section consists of providing all operations pertaining to the removal and disposal or salvage of existing manholes.

Article 20.2 Construction

Materials that are to be salvaged shall be removed in a workmanlike manner and delivered to a site as directed by the Engineer. A disposal site for non-salvageable materials shall be provided by the Contractor.

Any excavation required in the removal shall be considered incidental to this item. The Contractor shall backfill the excavation with a suitable, non-frost susceptible material and compact it to not less than ninety-five percent (95%) of maximum density as directed by the Engineer. If additional material is required for backfill, it will be paid for under the Item "Furnish Trench Backfill." Existing pipes shall be suitably plugged and abandoned unless otherwise noted.

Article 20.3 Measurement

Removal of existing sanitary manholes will be measured as units.

Article 20.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
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</thead>
<tbody>
<tr>
<td>Remove Existing Sanitary Sewer Manhole</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 50.21 ADJUST CLEANOUT TO FINISH GRADE

Article 21.1 General

The Work under this Section consists of providing all operations pertaining to adjustment of existing cleanouts to finish grade. All broken and/or missing cleanout components are to be replaced with new materials furnished and installed by the Contractor in accordance with these Specifications.

Article 21.2 Material

All materials used in the adjustment of cleanouts shall conform to the requirements for cleanouts as outlined in Section 50.12 - Construct Sanitary Sewer Cleanout.

Article 21.3 Construction

The Contractor may be required to adjust more than one type of cleanout under this Contract. All adjustments will be accomplished as directed by the Engineer. Any damage to cleanouts resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor's expense.

Article 21.4 Measurement

Cleanout adjustments will be measured per unit, complete in place.

Article 21.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
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</thead>
<tbody>
<tr>
<td>Adjust Cleanout to Finish Grade</td>
<td>Each</td>
</tr>
</tbody>
</table>
50-1 Sanitary Manhole - Type A - Pipe 8" to 24"
50-2 Sanitary Manhole - Type B - Pipe Dia 30" to 36"
50-3 Sanitary Manhole - Type C - Pipe Dia 40" to 48"
50-4 Type A and B Manhole Base Plan
50-5 Manhole Heights
50-6 Manhole Step
50-7 Sewer Service Connect for On-Site Lift Station 1-1/2" and 2"
50-8 Manhole Cover
50-9 Manhole Frame
50-10 Watertight Manhole Ring
50-11 Manhole Drop Connection
50-12 Typical Beaver Slide Manhole
50-13 Sanitary Sewer Service (Complete)
50-14 Lateral Connection to Concrete Pipe
50-15 Service Riser/Top Entry (for construction of new deep sewer - ductile iron)
50-16 Sanitary Sewer Service Connection (R.O.W. Only)
50-17 Sanitary Sewer Cleanout Cover
50-18 Sanitary Cleanout
50-19 Special Manhole and Cleanout Detail (inside protective well radius)
50-20 Special Manhole and Cleanout Detail (inside protective well radius)
50-21 Special Manhole and Cleanout Detail (inside protective well radius)
50-22 Horse Shoe Sanitary Sewer Manhole Detail
50-23 Contractor Field Installation Notes Sanitary Example
50-24 Manhole Cone Adjustment
50-25 Manhole Ring Adjustment
NOTES:

1. STEEL REQ'D FOR BARREL SHALL CONFORM TO ASTM C-478. IMBED STEEL IN BASE SO THAT FIRST BARREL SECTION IS CONNECTED WITH BASE.

2. ALL MANHOLE SECTIONS SHALL CONFORM TO ASTM C-478.

3. PROVIDE Z–LOK BOOTS FOR 8” TO 18” PIPE PENETRATIONS. PROVIDE A–LOK BOOTS FOR 20” THROUGH 24” PIPE PENETRATIONS. GROUT PER MANUFACTURER’S RECOMMENDATIONS.

4. COAT ALL EXTERNAL CONCRETE SURFACES OF MANHOLE WITH WATERPROOF BITUMINOUS COATING.

5. "RAM–NEK” OR EQUAL AND PRIME BARREL JOINTS. HEAT "RAM–NEK” AND SEAL SURFACES BEFORE FINAL ASSEMBLY.

6. SEAL MANHOLE JOINTS WITH "MAC WRAP” EXTERIOR PIPE JOINT SEALER OR APPROVED EQUAL, AFTER MANHOLE HAS BEEN WATERPROOFED (TYP ALL).

7. INSTALL INFI–SHIELD EXTERNAL UNI–BAND SEAL OVER FRAME, GRADE RINGS, AND TOP OF CONE.

8. WRAP EXTERIOR OF MANHOLE W/THREE LAYERS OF 8–MIL THICK POLYETHYLENE ENCASEMENT MATERIAL AFTER INSTALLING "MAC WRAP” AND INFI–SHIELD.

9. MANHOLE SHALL HAVE MINIMUM OF ONE (1) SIX–INCH (6”) GRADE RING.

10. BACKFILL AROUND MANHOLE WITH NFS MATERIAL (3–FEET MINIMUM). BACKFILL SHALL BE INCIDENTAL TO COST OF MANHOLE INSTALLATION.

11. FOUNDATION MATERIAL AS DIRECTED BY ENGINEER.
NOTES:

1. STEEL REQ'D FOR BARREL SHALL CONFORM TO ASTM C-478. IMBED STEEL IN BASE SO THAT FIRST BARREL SECTION IS CONNECTED WITH BASE.

2. ALL MANHOLE SECTIONS SHALL CONFORM TO ASTM C-478.

3. PROVIDE A-LOK BOOTS FOR 30" THROUGH 36" PIPE PENETRATIONS. GROUT PER MANUFACTURER’S RECOMMENDATIONS.

4. COAT ALL EXTERNAL CONCRETE SURFACES OF MANHOLE WITH WATERPROOF BITUMINOUS COATING.

5. "RAM-NEK" OR EQUAL AND PRIME BARREL JOINTS. HEAT "RAM-NEK" AND SEAL SURFACES BEFORE FINAL ASSEMBLY.

6. SEAL MANHOLE JOINTS WITH "MAC WRAP" EXTERIOR PIPE JOINT SEALER OR APPROVED EQUAL, AFTER MANHOLE HAS BEEN WATERPROOFED (TYP ALL).

7. INSTALL INFI-SHIELD EXTERNAL UNI-BAND SEAL OVER FRAME, GRADE RINGS, AND TOP OF CONE.

8. WRAP EXTERIOR OF MANHOLE W/THREE LAYERS OF 8-MIL THICK POLYETHYLENE ENCASEMMENT MATERIAL AFTER INSTALLING "MAC WRAP" AND INFI-SHIELD.

9. MANHOLE SHALL HAVE MINIMUM OF ONE (1) SIX-INCH (6") GRADE RING.

10. BACKFILL AROUND MANHOLE WITH NFS MATERIAL (3-FEET MINIMUM). BACKFILL SHALL BE INCIDENTAL TO COST OF MANHOLE INSTALLATION.

11. FOUNDATION MATERIAL AS DIRECTED BY ENGINEER.
NOTES:
1. STEEL REQ'D FOR BARREL SHALL CONFORM TO ASTM C-478.
2. ALL MANHOLE SECTIONS SHALL CONFORM TO ASTM C-478.
3. PROVIDE A-LOK BOOTS FOR 40" THROUGH 48" PIPE PENETRATIONS. GROUT PER MANUFACTURER'S RECOMMENDATIONS.
4. COAT ALL EXTERNAL CONCRETE SURFACES OF MANHOLE WITH WATERPROOF BITUMINOUS COATING.
5. "RAM-NEK" OR EQUAL AND PRIME BARREL JOINTS. HEAT "RAM-NEK" AND SEAL SURFACES BEFORE FINAL ASSEMBLY.
6. SEAL MANHOLE JOINTS WITH "MAC WRAP" EXTERIOR PIPE JOINT SEALER OR APPROVED EQUAL, AFTER MANHOLE HAS BEEN WATERPROOFED (TYP ALL).
7. INSTALL INFI-SHIELD EXTERNAL UNI-BAND SEAL OVER FRAME, GRADE RINGS, AND TOP OF CONE.
8. WRAP EXTERIOR OF MANHOLE W/THREE LAYERS OF 8-MIL THICK POLYETHYLENE ENCASEMENT MATERIAL AFTER INSTALLING "MAC WRAP" AND INFI-SHIELD.
9. MANHOLE SHALL HAVE MINIMUM OF ONE (1) SIX-INCH (6") GRADE RING.
10. BACKFILL AROUND MANHOLE WITH NFS MATERIAL (3- FEET MINIMUM). BACKFILL SHALL BE INCIDENTAL TO COST OF MANHOLE INSTALLATION.
11. FOUNDATION MATERIAL AS DIRECTED BY ENGINEER.
<table>
<thead>
<tr>
<th>LOCATION</th>
<th>E</th>
<th>F</th>
</tr>
</thead>
<tbody>
<tr>
<td>BACKYARDS, GRAVEL STREETS, AND ALLEY AREAS WHERE TRAVELED.</td>
<td></td>
<td>4”–8”</td>
</tr>
<tr>
<td>UNDEVELOPED AND SWAMPY AREAS.</td>
<td></td>
<td>24” MIN</td>
</tr>
<tr>
<td>HIGHWAY R.O.W.S OUTSIDE TRAFFIC AREAS.</td>
<td></td>
<td>6”</td>
</tr>
<tr>
<td>PAVED STREETS.</td>
<td></td>
<td>1/2”</td>
</tr>
</tbody>
</table>
NOTE:

1. DRIVE RUNG INTO PREFORMED OR DRILLED HOLES WITH A 6 TO 10 LB. SLEDGE HAMMER, AFTER CONCRETE IS CURED TO 3000 PSI MIN.

2. THE INSTALLED STEP SHALL RESIST A PULLOUT FORCE OF 1500 LBS.
DETAIL "A"
(SEE NOTE 4)

NOTES:
1. ROMAC STYLE CB SEWER SADDLE SHALL BE USED ON ALL PIPE.
2. TYPE "K" COPPER PIPE SHALL BE USED FOR ALL FORCE MAIN PIPE INSTALLATIONS OF 10-FOOT DEPTH OR MORE. HDPE ARCTIC PIPE SHALL BE USED FOR ALL FORCE MAIN PIPE INSTALLATIONS OF DEPTH LESS THAN 10-FOOT (SEE DETAIL"A").
3. THE USE OF HDPE ARCTIC PIPE WITH ELECTRIC HEAT TRACE SHALL ONLY BE USED WITH PRIOR WRITTEN APPROVAL BY AWWU ENGINEERING. ENDS OF HEAT TRACE CHANNEL SHALL BE SEALED WATER TIGHT.
4. RIGID BOARD INSULATION SHALL BE HIGH DENSITY EXTRUDED POLYSTYRENE, MIN. 60 P.S.I., EQUIVALENT TO R-18 PER 4-INCH THICKNESS.
3/4" RD. HANDLE

4" x 4"

30 1/2"

27"

10"

7"

1" SLOT

GREASE/LUBRICATE

1/4" NEOPRENE GASKET

LOCKING MECHANISM DETAIL

3/4"

2"

3/4" DRILLED & TAPPED

FROG

WATERTIGHT MANHOLE RING
MANHOLE TYPE "A"

DUCTILE IRON 10" MIN.

2" - 4" PENETRATION

REstrained JOINTS

COMPACTED BACKFILL 95% MAX. DENSITY.

DROP PIPE TO BE SAME SIZE AS THE INTERCEPTED SEWER.

90° TEE

90° BEND

POUR CONCRETE TO COMPACTED FILL OR ORIGINAL GROUND.

MATCH INFLUENT INVERT WITH EFFLUENT SPRINGLINE

FOR 8" TO 18" PIPE PENETRATIONS.

PROVIDE Z-LOK BOOT FOR PENETRATION WITH 20"-48" RANGE PIPE.

PROVIDE A-LOK GASKETS. GROUT PER MANUFACTURER'S RECOMMENDATIONS.
WHEN INSTALLING A BEAVER SLIDE THAT INTERCEPTS AN EXISTING SEWER AT A RIGHT ANGLE, THE CONNECTING INVERT OF THE BEAVER SLIDE IS TO INTERCEPT THE EXISTING SEWER SLIGHTLY ABOVE THE SPRINGLINE AS SHOWN. DISTANCE MEASURED FROM INVERT TO INVERT.

WHEN INSTALLING A BEAVER SLIDE WHERE THE FLOW IS STRAIGHT THROUGH THE MANHOLE, THE BEAVER SLIDE IS TO MATCH THE INVERT OF THE EXISTING LINE AND NOT TO EXTEND MORE THAN HALF-WAY THROUGH THE MANHOLE. DISTANCE MEASURED FROM INVERT TO INVERT.

PREMIX MORTAR PROHIBITED
Use mortar to form a weathertight seal.

C.I. Connector shall not extend into the concrete pipe in interior.

Diameter of hole in concrete pipe shall not exceed diameter "D" of the C.I. Connection plus 2".

D.I.P. pipe shall be used between M.H. & trunkline. Use 22 1/2" or 11 1/2" bends.

The existing ground around the conc. trunk shall be disturbed as little as possible.

8" Type II-A compacted to min. 95% max. density. Compact existing ground to min. of 95% max. density.

LATERAL CONNECTION TO CONCRETE PIPE
NOTE:
ALL DUCTILE IRON PIPE
AND MECHANICAL CONNECTORS
SHALL HAVE RESTRAINED
JOINTS.
INSTALL 2"x4" WOOD POST 3' ABOVE FINISH GRADE.

SLOPE = 2% (4" SERVICE)  
1% (6" SERVICE)

22 1/2° OR 45° SWEEP

SADDLE

22 1/2° OR 45°

SPRING LINE

2" x 4" WOOD POST

TEST PLUG (AIRTIGHT)

EDGE OF R.O.W. OR PERMANENT SEWER OR UTILITY EASEMENT

NOTES:
1. ROMAC STYLE CB SEWER SADDLE.
2. PIPE SHALL BE CLEANED PRIOR TO SADDLE INSTALLATION.
3. 4" OR 6" SCH 50 DIP

4" OR 6" SADDLE

LATERAL MAIN
NOTES:
1. CAST CLEANOUT FRAME AND COVER, EJW 3668 (OR EQUAL)
2. CASTING THICKNESS SUBJECT TO FOUNDRY REQUIREMENTS.
3. CASTING MUST BE Sized TO FIT 8-INCH, D.I.P. CLASS 50 ONLY.
4. COVER SHALL BE 1/2" BELOW TOP OF PAVEMENT OR 6" BURY BELOW GRAVEL, DIRT OR R.A.P ROAD.

1/2" SET SCREW 3 REQUIRED @ 120°
8" D.I.P.
SANITARY CLEANOUT COVER
(standard detail 50-18)

8" DUCTILE IRON
PIPE, CLASS AS
SPECIFIED ON THE
DRAWINGS

CLEANOUT STATION
AS SHOWN IN THE DRAWINGS

8" DUCTILE IRON
PIPE, CLASS AS
SPECIFIED ON THE
DRAWINGS

8' (MIN.)

8" D.I.P. 45° BEND W/ MEGALUG® RESTRAINED FITTING OR EQUAL (TYP. EACH SIDE)

VARIES LENGTH, USE FIELD LOK®

IF MORE THAN ONE PIPE LENGTH, USE FIELD LOK®

GASKET OR EQUAL

UNDISTURBED GROUND OR BACKFILL TO BE COMPACTED TO 95% MAX. DENSITY
1. TYPE "B" MANHOLE (MINIMUM)
2. FITTINGS SHALL BE "SHORT BODY" AND RESTRAINED W/ MEGALUG® OR EQUAL.
3. INSTALL MASON SAND TO SPRING LINE (SEE DETAIL 50–21 OR 50–22).
4. MANHOLE LID & FRAME SHALL BE WATERTIGHT IN ACCORDANCE WITH DETAIL 50–10.
NOTES:
1. TYPE "B" MANHOLE (MINIMUM).
2. FITTINGS SHALL BE "SHORT BODY" AND RESTRAINED W/ MEGALUG® OR EQUAL.
3. INSTALL MASON SAND TO SPRING LINE OF PIPE.
4. MANHOLE LID & FRAME SHALL BE WATERTIGHT IN ACCORDANCE WITH DETAIL 50–10.
NOTES:
1. TYPE "B" MANHOLE (MINIMUM).
2. FITTINGS SHALL BE "SHORT BODY" AND RESTRAINED W/ MEGALUG® OR EQUAL.
3. INSTALL MASON SAND TO SPRING LINE OF PIPE.
4. MANHOLE LID AND FRAME SHALL BE WATERTIGHT IN ACCORDANCE WITH DETAIL 50–10.
2 - #4 REBAR SPACED 2" APART ALL AROUND 2" & 4" FROM TOP.

INSTALL 3 EQUALLY SPACED (120° APART) LIFTING RINGS.

#4 REBAR AT 6" INTERVALS BOTH WAYS, EXTENDING FULL LENGTH OF BARRELL.

BARREL
TONGUE & GROOVE (GASKET CONSTR.)

PRECAST CONCRETE RING

PIPE O.D. +4"

2"-4"

2"-4"

6" SPRING LINE

3"

STD. TONGUE & GROOVE DETAIL A W/ RAM-NEK GASKET

NOTE: 1. NO REBAR TO EXTEND INTO PIPE OPENING.
2. MORTAR PENETRATIONS WITH JET SET OR EQUAL

HORSE SHOE SANITARY SEWER MANHOLE DETAIL
### CONTRACTOR FIELD INSTALLATION NOTES

**SANITARY EXAMPLE**

<table>
<thead>
<tr>
<th>STA</th>
<th>H.I.</th>
<th>T.B.M. EL.</th>
<th>INV. EL.</th>
<th>ROD INV.</th>
<th>ROD T.O.P.</th>
<th>FOOTAGE</th>
<th>TIME/SET</th>
<th>SLOPE/DESCR.</th>
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<td>74.70</td>
<td>69.06</td>
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</tbody>
</table>

**USE INVERT ELEVATIONS (INV) FOR SEWER**
**USE BOTTOM OF PIPE ELEVATIONS (BOP) FOR WATER**

**MUNICIPALITY OF ANCHORAGE**

**SCALE:** NTS
**APPROVED:**
**REVISED:** 10/08

**SECTION #** DIV 50
**DETAIL #** 50-23
NOTES:
1. RESET CONCRETE GRADE RING IN FULL BED OF MORTAR.
2. REFER TO ASTM DESIGNATION C-478 FOR DESIGN AND STRENGTH REQUIREMENTS.
3. RESET CONE IN RAM–NEK OR EQUAL.
4. ADJUST FRAME TO A DEPTH OF 1/2” BELOW SURFACE OF PAVEMENT. FEATHER EDGE OF PAVEMENT TO SMOOTH TRANSITION.
NOTES:
1. REFER TO ASTM DESIGNATION C-478 FOR DESIGN AND STRENGTH REQUIREMENTS.
2. WHEN AN ADJUSTMENT OF GREATER THAN 18" IN GRADE RINGS IS REQUIRED, ADJUST CONE I.A.W. STANDARD DETAIL 50-25 RATHER THAN GRADE RINGS.
3. IF NECESSARY, SHIM MANHOLE FRAME WITH STUD WASHERS, TO ADJUST FRAME TO A DEPTH OF 1/2" BELOW SURFACE OF PAVEMENT. FEATHER EDGE OF PAVEMENT TO SMOOTH TRANSITION. WHEN SHIMS ARE USED, SET MANHOLE FRAME IN A FULL BED OF MORTAR WITH SHIMS.
### STANDARD CONSTRUCTION SPECIFICATIONS
**FOR STORM DRAIN SYSTEMS**

**DIVISION 55**

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STANDARD CONSTRUCTION SPECIFICATIONS
FOR STORM DRAIN SYSTEMS
DIVISION 55

SECTION 55.01 GENERAL

Article 1.1 Scope of Work

The Work covered by these Specifications consists of providing all plant, labor, equipment, supplies, transportation, handling, storage and performance of all operations necessary to complete the construction for the pipe laying, jointing, and testing of storm drain systems and culverts.

Requirements for earthwork including trench excavation and backfill are specified in Division 20 - Earthwork.

Article 1.2 Applicable Standards

The latest revision of the following standards of the American Society for Testing and Materials (ASTM), the American Association for State Highway and Transportation Officials (AASHTO), and the American Water Works Association (AWWA) are hereby made part of this Specification.

ASTM A-48 Standard Specifications for Gray Iron Castings
ASTM C-76 Specification for Reinforced Concrete
ASTM C-150 Specification for Portland Cement
ASTM C-478 (AASHTO-199) Specification for Precast Reinforced Concrete Manhole Sections
ASTM C-990 Standard Specification for Joins for Concrete Pipe, Manholes, and Precast Box Sections Using Preformed Flexible Joint Sealants
ASTM D 1248 Polyethylene Plastics Molding and Extrusion Materials, Type III, High Density
ASTM D 3035 Polyethylene Plastic Pipe (SDR-PR) Based on Controlled Outside Diameter
ASTM D 3350 Polyethylene Plastics and Fittings Materials
AASHTO M-36 Corrugated Steel Pipe & Fittings
AASHTO M-45 Sand for Cement Mortar
AASHTO M-105 Gray Iron Castings
AASHTO M-190 Bituminous Coating of CMP
AASHTO M-196 Corrugated Aluminum Pipe & Fittings
AASHTO M-198  Joints for Concrete Pipe, Manholes, and Precast Box Sections Using Preformed Flexible Joint Sealants
AASHTO M-245  Precoated Galvanized Steel Culverts and Underdrains
AASHTO M-246  Precoated Galvanized Steel Sheets for Culverts and Underdrains
AASHTO M-274  Corrugated Aluminized Pipe and Fittings
AASHTO M-252  Corrugated Polyethylene Tubing 3"-10" diameter
AASHTO M-294  Corrugated Polyethylene Pipe, 12" diameter and larger
AASHTO M-306  Drainage, Sewer, Utility, and Related Castings
Federal Specification SS-S-210  Sealing Compound, Preformed Plastic, for Expansion Joints and Pipe Joints

**Article 1.3 Surveys**

The Contractor shall layout in the field the alignment and grade of Work to be done under the Contract. The Contractor shall be responsible for the preservation of all line stakes, grade stakes, and hubs. In the event of their loss or destruction, the Contractor shall be responsible for their proper replacement. The line and grade for pipe lines shall be given from reference hubs offset from each manhole or cleanout. The Contractor shall be responsible for the transfer of the control points from the reference hubs to such hubs or batter boards as he may desire or need for the prosecution of the Work.

A Professional Land Surveyor licensed in the State of Alaska, subcontracted to the Contractor, shall perform all surveying, project control, monumentation, staking, profiles, and cross section measurements for pay item quantities. All personnel involved in measuring and recording survey data shall be directly employed by the Surveying Subcontractor and shall not be employed by the Contractor or any of the other Subcontractors for the duration of the Project. Failure to adhere to this requirement will result in non-payment for all Work affected by non-compliance. All survey work will adhere to Division 65 – Construction Survey.

**Article 1.4 Concrete and Mortar**

A. **Miscellaneous Concrete**

All concrete used in the construction of storm drains with the exception of precast manholes, manhole risers, cones, and catch basin barrels shall be Class A-3. Concrete Work shall conform to Division 30 – Portland Cement Concrete.

B. **Mortar**

Cement for mortar used in the construction of storm drain shall conform with the requirements of ASTM C-150, Type II. Sand shall conform with the requirements of AASHTO M-45. The mortar shall be composed of one (1) part cement and three (3) parts sand. The addition of lime is not permitted.
Article 1.5 Payment - General

Payment for all Work included in this Division shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described.
SECTION 55.02  FURNISH AND INSTALL PIPE

Article 2.1  General

The Work under this Section consists of the performance of all operations pertaining to furnishing and installing pipe for storm drain systems.

In the case of Owner-furnished pipe, the Owner shall allot to the Project pipe to accomplish the Work in amounts, exactly matching the Contractor's pay quantities for pipe. Any surplus pipe left over from this allotment at the end of the Project shall be returned from the Contractor's job sites to the Owner's designated pipe yard. If the Contractor withdraws from the Owner's pipe yard more than the amount required to match the payment quantities, the Contractor shall pay the Owner on the basis of the Owner's invoice price for pipe (including freight), plus ten percent (10%) overhead to reimburse the Owner for handling, warehousing, inspection, and administration.

Article 2.2  Material

A. General

All piping shall be in accordance with the Contract Documents conforming to the size and class shown and specified. Changes in class shall be made within one-half of a pipe length of the station indicated on the Drawings.

B. Corrugated Metal Pipe (CMP)

Corrugated metal pipe shall only be used in culvert crossings and similar applications. CMP use in a piped storm drain system is prohibited without approval from Street Maintenance and the Municipal Engineer. Corrugated metal pipe is intended to refer to both steel and aluminum. The pipe shall conform to the following specifications:

1. **Steel:** Corrugated steel pipe shall meet the requirements of AASHTO M-36.
2. **Aluminum:** Corrugated aluminum pipe shall conform to the requirements of the AASHTO M-196.
3. **Aluminum Coated (Aluminized):** Corrugated aluminized pipe shall conform to the requirements of AASHTO M-36 and AASHTO M-274.

All CMP fittings shall be fabricated in a workmanlike manner, develop the full strength of the material being joined, and finished to conform to the appropriate requirements of AASHTO M-36, AASHTO M-196 and AASHTO M-274.

Corrugated steel and aluminum pipe shall jointed by using coupling bands applied as recommended by the manufacturer and approved by the Engineer.
Dissimilar metals may only be used in extending in place metal CMP and reattachment of dissimilar metal end sections provided an electrical insulating material, at least one-sixteenth inch (1/16") in thickness, is used to separate the dissimilar materials.

All angles, bolts, and nuts shall be as recommended by the manufacturer for the type of pipe used and as approved by the Engineer.

The metal gauge for pipe to be used shall be in accordance with the Contract Documents.

If bituminous coating of CMP is required, the bituminous coating shall conform to the requirements of AASHTO M-190.

All welding performed by the Contractor on aluminum pipe shall incorporate the use of 4043 or 5356 alloy for welding wire. The welding shall be accomplished by either the "TIG" (tungsten, inert gas shielded) or "MIG" (metal arc welding, inert gas shielded) process.

End Section for Corrugated Metal Pipe - Galvanized steel and aluminum end sections shall be flared, beveled, shop-assembled units to serve as structural, hydraulic and esthetic treatment to corrugated metal pipe culverts. They may be attached to culverts by threaded bolts, by riveting or bolting in accordance with the manufacturer's standard procedure. End sections shall have a turned-down lip or toe plate at the wide end to act as a cutoff. Materials for steel end sections shall be galvanized steel conforming to the requirements of AASHTO M-36. The gauge shall be as follows:

- 16 Ga. Through 24" in diameter or 29" X 18" pipe-arch
- 14 Ga. 30" in diameter and 36" X 22" pipe-arch
  36" in diameter and 43" X 27" pipe-arch
- 12 Ga. Over 36" in diameter and 43" X 27" pipe-arch
  (except that the center panels of 60" in diameter and larger and 72" x 44" pipe-arch and larger, shall be 10 Ga.)

Galvanized stiffener angles shall supplement the usual reinforced side edges for sixty inches (60") in diameter and larger, seventy-nine by forty-nine inch (79" x 49") pipe-arch and larger.

If the end section is shop attached to a stub of pipe, the pipe stub shall not be lighter in gauge than the end section.

Materials for aluminum end sections shall comply with the provisions of AASHTO M-196 and fabrication shall comply with the requirements above.
C. Precoated Corrugated Metal Pipe (PCMP)

All precoated corrugated metal pipe and connecting bands shall be coated to meet the AASHTO DESIGNATION: M-245 and M-246 and the coating shall be 10 mils minimum thickness each side. All exposed edges including any perforated hole edges shall be coated with a liquid coating supplied by the supplier of the precoated corrugated pipe. All metal utilized for the precoated metal pipe shall conform to SubArticle 2.2.B - Corrugated Metal Pipe. All metal pipe utilized shall have a nominal wall thickness of 16 gauge for pipes twenty-one inches (21") and larger and 18 gauge for pipes eighteen inches (18") and smaller, unless otherwise noted.

D. Corrugated Polyethylene Pipe (CPEP)

Corrugated Polyethylene pipe shall conform to the following specifications:

1. Three inch through ten inch (3" through 10") diameters: the requirements of AASHTO M-252.

2. Twelve inch (12") and larger diameters: the requirements of AASHTO M-294.

The corrugated Polyethylene Pipe covered by these specifications is classified as follows:

Type C - This pipe shall have a full circular cross-section with a corrugated surface both inside and outside. Corrugations may be either annular or helical.

Type S - This pipe shall have a full circular cross-section, with an outer corrugated pipe wall and a smooth inner liner. Corrugations may be either annular or helical.

Type CP - This pipe shall be Type C with perforations.

Type SP - This pipe shall be Type S with perforations.

All CPEP fittings shall be rotational or blow molded and shall conform to the fitting requirements of AASHTO M-252 or M-294.

Join three inch to ten inch (3" - 10") CPEP with couplings corrugated to match the pipe corrugations or with push-on couplings with locking devices.

Join twelve inch (12") and larger CPEP with couplings, corrugated to match the index in the pipe corrugations and in a width not less than three-quarters (3/4) of the nominal pipe diameter. All couplings shall be manufactured to lap equally to a distance on each jointed pipe, to no less than the diameter of the pipe and shall provide a positive means of closure.
CPEP may be connected to CMP or may be used between or connected to dissimilar metals.

All flared end sections and saddles shall be constructed of the same material as the pipe and shall be factory assembled units to serve as structural, hydraulic, and/or aesthetic end treatment to CPEP culverts. CPEP connections shall be as recommended by the manufacturer. The cost of the end section and saddles shall be incidental to the pipe.

E. High Density Polyethylene Pipe (HDPEP)

High density polyethylene pipe shall conform to the following specifications:

The polyethylene resin shall be classified by ASTM D-1248 as Type III, Class C, Category 5, Grade P34, and have a minimum ASTM D-3350 cell classification of 335434C and a designation of PE 3408 by the Plastic Pipe Institute.

The polyethylene compound shall be suitably protected against ultraviolet light degradation by means of a two percent (2%) concentration of carbon black, well dispersed by pre-compounding in with the resin (by the resin manufacturer).

The pipe shall contain no recycled compound except that generated in the manufacturer's own plant from resin of the same specification from the same raw material supplier. The pipe shall be homogeneous throughout and free of visible cracks, holes, foreign inclusions, or other deleterious defects, and shall be identical in color, density, melt index, and other physical properties.

The pipe shall be designed according to the ISO modified formula in ASTM D-3035. The design pressure rating shall be expressed in terms of the static working pressure in psi for water at 73.4°F according to ASTM D-2837. The minimum allowable pressure rating for gravity pipe shall be 52 psi.

Join pipe lengths to one another using thermal butt fusion. Butt fusion of pipes shall be performed in accordance with the pipe manufacturer's recommendations for equipment and technique, using the correct size equipment and technique. Butt fusion will be performed only by personnel certified as competent by the polyethylene material supplier.

The Contractor shall provide butt fusion equipment compatible with the piping system being used as necessary to complete all joints on the project. All costs in connection with this equipment shall be included in the price bid for pipe installation.

Provide wall pipes or wall fitting as recommended by the pipe manufacturer to connect storm drain and catch basin drain pipes to manholes and catch basins.

Installation of all components shall be accomplished using the manufacturer's recommendations. Unless the Contractor's personnel are certified in the installation of polyethylene pipe, the pipe suppliers shall provide pipe personnel to instruct the
Contractor in the handling, installation, and testing of their products. The Contractor shall provide one supplier's representative at the start of construction for on-site services. Additional technical representative services, if necessary, shall also be at the Contractor's expense.

Random tests of field joints will be made by the Engineer, as necessary, as a quality control measure. The Contractor shall be responsible for removal or repair of unsatisfactory butt fusion joints.

**Article 2.3 Construction**

**A. Excavation and Backfill**

Excavation and backfill for furnishing and installing pipe shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

**B. Pipe Grade and Alignment**

Variance of individual pipe sections from established line and grade shall not be greater than those listed in the table below, providing that such variance does not result in a level or reverse sloping invert.

<table>
<thead>
<tr>
<th>Diameter (Inches)</th>
<th>Tolerance (Feet)</th>
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<tr>
<td>8</td>
<td>0.03</td>
</tr>
<tr>
<td>10</td>
<td>0.03</td>
</tr>
<tr>
<td>12</td>
<td>0.03</td>
</tr>
<tr>
<td>14</td>
<td>0.04</td>
</tr>
<tr>
<td>16</td>
<td>0.04</td>
</tr>
<tr>
<td>18*</td>
<td>0.05</td>
</tr>
</tbody>
</table>

*Note: For all pipe sizes over eighteen inches (18") in diameter, tolerance not to exceed five-hundredths feet (0.05').

During the progress of the Work, the Contractor shall provide instruments such as transits, levels, laser devices, and other facilities for transferring grades from offset hubs or for setting of batter boards or other construction guides from the control points and bench marks provided by the Contractor. The Contractor shall provide qualified personnel to use such instruments and who shall have the duty and responsibility for placing and maintaining such construction guides. The Contractor shall notify the Engineer forty-eight (48) hours prior to taking measurements on newly installed section of line and/or appurtenances for Record Documents.

If the method of transferring grades from the offset hubs to the pipe require batter-boards, they shall be at least one by six inches (1" x 6") supported on two by
four inch (2" x 4") stakes or approved metal rods and shall be placed every twenty-five feet (25’). At least three boards must be in place at any given time to facilitate checking of line and grade. Both line and grade shall be checked for each piece of pipe laid, except at tunnels where methods acceptable to the Engineer shall be used to carry forward line and grade.

The practice of pushing in uncompacted backfill over a section of pipe to provide a platform for transit and level alignment and grade observations shall be subject to the approval of the Engineer. If intermittent backfilling is allowed backfilling shall be accomplished in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

Due to the flexibility of the CPEP, the Contractor shall exert due care while placing bedding and/or filter material and compacting adjacent to and over the pipe. All placement bedding and/or filter material and compaction shall be per the manufacturer's recommendations or as approved by the Engineer.

The Contractor shall exert due care in handling the precoated corrugated metal pipe or while placing bedding and/or filter material around the pipe so as not to damage the coating. The Contractor shall obtain a liquid coating supplied by the precoated corrugated metal supplier which will be painted over scratched or cut sections of the pipe.

C. Pipe Laying

All pipe shall be laid with Class C Bedding unless otherwise required by the Contract Documents or directed by the Engineer.

Pipe laying shall in all cases proceed upgrade. Each pipe shall be laid true to line and grade and in such a manner as to form a close concentric joint with the adjoining pipe. The alignment of the installed pipe shall appear straight to visual observations and shall be such that a full circle of light can be seen between manholes, etc., when sighting along all points of the pipe circumference. Each section of pipe shall be handled carefully and placed accurately. Each section of pipe shall be properly supported to ensure true alignment and an invert which is smooth and free from roughness or irregularity. On helical pipe, the laps shall not impede the flow and all seams shall be aligned uniformly for the length of the run. At all times, when Work is not in progress, open ends of pipe and fittings shall be securely and satisfactorily closed so that no undesirable substances shall enter the pipe or fittings. All pipe shall be laid in accordance with the respective manufacturer's recommendations. Pipe shall not be laid when the bottom of the ditch or the sides to one foot (1’) above the pipe are frozen. Backfill containing frozen material shall not be placed, nor shall the trench be left open during freezing weather so that the temperature of the material near the pipe goes below freezing.
D. Televising Storm Drains.

New storm drains twelve inch (12") in diameter to thirty-six inch (36") in diameter shall be inspected by closed circuit television (CCTV) after completion of trench backfill and finished grading but prior to the placement of pavement or permanent trench resurfacing, to determine the existence and extent of any obstructions, structural deficiencies, or sags. Storm drains less than fifty feet (50’) in length for a single run are not required to be televised.

The Contractor shall do the televising. The Engineer reserves the right to retelevising any new storm drain work after the placement of pavement or permanent trench resurfacing, but before acceptance by the Engineer, to determine the existence and extent of any foreign material or obstructions such as, but not necessarily limited to, cement grout, wood, rocks, sand, concrete, or pieces of pipe, and any structural deficiencies or sags precipitated by the permanent resurfacing operations or other Contract Work. The Contractor shall notify the Engineer two (2) working days in advance of the anticipated date of the televising.

Five (5) working days shall be allowed for the Engineer to review each individual video recording of each and every storm drain documented on that particular recording. In the event that any deficiencies or sags are discovered by the Engineer, either by the Contractor's televising or the Engineer's retelevising, three (3) working days shall be allowed for the Engineer to determine whether the deficiencies or sags are repairable in place. If the Engineer determines that the deficiencies or sags are not repairable in place, the affected portion(s) shall be reconstructed in accordance with these Specifications.

The Contractor shall not be entitled to any additional working days due to delays resulting from the correction of any deficiencies or sags, either repairable or non-repairable in place, as determined by televised inspections and the Engineer.

1. General Requirements
   a. The video operator must have at least one (1) year of experience with a project of a similar nature within the past two (2) years.
   b. Video recordings shall be high quality color on high quality DVD/CD disk format (VHS format maybe considered, must be approved by the Engineer prior to use). Any out-of-focus video recordings or portions thereof, shall be cause for rejection of the video recordings and will necessitate retelevising at the Contractor’s expense.
   c. The Contractor shall notify the Engineer two Municipal working days prior to televising.
   d. The Contractor shall turn over the original video recordings to the Engineer immediately after recording.
e. Televising shall be done in one direction for the entire length between manholes; each section shall be isolated from the remainder of the storm drain as required. Sufficient water shall be supplied to cause drainage within the isolated section prior to televising.

f. For underground storm drain conduit installations, the maximum operation tolerance for a sag shall be one-hundredth foot (0.01') per inch of pipe diameter. No sag shall be longer than sixty feet (60'). When televised inspection is used to check for sag, a calibrated device acceptable to the Engineer shall be used to measure the depth of sag.

g. The Contractor shall not be entitled to any additional working days due to delays in securing the videotaping services of a private vendor.

2. Equipment for Televising

Television equipment shall include the television camera, television monitor, cables, power source, lights and other equipment necessary to the televising operation. The camera shall be specifically designed and constructed for operation in connection with storm drain inspection. The camera shall be self-operative in one hundred percent (100%) humidity conditions. Focal distance shall be adjustable through a range of from one inch (1") to infinity. The camera shall be self propelled or mounted on skids suitably sized for each pipe diameter to be investigated. Lighting for the camera shall minimize reflective glare. Camera and lighting quality shall be suitable to provide a clear, continuously in-focus picture of the entire inside periphery of the storm drain for all conditions encountered during the Work. The remote reading footage counter shall be accurate to within one-half percent (0.5%) over measured distance of the particular section being inspected and shall be displayed on the television monitor. The camera, television monitor and other components of the video system shall be capable of producing a minimum three hundred and fifty (350) line resolution color video picture. The equipment shall be capable of televising the entire length in one direction. When televising storm drains the camera shall be capable of scanning the joints for three hundred and sixty degrees (360°).

3. Televising Procedures

The camera shall be moved through the line at a uniform rate, stopping when necessary to ensure proper documentation of the condition of the storm drain but in no case shall the television camera be pulled at a speed greater than thirty feet (30') per minute. Manual winches, power winches, TV cable and powered rewinds or other devices that do not obstruct the camera view or interface with proper documentation of the storm drain conditions shall be used to move the camera through the storm drain.
If, during the televising operations, the television camera will not pass through an entire manhole section or storm access point section, the Contractor shall reset the equipment in a manner so that the inspection can continue opposite the obstruction. If the television camera encounters an obstruction within a section not accessible to a manhole or storm drain access point, the Contractor shall remove the obstruction by excavation or other appropriate means, replace whatever pipe is necessary, and retelevise the entire section.

Whenever non-remote powered and controlled winches are used to pull the television camera through the line, telephones, radios, or other suitable means of communication shall be set up between the two manholes or storm drain access points of the section being inspected to ensure that adequate communications exist between members of the crew.

The importance of accurate distance measurements is emphasized. Measurement for location of defects shall be above ground by means of a meter device. Marking on the cable, or the like, which would require interpolation for depth of manhole or storm access points, is not acceptable.

The accuracy of the measurement shall be checked daily by use of a walking meter, roll-a-tape, or other suitable device. Measurements shall begin at the centerline of the upstream manhole or storm drain access point, unless permission is given by the Engineer to do otherwise. Distance shall be shown on the video data view at all times.

4. Documentation of Televising

Audio and written documentation shall accompany all video tape(s) submitted to the Municipal Engineer. The voice recording of the video tape(s) shall make brief but informative comments on data of significance, including, but not limited to, the locations of unusual conditions, type and size of connection, collapsed section, the presence of scale and corrosion, and other discernible features.

The video tape(s) shall include the following:

a. Data View
   i. Report No.
   ii. Date of TV inspection.
   iii. Upstream and downstream manhole, storm drain access point or station numbers.
   iv. Current distance along reach (tape counter footage).
   v. Printed labels on tape container and recording cartridge with location information, date format information, and other descriptive information.
b. Audio
   i. Date of TV inspection.
   ii. Confirmation of upstream and downstream manhole, storm access point or station numbers.
   iii. Description of pipe size, types and pipe joint length.
   iv. Description and location of each defect.
   v. Description and location of each service connection.

c. Written
   i. Date of TV inspection.
   ii. Tape number.
   iii. Location, size, type, and length of pipe.
   iv. Direction of flow and measurement ("From" manhole/storm drain access point/station number "To" manhole/storm drain access point/station number).
   v. Tape counter numbers (beginning and end).
   vi. Sketch showing the street and cross streets where the TV inspection was made.
   vii. Description and location of each defect.
   viii. Description and location of each connection.

**Article 2.4 Measurement**

Measurement for all sizes of pipe shall be based on the horizontal distances and shall be from center to center of manholes, from the center of manholes to center of catch basins, from center of manholes to center of cleanout wye, and from center of manhole to end of pipe including flared end sections. Televising storm drains is considered incidental to the pay item and no separate payment shall be made.

**Article 2.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units.

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
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<tbody>
<tr>
<td>Furnish, Install, and Televise Pipe</td>
<td></td>
</tr>
<tr>
<td>(Size, Type, Class, Material, Gauge and Type of Coating)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 55.03 SUBDRAINS

Article 3.1 General

The Work under this Section consists of the performance of all operations pertaining to furnishing and installing subdrains.

Article 3.2 Material

A. All piping shall be in accordance with the Contract Documents and shall be the sizes shown and specified.

B. The Contractor shall use perforated steel, perforated aluminum or perforated aluminized coated corrugated metal pipe as noted. Corrugated metal pipe shall conform to the provisions of Section 55.02 - Furnish and Install Pipe. Perforations shall be located and sized in accordance with the requirements of AASHTO M-36. The top row of holes shall not be less than 22-1/2 degrees below the horizontal axis as shown on Standard Detail 55-3.

C. Corrugated Polyethylene Pipe (CPEP) shall conform to the provisions of Section 55.02 - Furnish and Install Pipe. Size and locate perforations in accordance with the requirements of AASHTO M252. Cleanly cut perforations so as not to restrict the inflow of water, and uniformly space along the length and circumference of the pipe. Contractor shall locate the top row of holes not less than 22 1/2º below the horizontal axis as shown on Standard Detail 55-3. Center perforations in the corrugation valleys. Provide water inlet area not less than a minimum of one square inch per linear foot of pipe. Perforations may be slots or holes. Slots shall be a maximum of one-tenth inch (1/10") wide. Holes shall not exceed three-sixteenth inch (3/16") diameter.

D. Geotextile fabric shall conform to Division 20, Section 20.25 – Geotextile Fabric, and shall be non-woven, pervious drainage material.

Article 3.3 Construction

Refer to Standard Detail 55-3 for construction of subdrains. Each phase of construction shall be accomplished in accordance with the applicable sections of these Specifications. Excavation and backfill for furnishing and installing of subdrains shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill. Furnishing and installing subdrains shall be in accordance with Section 55.02 - Furnish and Install Pipe. Furnish filter material in accordance with Division 20, Section 20.17 - Furnish Filter Material.

Article 3.4 Measurement

Measurement for all sizes of pipe shall be based on the horizontal distances and shall be from center to center of manholes, from the center of manholes to center of catch basins, from center of manholes to center of cleanout wye, and from center of manhole to end of pipe including flared end sections. Measurement includes: Furnishing, Installing, and
Televising Pipe; Furnishing Filter Material; and, if applicable per Contract Documents, Furnishing and Installing Geotextile Fabric.

Article 3.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section including furnishing and installing pipe, furnishing and placing filter material and, when required by the Contract Documents, furnishing and installing Geotextile Fabric.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish &amp; Install Subdrain (Size, Type, Class, Material, and Gauge of Pipe, and Type of Filter Material)</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Furnish &amp; Install Subdrain with Geotextile (Size, Type, Class, Material, and/or Gauge of Pipe, Type of Filter Material, and Type of Geotextile Fabric)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 55.04    CONNECTIONS TO EXISTING MANHOLES OR CATCH BASINS

Article 4.1    General

The Work under this Section consists of the performance of all operations pertaining to the construction required for connections to existing manholes or catch basins.

Article 4.2    Construction

Excavation and backfill for connections to existing manholes or catch basins shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

Connections to existing manholes or catch basins shall be made in a workmanlike manner. The invert shall be brought into the existing manhole at the elevation shown on the Drawings. The downstream pipe in manholes shall be screened to prevent entry of mortar or other debris from entering the system.

After connection is made to a storm drain manhole and the mortar holding the pipe in place has set, cut the pipe off evenly so that no more than two inches (2") of pipe protrudes into the manhole and any screening shall be removed.

Article 4.3    Measurement

Connection to existing manholes shall be measured as complete units in place.

Article 4.4    Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Connect to Existing Storm Drain Manhole</td>
<td>Each</td>
</tr>
<tr>
<td>Connect to Existing Storm Drain Catch Basin</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.05  MANHOLES AND CATCH BASIN MANHOLES

Article 5.1  General

The Work under this Section consists of the performance of all Work required for the construction of storm drain manholes and catch basin manholes complete with frames and covers.

Article 5.2  Material

A.  Frames and Covers

The requirement for tensile strength of the gray cast iron shall conform to the requirements of AASHTO M-306. Manhole frames, covers, and grates shall be furnished with machined horizontal bearing surfaces and shall conform to the Standard Details. The cover or grate shall not rock when rotated to any position in the frame. Catch basin manhole castings shall conform to the Standard Details.

Gray iron castings shall have appropriate certifications and be individually marked in accordance with the requirements of AASHTO M-306. Castings which do not possess appropriate AASHTO M-306 certifications and markings shall be replaced by the Contractor at no expense to the Owner.

B.  Reinforced Concrete Manholes

Material used in the construction of reinforced concrete manholes shall conform to the requirements of ASTM C-478 and the Standard Details. Cones shall be eccentric unless otherwise approved. Forty-eight inch (48") reinforced concrete pipe may be used for manhole riser sections as an alternate. This pipe shall conform to the requirements of ASTM C-76 with a minimum thickness of five inches (5").

Each precast concrete barrel section and eccentric cone shall be set and sealed by use of a pre-molded plastic gasket pipe joint sealer as manufactured by Henry Co, Ram-Nek Sealant Division or equal and installed to the manufacturer's specification and meets AASHTO M-198, ASTM C990 or Federal Specification SS-S-210.

Cement for mortar used in the construction of manholes shall conform to the requirements of ASTM C-150, Type II. Sand shall conform to the requirements of AASHTO M-45. The mortar shall be composed of one (1) part cement and three (3) parts sand. The joints shall be constructed so as to produce a smooth, regular, watertight surface. Water shall be added in minimum amounts to provide plasticity in placing the mortar. Each concrete adjusting ring and manhole cover/frame that falls outside of a paved road section shall be set and sealed by a pre-molded plastic gasket sealer. Each concrete adjusting ring and manhole cover/frame that falls in a paved road section/sidewalk shall be set in a full bed of mortar.
Refer to Division 30, Section 30.01, Article 1.6 - Mix Requirements for Classes of Concrete, for Specifications pertaining to Class A-3 concrete as required in forming manhole inverts.

Reinforcement steel shall conform to the requirements of ASTM A-185, ASTM A-615, Grade 60 steel, or better, and the Standard Details.

Article 5.3 Construction

A. General

Excavation and backfill for the construction of storm drain manholes and catch basin manholes shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

All portions of the manholes must be approved by the Engineer prior to installation in the storm drain system. The Contractor shall provide timely notice (at least two Working days in advance of casting) to allow time for the Engineer to arrange for necessary inspections. Installation of manhole sections without the Engineer's written approval shall not be allowed. This approval does not relieve the Contractor of the responsibility for protection of manholes against damage during handling and installation.

The manhole frames and covers shall be brought to grades shown on the Drawings unless otherwise approved by the Engineer. Manhole rings shall be set in a full bed of mortar and made secure. Grade adjustment rings must be set centered over the manhole and catch basin cone or lid opening with no lateral offset. No more than a one-quarter inch (1/4") lateral offset is permitted between grade adjustment rings. Total cumulative offset between grade adjustment rings shall not exceed one-half inch (1/2"). Manhole rings and catch basin frames shall be set centered on the opening with a maximum lateral offset of one-half inch (1/2") permitted.

Manholes shall be installed at the location shown on the Drawings and primary leads shall enter radially at the invert elevations specified. The base section shall be set plumb on a prepared surface. Prepared surface shall be compacted to a minimum of ninety-five percent (95%) of maximum density.

In the case of precast manhole barrel sections where holes need to be bored to provide for the storm drain pipe, the diameter of the bore shall not exceed the outside diameter of the storm drain pipe plus one and one-half inches (1.5").

Where indicated on the Drawings, a stub shall be provided for future connections to the manhole. The stub shall be sized and positioned as indicated. The end of the stub shall be stopped with a wooden plug, concrete biscuit, or other adequate methods to prevent water, earth or other substances from entering the pipe. Manholes up to twelve feet (12') in depth shall have ten foot (10') stubouts; over twelve feet (12') in depth shall have twenty foot (20') stubouts.
In the case of poured-in-place manhole construction, if the Contractor elects to accomplish the manhole construction utilizing more than one continuous concrete pour, a keyed construction joint shall be used. These manholes shall have poured-in-place bases. Precast concrete barrel sections shall be set and sealed with premolded plastic gasket. Premolded plastic gaskets for sealing pre-cast concrete barrel sections for manholes shall meet AASHTO M-198, ASTM C-990, or Federal Specification SS-S-210 and shall be installed in accordance with the manufacturer’s recommendations. Gaskets shall be trimmed on the inside of the manhole to prevent the excess gasket material from entering the storm drain lines.

B. Storm Drain Manholes and Catch Basin Manholes

Contractor shall construct storm drain manholes in accordance with the Drawings and Standard Details. In the invert of manholes, Contractor shall construct a catch of eighteen inches (18") minimum depth, unless otherwise specified.

After connecting the storm drain pipe to reinforced concrete manhole or catch basin, seal annular space around pipe penetrations with cement mortar, or an approved equal. Cement mortar shall conform to the requirements of ASTM C-150, Type II. After the mortar has firmly set, Contractor shall cut the pipe evenly so that no more than two inches (2") of the pipe protrudes into the manhole.

Article 5.4 Measurement

Manholes and catch basin manholes shall be measured as units complete in place. Depth of manholes and catch basin manholes shall be based upon a measurement to the nearest foot from top of casting to the top of the base slab. Standard depths for manholes and catch basin manholes shall be constructed in accordance with the Standard Details and designated as to type.

<table>
<thead>
<tr>
<th>TYPE</th>
<th>STANDARD DEPTH</th>
</tr>
</thead>
<tbody>
<tr>
<td>Type I, II, III</td>
<td>twelve feet (12’)</td>
</tr>
</tbody>
</table>

All depths over the specified standard depth shall be paid for under the bid item "Additional Depth to Manhole" as defined below:

Additional Depth for Manholes:

This item consists of the construction of additional depth to manholes over and above the twelve foot (12’) depth specified below.

Additional depth to manholes and catch basin manholes shall be complete in place.
Article 5.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct (Type, Diameter*) Manhole</td>
<td>Each</td>
</tr>
<tr>
<td>Construct (Type) Catch Basin Manhole</td>
<td>Each</td>
</tr>
<tr>
<td>Additional Depth to (Type) Manhole</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>

* For Type III manholes, include the diameter in the descriptor for the appropriate pay item.
SECTION 55.06 WATERTIGHT MANHOLE FRAMES AND COVER

Article 6.1 General

The Work under this Section consists of the performance of all Work required for the construction of watertight manhole frames and covers.

Article 6.2 Material

Watertight frames and covers for manholes and similar appurtenances shall be of cast iron and conform to the dimension shown in the Standard Details. The requirement for tensile strength of the gray iron shall be 30,000 PSI minimum in accordance with the requirements of ASTM A-48 and the requirement for transverse breaking load shall be 2,000 pounds in accordance with the requirements of ASTM A-438. Contact surfaces between frames and covers shall be machined to provide a uniform contact surface. Manhole covers shall have identification letters as shown on the Standard Details.

Article 6.3 Construction

Provide watertight Manhole Frames and Covers as indicated on the Drawings and in accordance with the Standard Details.

Article 6.4 Measurement

Watertight manhole frames and covers shall be measured as complete units in place.

Article 6.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment is to be made only for the additional cost of furnishing and installing the watertight frame and cover which exceeds the cost of the standard frame and cover included in the completed manhole unit price.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Additional Cost of Watertight Manhole Frame and Cover</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.07  ADJUST STORM DRAIN MANHOLE CONE TO FINISH GRADE

Article 7.1  General

The Work under this Section consists of providing all operations pertaining to the adjustment of existing manhole cones to finish grade. All broken and/or missing manhole components are to be replaced with new materials furnished and installed by the Contractor.

Article 7.2  Material

All materials used in the adjustment of manhole cones including mortar, steps, barrel sections, blackpremolded plastic gaskets, etc., shall conform to the requirements for manholes as outlined in Section 55.05 - Manholes and Catch Basin Manholes. Radial concrete manhole blocks may be used for upward adjustments in certain cases if approved by the Engineer.

Article 7.3  Construction

The Contractor shall remove the existing cone and add to or remove portions of the barrel of each manhole requiring a cone adjustment. Each precast concrete barrel and cone section shall be set upon and sealed with a premolded plastic gasket which shall meet AASHTO M-198, ASTM C990, or Federal Specification SS-SS-210. Any damage to manholes resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor’s expense. All inverts, benchwalls, and/or catch areas shall be left clean and free from any foreign materials.

Contractor shall adjust the manhole cone to finish grade prior to placement of asphalt pavement. New asphalt shall not be cut for adjustments.

Article 7.4  Measurement

Manhole cone adjustments shall be measured as units, complete in place.
Article 7.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for cone adjustments shall include compensation for changes in height per the applicable Standard Details, unless otherwise directed by the Engineer. In no case will payment for both ring and cone adjustments be made for the same manhole. Any adjustments requiring cutting of new asphalt shall not be paid and shall be deducted from the plan quantity.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Storm Drain Manhole Cone</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.08  ADJUST STORM DRAIN MANHOLE RING TO FINISH GRADE

Article 8.1  General

The Work under this Section consists of providing all operations pertaining to the adjustment of existing manhole rings to finish grade. All broken and/or missing manhole components are to be replaced with new materials furnished and installed by the Contractor in accordance with these Specifications.

Article 8.2  Material

All materials used in the adjustment of manhole rings shall conform to the requirements for manholes as outlined in Section 55.05 – Manholes and Catch Basin Manholes.

The Contractor may utilize Neenah R-1979 Series Manhole Adjusting Rings, or an approved equal, for adjusting the manhole to finished grade.

Article 8.3  Construction

The Contractor shall adjust the manhole rings in accordance with the applicable Standard Details. The Contractor shall set the adjusting rings in a bed of premolded plastic gasket material that meets AASHTO M-198, ASTM C990, or Federal Specification SS-S-210. The casting can be set in a bed of mortar with steel adjusting shims in the event the grade will not allow the premolded plastic gasket material. The steel shims shall be placed in four locations as a minimum and must be approved by the Engineer. Any damage to manholes resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor’s expense.

Grade adjustment rings must be set centered over the manhole and catch basin cone or lid opening with no lateral offset. No more than a one-quarter inch (1/4”) lateral offset is permitted between grade adjustment rings. Total cumulative offset between grade adjustment rings shall not exceed one-half inch (1/2”). Manhole rings and catch basin frames shall be set centered on the opening with a maximum lateral offset of one-half inch (1/2”) permitted.

Milling is an approved method of lowering the manhole grade. A horizontal milling processware as the casting is milled to lower the top to meet the finish grade of the street. This method must be submitted to the Engineer for approval.

Contractor shall remove and replace pavement around the manhole prior to adjustment in such a way to minimize impact to the travel path of the roadway. Contractor shall either use infrared treatment to amalgamate old and new pavement or shall make the pavement cut in such a way to prevent a straight line patch from occurring perpendicular to the direction of travel. Pavement cuts shall be made in a diamond shape in relation to the travel path rather than a square shape.

Contractor shall adjust the manhole cone to finish grade prior to placement of asphalt pavement. New asphalt shall not be cut for adjustments.
Article 8.4 Measurement

Manhole ring adjustments shall be measured as units, complete in place.

Article 8.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 – Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for ring adjustment shall include full compensation for changes in height. In no case will payment for both ring and cone adjustments be made for the same manhole. Any adjustments requiring cutting of new asphalt shall not be paid and shall be deducted from the plan quantity.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Storm Drain Manhole Ring</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.09  CONSTRUCT CATCH BASIN

Article 9.1  General

The Work under this Section consists of the performance of all operations pertaining to the construction and installation of catch basins.

Article 9.2  Material

Materials used in the construction of catch basins shall conform to the requirements of ASTM C-478 and the Standard Details.

Cement for mortar used in the construction of catch basins shall conform with the requirements of ASTM C-150, Type II. Sand shall conform with the requirements of AASHTO M-45.

Article 9.3  Construction

Excavation and backfill for furnishing and installing of catch basin shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

After the mortar has set firmly, the pipe is to be cut off evenly so that not more than two inches (2") of the pipe protrudes into the catch basin.

Reducing slab shall be set and sealed by a pre-molded plastic gasket joint sealer as manufactured by Henry Co., Ram-Nek Sealant Division or equal and installed to the manufacturer’s specification. Plastic gasket joint sealers shall meet AASHTO M-198, ASTM C-990, or Federal Specifications SS-S-210.

Contractor shall bring catch basin rings and covers to the grades shown on the Drawings. Grade stakes defining the elevation of the casting, and hub stakes with tacks to define the line for the curb face shall be set by the Contractor. The Contractor may accomplish final setting of the casting by wedging it up with masonry material as approved by the Engineer. The casting shall then be set in a full bed of mortar and made secure.

Mortar used in the construction of catch basins shall be composed of one (1) part cement and three (3) parts sand. All joints and connections are to be mortared. The joints shall be made so as to produce a smooth, regular, watertight surface. Water shall be added in minimum amounts to provide plasticity in placing the mortar.

Contractor shall use Class A-3 concrete, as defined in Division 30, Section 30.01, Article 1.6 - Mix Requirements For Classes of Concrete, in the formation of catch basin base slabs.

Article 9.4  Measurement

Catch Basins shall be measured as units complete in place.
Article 9.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Catch Basin</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.10 RELOCATE CATCH BASIN OR CATCH BASIN MANHOLE

Article 10.1 General

The Work under this Section consists of providing all operations pertaining to relocating of existing catch basin or catch basin manholes.

Article 10.2 Material

All materials used in relocation of catch basins or catch basin manholes shall conform to the requirements for catch basins as outlined in Section 55.05 - Manholes and Catch Basin Manholes, and Section 55.09 - Construct Catch Basin.

Article 10.3 Construction

The Contractor shall note the fact that he may be required to relocate more than one type of catch basin or catch basin manhole under this Contract. All excavation, trenching and backfill necessary for the removal and relocation shall be considered incidental to this item. The Contractor shall backfill the excavation with suitable, non-frost-susceptible material and compact it to not less than ninety-five percent (95%) of maximum density as directed by the Engineer. If additional material is required for backfill it will be paid for under the item "Furnish Trench Backfill." Existing leads may require relocation up to a maximum length of fifteen feet (15') to provide proper alignment. Such relocation shall be considered incidental to this Item. Relocation of existing pipe leads and any additional pipe leads shall be incidental Work. Pipe used shall be the same size and type as the existing leads. The relocated catch basin or catch basin manholes shall be adjusted to finish grade as directed by the Engineer.

Article 10.4 Measurement

Relocation of catch basins or catch basin manholes will be measured on a basis of units complete in place at the new location and accepted by the Engineer.

Article 10.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Relocate Catch Basin</td>
<td>Each</td>
</tr>
<tr>
<td>Relocate Catch Basin Manhole</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.11 REMOVE MANHOLE OR CATCH BASIN

Article 11.1 General

The Work under this Section consists of providing all operations pertaining to the removal and disposal or salvage of existing manholes.

Article 11.2 Construction

Salvaged materials shall be removed in a workman-like manner and delivered to a site as directed by the Engineer. Non-salvageable materials shall be removed to a Contractor-provided disposal site.

Any excavation required in the removal shall be considered incidental to this item. The Contractor shall backfill the excavation with a suitable, non-frost susceptible material and compact it to not less than ninety-five percent (95%) of maximum density as directed by the Engineer. If additional material is required for backfill, it will be paid for under the Item "Furnish Trench Backfill." Existing pipes shall be suitably plugged and abandoned unless otherwise noted.

Article 11.3 Measurement

Removal of existing manholes or catch basins will be measured as units.

Article 11.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Manhole</td>
<td>Each</td>
</tr>
<tr>
<td>Remove Catch Basin</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.12  ADJUST CATCH BASIN TO FINISH GRADE

Article 12.1  General

The Work under this Section consists of providing all operations pertaining to the adjustment of existing catch basins to finish grade.

Article 12.2  Material

All materials used in the adjustment of catch basins shall conform to the requirements for catch basins as outlined in Section 55.09 - Construct Catch Basin.

Article 12.3  Construction

Rotational as well as vertical displacement of the catch basin top and casting might occur. All adjustments will be accomplished as directed by the Engineer. Any damage to catch basins resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor's expense.

Grade adjustment rings must be set centered over the catch basin cone or lid opening with no lateral offset. No more than a one-quarter inch (1/4") lateral offset is permitted between grade adjustment rings. Total cumulative offset between grade adjustment rings shall not exceed one-half inch (1/2"). Catch basin frames shall be set centered on the opening with a maximum lateral offset of one-half inch (1/2") permitted.

Article 12.4  Measurement

Catch basin adjustments shall be measured as units, complete in place.

Article 12.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit.

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Catch Basin to Finish Grade</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.13   ABANDON CATCH BASIN LEAD

Article 13.1   General

The Work under this Section consists of performing all operations pertaining to the abandonment of catch basin leads. Catch basin leads to be abandoned may be crushed in place, filled with sand slurry, or removed, at Contractor's option and approval by the Engineer.

Article 13.2   Materials

Sand slurry shall consist of a mixture of water and sand with an approximate ratio of seven gallons of water per cubic foot of sand. Native materials that contain no lumps, frozen material, organic matter, or other deleterious material are acceptable for use in the slurry mixture.

Article 13.3   Construction

Contractor shall abandon all catch basin leads as shown on the Drawings. The opening in the storm drain manhole where the catch basin lead enters shall be plugged with concrete grout and the lead filled with sand slurry.

Where catch basin leads lie within trench excavation, as called for in the Drawings and Specifications, the leads shall be removed.

Article 13.4   Measurement

Abandonment of each catch basin lead shall be measured as a complete unit. This item will include materials, excavations, placement of materials, disposal of unusable materials, backfill, and incidental operations.

Article 13.5   Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Abandon Catch Basin Lead</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.14   CONSTRUCT STORM CLEANOUT

Article 14.1   General

The Work under this Section consists of the performance of all Work required for the construction and installation of storm drain cleanouts.

Article 14.2   Material

Materials used in the construction of storm drain cleanouts shall conform to the Standard Details.

Article 14.3   Measurement

Storm drain cleanouts shall be measured as units, complete in place.

Article 14.4   Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Storm Drain Cleanout</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.15  ADJUST STORM CLEANOUT TO FINISH GRADE

Article 15.1  General

The Work under this Section consists of providing all operations pertaining to adjustment of existing cleanouts to finish grade. All broken and/or missing cleanout components are to be replaced with new materials furnished and installed by the Contractor in accordance with these Specifications.

Article 15.2  Material

All materials used in the adjustment of cleanouts shall conform to the requirements for cleanouts as outlined in Section 55.14 - Construct Storm Cleanout.

Article 15.3  Construction

The Contractor may be required to adjust more than one type of cleanout under this Contract. All adjustments will be accomplished as directed by the Engineer. Any damage to cleanouts resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor's expense.

Article 15.4  Measurement

Cleanout adjustments will be measured per unit, complete in place.

Article 15.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Storm Cleanout to Finish Grade</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.16 CONSTRUCT DROP CONNECTION

Article 16.1 General

The Work under this Section consists of the performance of all Work required for the construction and installation of storm drain drop connections to manholes.

Article 16.2 Material

Pipe and fittings used in the construction of drop connections for storm drain shall conform to the requirements of Section 55.02 - Furnish and Install Pipe and the Standard Details.

Article 16.3 Construction

Excavation and backfill for the construction of drop sewer connection to manhole shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

Over-excavation under drop connection shall require compaction of not less than ninety-five percent (95%) of the maximum density prior to installation of the pipe and fittings, or the concrete cradle.

Refer to Division 30, Section 30.01, Article 1.6 - Mix Requirements for Classes of Concrete for specifications pertaining to Class A-3 concrete.

Article 16.4 Measurement

Storm drain drop connections shall be measured as units, complete in place.

Article 16.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Drop Storm Drain Connection (Detail #)</td>
<td>Each</td>
</tr>
</tbody>
</table>

SECTION 55.17  FLUME DOWNDRAIN

Article 17.1  General

The Work under this Section consists of performing all operations pertaining to furnishing and installing flume downdrain(s) with anchor assemblies at locations shown on the Drawings.

Article 17.2  Materials

All material utilized in the fabrication of the galvanized metal flume downdrain(s) shall conform to Section 55.02 - Furnish and Install Pipe with a minimum sheet thickness of six-hundredths inches (0.060”).

Article 17.3  Construction

The flume downdrain(s) shall be fabricated in accordance with the details and dimensions shown on the Drawings. No dissimilar metal shall be allowed at any installation. Anchor assemblies shown on the Drawings may be used with an aluminum installation provided the anchor assemblies are electrically insulated. All flume sections shall be connected together and to the existing pipe by means of galvanized bolts as indicated on the Drawings.

Article 17.4  Measurement

Measurement shall be based on the horizontal length of flume downdrain measured from the top end of the flume downdrain to end of flared or half-round metal pipe modified end section complete in place with anchors properly placed in the ground and bolted to the flume downdrain pipe.

Article 17.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install Flume Downdrain (Size)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 55.18  FOOTING DRAIN SERVICES

Article 18.1  General

The Work under this section includes all material, labor, and equipment necessary for the disconnection and reconnection of six inch (6”) footing drain services to the storm drain system.

Article 18.2  Material

Provide six inch (6”) CPEP (Type S) Pipe and Filter Material (Type B).

Connect footing drain services to the storm drain with a six inch (6”) saddle of a type recommended by the pipe manufacturer.

Article 18.3  Construction

Excavation and backfill for the construction of footing drain services is incidental to the bid item reconnect footing drain services. The number and approximate location of footing drain services are shown on the Drawings.

Article 18.4  Measurement

Footing drain services is measured as units, complete in place.

The Work under this Section includes all materials, equipment, and work required to disconnect and reconnect the footing drain services as indicated on the Drawings and in accordance with this Division and Division 20 – Earthwork. Such materials, equipment, and Work are incidental and no additional payment is made for the following:

- Trench Excavation and Backfill, Unusable or Surplus Excavation, Disposal of Unsuitable or Surplus Material, Usable Excavation, Type B Filter Material, Furnish and Install Subdrain Pipe, Mechanical Compaction, Type II Trench Backfill, Shoring, Sheetinig, and Bracing, Portable and Steel Shield, and Canousa Wrap.

Article 18.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Reconnect Footing Drain Service</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.19 CONSTRUCT OPEN DITCH

Article 19.1 General

The Work under this Section consists of the performance of all Work required for the excavation, embankment and spreading of material necessary to construct an open ditch.

Article 19.2 Construction

A. Excavation

Excavation shall be to the grade and ditch cross section shown on the Drawings. The final ditch shall have no projections of roots, stumps, rock or similar matter. Material hauled from the job site for disposal shall be paid for under Division 20, Section 20.27 – Disposal of Unusable or Surplus Material.

B. Embankment

Embankment shall be to the shape and at the location shown on the Drawing. The type of material utilized to construct ditch banks and dikes shall be as noted on the Drawing, or as approved by the Engineer. If additional material is required for embankment, it will be paid for under Division 20 - Earthwork.

C. Cleanup

The Contractor shall maintain the ditch and keep it open and free from all debris, as directed by the Engineer until final acceptance.

Article 19.3 Measurement

Measurement for open ditch construction shall be per linear foot along the slope of the ditch.

Article 19.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Construct Open Ditch</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 55.20  CULVERT

Article 20.1  General

The Work under this Section consists of the performance of all materials and operations required to furnish and install culverts.

Article 20.2  Construction

Excavation and backfill for furnishing and installing of culverts shall be in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

The Contractor shall furnish and install culverts as shown on the Drawings. The pipe shall be installed to the alignment and grades as required by the Drawings. Pipe materials shall meet the Specifications included in Section 55.01 - General and Section 55.02 - Furnish and Install Pipe, of this Division. The pipe shall be installed so that there is a minimum of twelve inches (12") of cover over the pipe before the placement of surfacing materials. Excavation, backfilling, compaction, and grading or ditching necessary to direct water into or out of the culvert, are incidental items and no separate payment shall be made.

Where additional backfill material is required, it shall be classified fill or backfill in accordance with Division 20, Section 20.21 – Classified Fill and Backfill and as directed by the Engineer. Disposal of unusable material shall be paid under "Unusable Excavation" or "Disposal of Unsuitable or Surplus Material” as designated in the Bid Proposal.

Article 20.3  Measurement

Measurement of culverts shall be per linear foot along the slope of the pipe from end to end.

Article 20.4  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Culvert (Pipe Size, Type, Gauge, Shape)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 55.21 FIN DRAIN

Article 21.1 General

The Work under this Section consists of performing all operations pertaining to furnishing and installing an impervious subsurface fin drain system as shown on the Drawings or as directed by the Engineer.

Article 21.2 Materials

The fin drain system shall consist of a flexible, impervious, vertical core made of a deeply-dimpled, high-strength styrene sheet and a perforated storm pipe enveloped in a non-woven polypropylene filter fabric.

The subsurface fin drain system shall be Miradrain 5000 (for double-sided drainage) manufactured by Mirafi Inc., P.O. Box 240967, Charlotte, NC, 28224, telephone (704) 523-7477, or an approved equal.

A. Fin Core

The fin core shall consist of a deeply-dimpled, high-strength, non biodegradable styrene sheet. Provide fin core which is dimpled on both sides of the shaft. Dimple pattern shall create open channels between the dimples 0.40 to 0.80 cm wide and not less than 0.80 cm deep, which allows water flow along the face of the fin core on both sides in all directions.

B. Filter Fabric

The filter fabric shall conform to the requirements of Division 20, Section 20.25 - Geotextile Fabric for Subsurface Drainage and Riprap Liner or an approved equal.

C. Pipe

The perforated encased pipe shall conform to the requirements of Section 55.02 - Furnish and Install Pipe. Compaction adjacent to the pipe shall conform to Division 20, Section 20.13 - Trench Excavation and Backfill and the manufacturer’s recommendations. The perforations in the pipe shall conform to Section 55.03 - Subdrains.

Article 21.3 Construction

Contractor shall install the fin drain in accordance with the manufacturer’s recommendations and the applicable provisions of Division 20, Sections 20.17 - Furnish Filter Material; Section 20.01, Article 1.5 - Compaction Standards; Section 20.25 - Geotextile Fabric; Section 55.02 - Furnish and Install Pipe; Section 55.03 – Subdrains; and this Section. The Work under this Section shall include mechanical compaction, non-woven geotextile fabric, pipe, fin core, installing the fin drain system, filter material (Type C), Trench Excavation and Backfill, and Disposal of Unusable or Surplus Material.
Article 21.4 Measurement

The method of measurement for furnishing and installing the fin drain shall be per linear foot based on the horizontal distance measured from center of manhole to center of manhole and center of manhole to center of cleanout riser.

Article 21.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install Fin Drain (Size, Type)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 55.22 OIL AND GRIT SEPARATOR

Article 22.1 General

The Work under this section consists of performing all operations pertaining to constructing a storm drain oil and grit separator, complete with manhole structure, frames, covers, and diversion apparatus as shown on the Drawings, or as the Engineer directs.

Article 22.2 Description

The oil and grit separator is a below-grade structure consisting of a prefabricated diversion apparatus fastened securely to the inside of a concrete storm drain manhole. The separator is designed to remove oil and sediment from stormwater and to bypass flows during peak events to prevent scour of accumulated sediment.

Contractor shall furnish and install an oil and grit separator, Stormceptor Model STC3600 manufactured by:

Rinker Materials/Stormceptor
800 NE Tenney Road, Suite 413
Vancouver, WA 98685
Phone: 503-572-9894
FAX: 503-296-2023

Local Contact: D & S Concrete, Inc.
2140 East Dimond Boulevard
Anchorage, AK 99507
Phone: 907-349-6031
FAX 907-349-4597
or an approved equal.

Article 22.3 Materials

All excavation, backfill, and compaction required to install the oil and grit separator is incidental to this item. Contractor shall backfill the excavation with Type II Classified Fill and Backfill material. If foundation material is required, it will be paid under the bid item “Foundation Backfill (Type II).”

The storm drain manhole shall conform to the requirements of Section 55.05 - Manholes and Catch Basin Manholes and the Drawings. The diversion apparatus shall conform to the requirements of the oil and grit separator manufacturer’s specifications.

Contractor shall provide access to the structure through two (2) manhole frames and covers. The smaller cover shall conform to Standard Detail 55-4. The manufacturer of the oil and grit separator shall provide the larger cover clearly marked “oil/grit separator” and the larger cover shall support HS-20 loadings.

Article 22.4 Construction

Contractor shall install the separator in accordance with Section 55.05 - Manholes and Catch Basin Manholes and with the separator unit manufacturer’s specifications.
Contractor shall backfill around the manhole with a minimum of three feet (3’) Type II Classified Fill and Backfill to the full depth of the manhole, compacted in accordance with Division 20, Section 20.21 - Classified Fill and Backfill. Classified Fill and Backfill is incidental to this pay item, and no separate payment shall be made.

**Article 22.5 Measurement**

Oil and grit separator is measured as a complete unit in place and shall include the concrete manhole, diversion apparatus, frames, covers, and classified backfill. All clearing and grubbing or excavation, and providing Type II Classified Fill and Backfill, disposal of unusable or unsuitable material necessary to construct the oil and grit separator, is incidental to this Work item. Foundation backfill, if required, will be paid pursuant to Division 20, Section 20.19 - Furnish Foundation Backfill.

**Article 22.6 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Oil and Grit Separator (Model #)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 55.23   HEAT TRACE SYSTEM

Article 23.1   General

The Work under this Section consists of performing all operations pertaining to furnishing and installing a heat trace system as required by the Drawings and these Specifications.

Locations of heat cables, manholes, catch basin and storm drain services, and appurtenances shown on the Drawings are approximate and the exact locations will be established in the field in consultation with the Engineer.

Materials furnished shall be new, except such used materials as may be specifically provided for on the Drawings or Special Provisions. Where an existing system is to be modified, the existing materials shall be reused on the project or disposed as shown on the Drawings or in the Special Provisions.

All systems shall be complete and in operation with all materials in conformance with Contract Documents at the time of final acceptance.

Article 23.2   Regulations and Codes

Work shall be as specified in Division 80, Section 80.01, Article 1.2 - Regulations and Codes and this Section.

Heat cables shall be provided with Factory Mutual or UL listing indicating approval for freeze protection in the indicated locations shown on the Drawings and specified herein.

Article 23.3   Equipment Lists and Drawings

Work shall be as specified in Division 80, Section 80.01, Article 1.3 - Equipment List(s) and Drawings and this Section. The portfolio(s) shall include:

1. Submit manufacturer’s installation and testing instructions.

2. Submit operations and maintenance instructions.

3. Submit complete control diagrams in block and schematic form indicating wire sizes, raceway sizes, locations, component configurations, equipment layouts, and all other information necessary to completely describe each and every portion of the controls installation.

Article 23.4   Warranties, Guarantees, and Inspection Sheets

Provide in accordance with Division 80, Section 80.01, Article 1.4 - Warranties, Guarantees, and Inspection Sheets.
Article 23.5 Safety Precautions

Before starting Work on existing heat cable systems, Contractor shall disconnect and lock-off load centers feeding circuits for the heat trace system. Disconnection shall be made by utility or main and branch breakers locked in “off” position.

Suitable signs shall be posted at load centers when Contractor is working on any of the circuits fed from the load center.

Article 23.6 Heat Cables

Self-limiting thermoplastic insulation, parallel resistance heating cable with a 300-volt insulation rating, with output of eight (8) watts/linear foot at a 240-volt single phase operation and tinned copper metal braid with fluoropolymer overjacket as manufactured by Thermon Type FLX (8-FLX-2-FOJ). Substitutions: Items of other manufacturers of same quality, construction, rating, capacity, function, and characteristics may be acceptable.

Labeling on cable: Manufacturer, catalog number, rating in volts, or in volts and AMPS.

Color coding: Non-heating lead wire shall be red for 240-volt nominal circuit voltage.

Contractor shall install cables in accordance with manufacturer’s recommendations for this application.

The heat cable insulation’s resistance and continuity shall be tested in accordance with methods and times as required in manufacturer’s recommendations. Tests shall be conducted and test results recorded at the following minimum times:

1. When the cable arrives on the job site before installation.
2. After it is installed completely.

Contractor shall install heat cable(s) in watertight galvanized rigid conduit and junction boxes in manholes, storm drains, and other parts of storm drain systems as shown on the Drawings.

Contractor shall install cable in a single length without splices or joints. Contractor shall provide bonding and grounding of drain in accordance with cable manufacturer’s recommendations.

All cables shall be protected by 30 mA GFI circuit breakers.

Contractor shall secure cable in dead-end raceways in accordance with manufacturer’s recommendations to ensure cable remains in place after installation.

See Drawings for additional installation requirements.

Contractor shall mount all components grouped neatly and securely fastened to surfaces in accordance with manufacturer’s recommendations.
Provide engraved nameplate at junction box serving heat cables in each storm drain manhole. Nameplate shall be white letters on red background and include the message “WARNING 240 V. HEAT CABLE(S)” and include load center location.

**Article 23.7 Measurement**

The length of measurement shall be the horizontal length of the storm drain systems to be heat traced. Measurement includes all terminations, markings, and incidental supplies as required to meet the provisions of this Section. Cables installed in manholes and catch basins will not be measured, but rather the following distances will be considered standard unless determined otherwise by the Engineer.

- Manhole       forty feet (40’)
- Catch Basin   sixteen feet (16’)

**Article 23.8 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Heat Trace</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
INDEX OF STANDARD DETAILS

55-1  CPEP Storm Pipe Connection
55-2  Corrugated Metal Pipe Band Detail
55-3  Subdrain
55-4  Storm Drain Manhole Type I - Pipe ≤ 24"
55-5  Storm Drain Manhole Type II - 24" to 36"
55-6  Storm Drain Manhole Type III
55-7  Storm Drain Manhole Cover
55-8  Storm Drain Top Intake Cover
55-9  Storm Drain Beehive Intake Cover
55-10 Manhole Heights
55-11 Precast Concrete Reducing Slab (72" or 48" to 26")
55-12 Precast Concrete Reducing Slab (72" to 48")
55-13 Precast Concrete Two Hole Reducing Slab (72" to two 25 1/2")
55-14 Precast Concrete Reducing Slab (112" to 72")
55-15 Precast Concrete Reducing Slab (140" to 72")
55-16 Precast Concrete Reducing Slab (168" to 72")
55-17 Manhole Cone Adjustment
55-18 Manhole Ring Adjustment
55-19 Catch Basin Inlet Frame and Hood for Type 1 Curb and Gutter
55-20 Catch Basin Inlet Grates for Type 1 Curb and Gutter
55-21 Catch Basin Inlet for Type 2 Curb and Gutter
55-22 Precast Catch Basin
55-23 Storm Drain Cleanout
55-24 Storm Drain Drop Connection (2’ Min Drop)
55-25 Storm Drain Drop Connection (4’ Min Drop)
55-26 Footing Drain Service Detail
55-27 Driveway Culvert Details
NOTES:

1. MATCH INVERTS OF EXISTING PIPE AND NEW CPEP PIPE. INNER SLEEVE SHALL FORM A SMOOTH TRANSITION, WITHOUT AN ABRUPT EDGE WITH NEW CPEP PIPE AND EXISTING CMP PIPE.

2. INSTALL 24” LONG INNER SLEEVE.

3. INSTALL A 36” LONG OUTER SLEEVE, CENTER ON JOINT, AS A CONCRETE FORM.

4. FILL OUTER SLEEVE WITH NON-SHRINK MORTAR.

5. USE POTABLE WATER IN MINIMUM AMOUNTS TO PROVIDE PLASTICITY IN PLACING THE MORTAR.

6. BACKFILL AND COMPACT TRENCH.
NOTES:
1. 12” THRU 36” PIPE ENDS RE-CORRUGATED TO ANNULAR 2 VALLEYS MIN. PER END.
2. 48” THRU 120” PIPE ENDS RE-CORRUGATED TO ANNULAR 4 VALLEYS MIN. PER END.
3. BAND ANGLES TO BE 2”x2”x12” GA. MIN.
4. BAND MATERIAL AND FABRICATION SHALL CONFORM TO AASHTO M-36 AND AASHTO M-218; PROVIDE 16 GAUGE BANDS FOR 12” THRU 120” DIAMETER PIPES.
5. DIMPLED TYPE CONNECTING BANDS ARE ALLOWABLE ONLY WHERE FITTINGS ARE USED IN NEW OR EXISTING CONSTRUCTION, FOR REPAIRS TO DAMAGED CMP, AND FOR EXTENSIONS TO CMP WITHOUT ANNULAR ENDS. SIZE BANDS IN ACCORDANCE WITH ABOVE SCHEDULE (MIN. 12”).
6. BOLT SIZE SHOULD BE 1/2” DIAMETER BY 8” LONG. NUTS SHALL BE PROVIDED WITH A WASHER.
NOTES:
1. PROVIDE FILTER MATERIAL TYPE AS SPECIFIED IN THE CONTRACT DOCUMENTS.
2. COMPACT BACKFILL UNDER THE EXISTING OR PROPOSED ROAD PRISM TO A MINIMUM OF 95% OF MAXIMUM DENSITY.
3. TRENCH BACKFILL SHALL BE CLASSIFIED BACKFILL AS SPECIFIED IN THE DRAWINGS.
NOTES:

1. MANHOLE SECTIONS SHALL CONFORM TO A.S.T.M. C-478.

2. EXTEND PIPE 2" INTO MANHOLE. SEAL PIPE PENETRATIONS WITH NON-SHRINKABLE GROUT MIXED WITH POTABLE WATER I.A.W. MANUFACTURERS RECOMMENDATIONS.

3. BLOCKOUTS SHALL BE FORMED.

4. PLACE RUNGS 12" ON-CENTER ON UNOBSERVED SIDE OF MANHOLE 18" MAX. FROM BOTTOM OF MANHOLE & 6" MAX. FROM TOP OF CONE. IF UNOBSERVED SIDE NOT AVAILABLE, BOTTOM RUNG TO BE PLACED 6" OVER SMALLEST PIPE. SEE MANHOLE STEP STANDARD DETAIL 50-6.

5. MANHOLE SHALL HAVE MINIMUM OF ONE 6" GRADE RING.

6. BACKFILL AROUND MANHOLE WITH A MINIMUM OF 3' TYPE II CLASSIFIED FILL & BACKFILL. BACKFILL SHALL BE INCIDENTAL TO COST OF MANHOLE INSTALLATION.

7. CATCH BASIN LEADS SHALL ENTER THE MANHOLE AT LEAST ONE PRIMARY LEAD DIAMETER ABOVE THE TOP OF THE PRIMARY LEAD UNLESS MINIMUM PIPE SLOPES CANNOT BE ACHIEVED.

8. STEEL REQ'D FOR BARREL SHALL CONFORM TO A.S.T.M. C-478. EMBED STEEL IN BASE SO THAT FIRST BARREL SECTION IS CONNECTED WITH BASE.

9. "RAM-NEK" OR EQUAL AND PRIME BARREL JOINTS. HEAT "RAM-NEK" AND SEAL SURFACES BEFORE FINAL ASSEMBLY.

10. PRIMARY LEADS NOT TO EXCEED 24" CPEP OR HDPEP WITH INCLUDED ANGLE BETWEEN LEADS GREATER THAN OR EQUAL TO 135°, OR PRIMARY LEADS NOT TO EXCEED 18" CPEP OR HDPEP WITH INCLUDED ANGLE LESS THAN 135°.

11. A TYPE I MANHOLE SHALL NOT BE USED WHEN BOTH CATCH BASIN AND ACCESS FUNCTIONS ARE REQUIRED.
NOTES:

1. USE STANDARD DETAIL 55–4, NOTE 1
2. USE STANDARD DETAIL 55–4, NOTE 2
3. USE STANDARD DETAIL 55–4, NOTE 3
4. USE STANDARD DETAIL 55–4, NOTE 4
5. USE STANDARD DETAIL 55–4, NOTE 5
6. USE STANDARD DETAIL 55–4, NOTE 6
7. USE STANDARD DETAIL 55–4, NOTE 7
8. USE STANDARD DETAIL 55–4, NOTE 8
9. USE STANDARD DETAIL 55–4, NOTE 9
10. PRIMARY LEADS NOT TO EXCEED TWO 36" CPEP OR HDPEP WITH INCLUDED ANGLE BETWEEN LEADS GREATER THAN OR EQUAL TO 135°.
11. USE THE TWO HOLE PRECAST REDUCING SLAB IDENTIFIED IN STANDARD DETAIL 55–13 WHEN ACCESS AND CATCH BASIN FUNCTIONALITY IS REQUIRED.
**NOTES:**

1. USE STANDARD DETAIL 55-4, NOTE 1
2. USE STANDARD DETAIL 55-4, NOTE 2
3. USE STANDARD DETAIL 55-4, NOTE 3
4. USE STANDARD DETAIL 55-4, NOTE 4
5. USE STANDARD DETAIL 55-4, NOTE 5
6. USE STANDARD DETAIL 55-4, NOTE 6
7. USE STANDARD DETAIL 55-4, NOTE 7
8. STEEL REQ'D FOR BARREL SHALL CONFORM TO A.S.T.M. C-478.
9. USE STANDARD DETAIL 55-4, NOTE 9
10. CONE CANNOT REDUCE TO LESS THAN 72" WHEN BOTH CATCH BASIN AND ACCESS FUNCTIONS ARE REQUIRED. SEE STANDARD DETAILS 55-4 AND 55-5.

**LEGEND**

<table>
<thead>
<tr>
<th>MH I.D.</th>
<th>96&quot;</th>
<th>120&quot;</th>
<th>144&quot;</th>
</tr>
</thead>
<tbody>
<tr>
<td>&quot;A&quot;</td>
<td>96&quot;</td>
<td>120&quot;</td>
<td>144&quot;</td>
</tr>
<tr>
<td>&quot;B&quot;</td>
<td>12&quot;</td>
<td>16&quot;</td>
<td>16&quot;</td>
</tr>
<tr>
<td>&quot;C&quot;</td>
<td>12&quot;</td>
<td>14&quot;</td>
<td>14&quot;</td>
</tr>
<tr>
<td>&quot;D&quot;</td>
<td>8&quot;</td>
<td>10&quot;</td>
<td>12&quot;</td>
</tr>
<tr>
<td>&quot;E&quot;</td>
<td>76&quot;</td>
<td>72&quot;</td>
<td>56&quot;</td>
</tr>
</tbody>
</table>

**STORM DRAIN MANHOLE TYPE III**
SECTION AA

2000 P.S.I. STRENGTH REQUIREMENT
FOR TRANSVERSE BREAKING LOAD
PER A.S.T.M. A-438

STORM DRAIN
TOP INTAKE COVER
### LOCATION

<table>
<thead>
<tr>
<th>Description</th>
<th>E</th>
<th>F</th>
</tr>
</thead>
<tbody>
<tr>
<td>BACKYARDS, GRAVEL STREETS, AND ALLEY AREAS WHERE TRAVELED.</td>
<td></td>
<td>6” TO 12”</td>
</tr>
<tr>
<td>UNDEVELOPED AND SWAMPY AREAS</td>
<td></td>
<td>24” MIN</td>
</tr>
<tr>
<td>HIGHWAY R.O.W.’S OUTSIDE TRAFFIC AREAS</td>
<td></td>
<td>6”</td>
</tr>
<tr>
<td>PAVED STREETS (FEATHER PAVEMENT AT EDGE TO SMOOTH TRANSITION)</td>
<td></td>
<td>1/2”±1/4”</td>
</tr>
</tbody>
</table>
8" THICK SLAB WITH REINFORCEMENT PLACED 2" CLEAR OF THE BOTTOM FACE.
GROOVE FOR 48" MANHOLE SECTION

#5 BARS

6" TYPICAL

48" HOLE

2" TYPICAL

8" THICK SLAB WITH REINFORCEMENT PLACED 2" CLEAR OF THE BOTTOM FACE.
(3) BARS EA SIDE, TYPICAL WITH STANDARD 180° HOOK EACH END.

#5 BARS @ 6" OC EACH WAY, BOT.

25 1/2" HOLE

84" OD

6" CLR

HOOP BARS T&B

#4x80" Ø

POLYPROPYLENE LADDER RUNG (SEE STANDARD DETAIL 50-6)

#5x29½" Ø

#5 BARS PER PLAN

1 1/2"

#5x74" Ø

WWF

12"

PRECAST CONCRETE TWO HOLE REDUCING SLAB (72" TO TWO 25 1/2")
(3) BARS WITH STANDARD 180° HOOK EACH END, TYPICAL 3 LOCATIONS

140" OD

HOOP BARS T&B

#5 BARS @ 6" OC EACH WAY, BOT.

#5x136"φ

6x6 W2.9xW2.9 WWF

14"

1 1/2"

#5 BARS PER PLAN

#5x76"φ

PRECAST CONCRETE REDUCING SLAB (140" TO 72")

MUNICIPALITY
OF ANCHORAGE

SCALE: NTS
APPROVED:
REVISED: 11/08

SECTION # 55.05
DETAIL # 55-15
PRECAST
CONCRETE REDUCING SLAB
(168’ TO 72’)

#5 BARS @ 6” OC EACH WAY, BOT.

(3) BARS WITH STANDARD 180° HOOK EACH END, TYPICAL 3 LOCATIONS

#5x162”ø

6x6 W2.9xW2.9 WWF

#5 BARS PER PLAN

#5x76”ø
FINISH GRADE

1/2"±1/4" (SEE NOTE 4)

EXISTING CONE

18" MAX.

3" MIN.

BARREL OF
EXISTING M.H.

EXISTING CONE

PROVIDE STEP FOR
EVERY 12" OF
ADDED HEIGHT.
SEE STANDARD
DETAIL 50–6

REMOVE CONE & ADD
OR REMOVE PRECAST
RISER SECTIONS OR
RADIAL CONCRETE M.H.
BLOCKS AS NEEDED.

NOTES:
1. RESET CONCRETE GRADE RING IN BEDDING MATERIAL AS SPECIFIED IN SECTION 55.05, ARTICLE 5.2.B — REINFORCED CONCRETE MANHOLES.
2. REFER TO ASTM DESIGNATION C-478 FOR DESIGN AND STRENGTH REQUIREMENTS.
3. RESET CONE IN RAM–NEK OR EQUAL.
4. ADJUST FRAME TO A DEPTH OF 1/2"±1/4" BELOW SURFACE OF PAVEMENT. FEATHER EDGE OF PAVEMENT TO SMOOTH TRANSITION.
NOTES:
1. REFER TO ASTM DESIGNATION C-478 FOR DESIGN AND STRENGTH REQUIREMENTS.
2. WHEN AN ADJUSTMENT OF GREATER THAN 12" IN GRADE RINGS IS REQUIRED, ADJUST CONE I.A.W. STANDARD DETAIL 55-17 RATHER THAN GRADE RINGS.
3. IF NECESSARY, SHIM MANHOLE FRAME WITH STUD WASHERS, TO ADJUST FRAME TO A DEPTH OF 1/2"±1/4" BELOW SURFACE OF PAVEMENT. FEATHER EDGE OF PAVEMENT TO SMOOTH TRANSITION. WHEN SHIMS ARE USED, SET MANHOLE FRAME IN A FULL BED OF MORTAR WITH SHIMS.
(2) 1" HANDLING HOLES ON A 30" DIA. BOLT CIRCLE

(2) 1" CORED WEEP HOLES

(2) 3/4" CORED HOLES FOR BOLTING TO T-BACK

CURB INLET FRAME

SECTION AA

SECTION BB

3/4" RAISED LETTERS (RECESSED FLUSH)
RAISED FISH (RECESSED FLUSH)

SECTION DD

CURB INLET HOOD

SECTION CC

NOTES
THE NOTES FROM STANDARD DETAIL 55-20 APPLY.

CATCH BASIN INLET FRAME AND HOOD FOR TYPE 1 CURB and GUTTER

MUNICIPALITY OF ANCHORAGE

SCALE: NTS
APPROVED: 10/08
REvised:

SECTION # 55.05,09
DETAIL # 55-19
CATCH BASIN INLET GRATES
FOR TYPE 1 CURB and GUTTER

NOTES:
1. MINIMUM CASTING WEIGHT SHALL BE 400 LBS. FOR CURB INLET FRAME, HOOD & GRATE.
2. CURB INLET HOOD & GRATE SHALL CONFORM TO ASTM A536.
3. GRATE SHALL BE AS SHOWN ON THE DRAWINGS OR SPECIFIED BY THE ENGINEER.
CURB INLET FRAME

SECTION AA

33 3/4" DIA.
25 7/8"

SECTION BB

22"

CURB INLET GRATE

SECTION CC

3/8" RAISED LETTERS (RECESSED FLUSH)

OUTFALL TO STREAM: DUMP NO POLLUTANTS

17 1/4"
24"

5 5/16"

MUNICIPALITY
OF ANCHORAGE

SCALE: NTS
APPROVED:
REVISED: 10/08

CATCH BASIN INLET FOR TYPE 2 CURB and GUTTER

SECTION # 55.09
DETAIL # 55–21
CATCH BASIN INLET
- FOR TYPE I CURB & GUTTER
  SEE STANDARD DETAILS 55-19
  AND 55-20
- FOR TYPE II CURB & GUTTER
  SEE STANDARD DETAIL 55-21

OFFSET REFERENCE POINT AT
TOP BACK & MIDPOINT OF CURB
BOX; MATCH BACK OF CURB

2-6" HIGH PRECAST CONC. GRADE
RING SHALL MEET A.S.T.M. C-478

PAVEMENT

MIN. STEEL REQ'D =
0.12 SQ. IN. PER
LINEAR FOOT

10" PIPE AT 4.0%
MIN. GRADE OR AS
DIRECTED BY THE
ENGINEER

SIDE VIEW

4" MIN. 4" MIN.
48" I.D.

2" COVER

MIN. 4" REBAR AT 12"
INTERVALS BOTH WAYS

#4Ø IN SLAB

2" TYPICAL

#3Ø

6" TYP.

DEPRESS
FLOWLINE 1"

2-6" HIGH
PRE-CAST
CONC. GRADE
RINGS

FRONT VIEW

5'-0"

5'-0"

5'-9"

1"

23"

4" MIN.

3"

2"

4'-0"

5'-0"

NOTES:
1. COMPRRESSIVE STRENGTH OF CONCRETE SHALL BE
MINIMUM 4000 P.S.I., EXCEPT BASE SLAB WHICH
MAY BE 3000 P.S.I. CONNECT BASE & BARREL
WITH CONTINUOUS STEEL.
2. SEE ASTM C-478 FOR DESIGN REQUIREMENTS
AND MINIMUM REINFORCING STEEL REQUIRED.
3. AT CATCH BASIN, DELETE CONCRETE GUTTER PAN,
PAVE TO FACE OF CATCH BASIN INLET.

REPLICATING SLAB
STORM DRAIN CLEANOUT

1. 3000 P.S.I. MINIMUM COMPRESSIVE STRENGTH CONCRETE.

NOTE:

MATCH INSIDE TOP OF PIPE TO INSIDE RING

STORM PIPE (12" MIN) SIZE, TYPE & GAUGE AS SPECIFIED IN DRAWINGS

1- OR 2-PIECE CONNECTING BANDS

CENTER OF CLEANOUT ELL IS AS STATIONED ON THE DRAWINGS

PROVIDE ADAPTORS IF CLEANOUT IS ATTACHED TO PIPE LARGER THAN 12" IN DIAMETER

ORIGINAL GROUND DO NOT OVER EXCAVATE

FINISH GRADE

VARIABLE

EJW V-1610 OR APPROVED EQUAL

4"

8" MIN

2-1/2" MIN

3" MAX

45° ELL

45°
NOTES:
1. 3000 P.S.I. MIN. COMPRESSIVE STRENGTH CONCRETE FOR CRADLE.
2. PIPE SHALL PROTRUDE 2" INTO MANHOLE.
ONE- OR TWO-PIECE CONNECTING BANDS

PIPE SIZE, TYPE AND GAGE AS SPECIFIED IN DRAWINGS

TOP OF CONC. CRADLE

SAME DIA.

MORTAR CONNECTION

FOR 4'-0" MIN. DROP

MANHOLE WALL

DAM LOWER HALF OF PIPE WITH BRICK SET IN MORTAR

45° WYE SOLID BEARING

P.C.C. CRADLE 45° ELBOW

4" MIN. P.C.C. COVER

MANHOLE BASE

90° ELBOW

NOTES:
1. 3000 P.S.I. MIN. COMPRESSIVE STRENGTH CONCRETE FOR CRADLE
2. PIPE SHALL PROTRUDE 2" INTO MANHOLE.
NOTES:
1. FINAL LOCATION OF THE FOOTING DRAIN SERVICE MAY BE ADJUSTED BY THE ENGINEER.
2. BACKFILL WITH TYPE II CLASSIFIED FILL AND BACKFILL WITHIN ROAD PRISM. BACKFILL WITH NATIVE MATERIAL BEHIND CURB.
3. WHEN FOOTING DRAIN CONNECTS DIRECTLY TO A MANHOLE, OMIT THE 22 1/2" BEND AND CONSTRUCT THE INVERT A MINIMUM OF 1' ABOVE THE DOWNSTREAM INVERT.
4. CONNECT TO ON—PROPERTY FOOTING DRAIN, WHEN PRESENT, AT PROPERTY LINE, AND OMIT MARKER POST. CONTRACTOR SHALL ADAPT AND PROVIDE BELL—REDUCER OR COUPLING CONNECTION TO EXISTING FOOTING DRAIN OF WHATEVER PIPE SIZE AND TYPE AND RESOLVE CONNECTION DETAILS WITH PROPERTY OWNER AND THE ENGINEER. CONNECTION TO EXISTING FOOTING DRAIN SHALL BE INCIDENTAL TO THIS PAY ITEM, AND NO ADDITIONAL PAYMENT SHALL BE MADE.
NOTES:
1. CULVERT DIAMETER IS MINIMUM 18" OR AS SPECIFIED IN THE DRAWINGS.
2. CULVERT INVERTS SHALL MATCH BOTTOM OF DITCH PROFILE. CONTRACTOR SHALL GRADE DITCH ON BOTH ENDS OF CULVERT PRIOR TO INSTALLATION TO ENSURE POSITIVE DRAINAGE.
3. DRIVEWAY CULVERTS SHALL HAVE A MINIMUM 12" COVER FROM BOTTOM OF A.C. PAVEMENT TO TOP OF PIPE.
4. CULVERT SHALL BE BEDDED IN MINIMUM 6" CLASS "C" BEDDING MATERIAL. BACKFILL SHALL BE TYPE II-A CLASSIFIED FILL & BACKFILL COMPACTED TO 95% OF MAXIMUM DENSITY. BACKFILL AND BEDDING ARE INCIDENTAL TO COST OF CULVERT INSTALLATION.
5. CULVERT END SECTIONS SHALL BE FLARED AND ARE INCIDENTAL TO CULVERT INSTALLATION.
6. LANDING AREA MAXIMUM SLOPE ±2% RESIDENTIAL DRIVEWAY, 10' MINIMUM.

COMMERCIAL/INDUSTRIAL DRIVEWAY, 20' MINIMUM.
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## FOR WATER SYSTEMS
### DIVISION 60
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SECTION 60.01 GENERAL

Article 1.1 Scope of Work

The Work covered by these Specifications consists of providing all plant, labor, equipment, supplies, material, transportation, handling and storage, and performing all operations necessary to complete the construction of all water facilities that will be distributing water by the Anchorage Water and Wastewater Utility (AWWU). Requirements for earthwork including trench excavation and backfill are specified in Division 20 - Earthwork.

Article 1.2 Applicable Standards

The most recent revision of the following standards are hereby made a part of these Specifications:

- ASTM B88 Specification for Seamless Copper Water Tubing
- ASTM D256 Test Methods for D-C Resistance of Plastics and Electrical Insulating Materials
- ASTM D3035 Specification for Polyethylene (PE) Plastic Pipe (SDR-PR) Based on Controlled Outside Diameter
- ASTM D3261 Specification for Butt Heat Fusion Polyethylene (PE) Plastic Fittings for Polyethylene (PE) Plastic Pipe and Tubing
- ASTM D3350 Specification for Polyethylene Plastic Pipe and Fittings Materials
- AASHTO M45 Sand for Cement Mortar
- AWWA A100 Water Wells
- AWWA C104/ ANSI A21.4 Cement-Mortar Lining for Ductile-Iron Pipe and Fittings for Water
- AWWA C105/ ANSI A21.5 Polyethylene Encasement for Ductile Iron Piping for Water and Other Liquids
- AWWA C110/ ANSI A21.10 Ductile-Iron and Gray-Iron Fittings, 3 in. through 48 in., for Water and Other Liquids
AWWA C111/ ANSI A21.11 Rubber-Gasket Joints for Ductile-Iron and Gray-Iron Pressure Pipe and Fittings

AWWA C115/ ANSI A21.15 Flanged Ductile-Iron Pipe with Threaded Flanges

AWWA C151/ ANSI A21.51 Ductile-Iron Pipe, Centrifugally Cast in Metal Molds or Sand-Lined Molds, for Water or Other Liquids

AWWA C303 Reinforced Concrete Pressure Pipe, Steel Cylinder Type, Pre-Tensioned, for Water and Other Liquids

AWWA C500 Gate Valves for Water and Sewerage Systems

ANSI/ AWWA C502 Dry-Barrel Fire Hydrants

ANSI/ AWWA C504 Rubber-Seated Butterfly Valves

ANSI/ AWWA C600 Installation of Ductile-Iron Water Mains and Their Appurtenances

AWWA C651 Disinfecting Water Mains

ANSI/ AWWA C652 Disinfection of Water Storage Facilities

ANSI/ AWWA C800 Underground Service Line Valves and Fittings

AWWA C901 Standard for Polyethylene (PE) Pressure Pipe and Tubing, 1/2 in. through 3 in., for Water Service

ANSI/ AWWA D100 Welded Steel Tanks for Water Storage

ANSI/ AWWA D102 Coating Steel Water-Storage Tanks

SSPC-SP Steel Structures Painting Council Surface Preparation Specifications

SSPC-PA Steel Structures Painting Council Paint Application Specifications

NSF (Standard 61) Drinking Water System Components – Health Effects
Article 1.3  Survey

Survey shall be performed by the Contractor per Division 65 - Construction Survey.

Article 1.4  Pipe Insulation

Rigid board insulation required for frost protection of water mains and services shall be high density extruded polystyrene, Minimum 60 PSI, equivalent to R-20 per four inch (4") thick insulation. The design engineer may request authorization from the AWWU plan reviewer, prior to plan approval, to substitute expanded polystyrene rigid board insulation if the geotechnical report reflects suitable soil conditions. The designer shall submit analysis and justification demonstrating the insulation will not be exposed to seasonal ground water influxes or a high soil moisture content. Subsequent to AWWU’s review and approval, all drawing references to substitute expanded for extruded insulation shall reflect “Expanded Polystyrene Rigid Board Insulation meeting ASTM D2842”.

Article 1.5  Payment - General

Payment for all Work included in this Division shall be paid for in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described.
SECTION 60.02   FURNISH AND INSTALL PIPE

Article 2.1   General

The Work under this Section consists of the performance of all Work required for furnishing and installing water pipe, fittings, and bolts for ductile iron pipe, conductivity straps and thrust restraint systems. The Contractor shall install piping systems in accordance with these Specifications and manufacturer's recommendations, and in conformity with the lines and grades as shown on the Drawings, unless otherwise approved. The use of pipe containing asbestos materials shall be prohibited. Removal of existing wood stave water line shall be considered incidental to this Section and no separate payment shall be made.

Contractor shall provide seventy-two (72) hours written notice to the Engineer, the Anchorage Fire Department, and affected property owners/businesses prior to anticipated mainline flow interruptions. It shall be the Contractor's responsibility to coordinate "turn-off" and "turn-on" with the Engineer.

Article 2.2   Material

A. Ductile Iron Pipe

Ductile Iron Pipe shall conform to the requirements of AWWA C151, with cement mortar lining conforming to the requirements of AWWA C104/ANSI A24.1. Class 52 pipe shall be used for all pipe unless otherwise specified in the Special Provisions.

Fittings shall be a minimum of 250 pounds pressure rating, mechanical joint or all bell, lined or unlined, either cast iron or ductile iron, unless otherwise required by the Contract Documents. All fittings shall conform to the requirements of AWWA C110/ANSI A21.10. Rubber gasket joints for ductile iron pipe and fittings shall conform to the requirements of AWWA C111/ANSI A21.11.

All bolts and nuts supplied with fittings and appurtenances shall conform to ASTM A307 and A563, Grade A or B. All bolts shall be stamped with the grade marking on the head of the bolt, and shall be “307A” or “307B”. All bolts and nuts shall be factory-coated with a blue fluoropolymer coating for corrosion resistance.

In areas of known high-corrosion conditions, stainless steel bolts and nuts shall be supplied with fittings and appurtenances. Stainless steel bolts and nuts shall be Type 316 stainless steel with a minimum tensile strength of 75,000 PSI and shall conform to ASTM F593 and F594. All bolts shall be stamped with the grade marking on the head of the bolt, and shall be “T-316”, “316”, or “F-593”.

B. Concrete Cylinder Pipe

Concrete Cylinder Pipe shall conform to the requirements of AWWA C303 and as otherwise required by the Contract Documents.
C. Copper Service Pipe

Pipe used under this Specification shall be soft-drawn, seamless, annealed copper pipe, suitable for use as underground service water connections for general plumbing purposes, and shall comply with the requirements of ASTM B88 for Type K soft copper as manufactured by the American Brass Company, or equal.

D. Continuity Straps

Continuity straps shall be stranded or solid, rubber or plastic coated, Number 2 copper wire.

E. Thrust Restraint System

Unless otherwise detailed on the Drawings, ductile iron pipe joints shall be push-on rubber gasket types conforming to AWWA C111. Where specified on the Drawings and/or required in these Specifications, ductile iron pipe shall be installed with restrained-joint thrust restraint systems. Ductile iron thrust restraint systems shall be EBAA Iron MEGALUG®, Romac Industries RomaGrip, or U.S. Pipe Field LOK® Gasket. If the ductile iron pipe restrained-joint system proposed for use in the Work is not one of these systems, the Contractor must field demonstrate to the Engineer the installation and/or construction of each new restrained-joint or restraining system. The Contractor shall provide AWWU with a minimum of forty-eight (48) hours notice, excluding non-working days, to coordinate the review of the field demonstration. The Engineer will ask for verification that the restrained-joint system is installed in accordance with the jointing system manufacturer’s written instructions. If, in the opinion of the Engineer, the Contractor fails to install the restrained-joint system in accordance with the jointing system manufacturer’s recommendations, the Contractor shall remove the disapproved system and replace with a new restrained-joint system. The Contractor shall provide access to the field demonstration location and all trench excavation, dewatering and backfill operations prior to, during, and after the restrained-joint system is reviewed by the Engineer. The cost for coordinating and providing access for review of the Contractor’s installation and/or construction of the restrained-joint system shall be incidental to the bid item under construction.

All restrained-joint ductile iron installation areas shall include joints, fittings, and piping deflection points.

The Contractor shall provide pipe manufacturer submittals, which include thrust restraint calculations prior to construction.

F. High Density Polyethylene Pipe

The pipe and fitting material shall have a cell classification of 445574C in accordance with ASTM D3350. In addition, the material must exceed 1,000 hours when tested in accordance with the Ring Environmental Stress Crack Resistance Test (Radar Ring Test) with fewer than twenty percent (20%) failures. Also, the
extruded pipe shall have impact strengths greater than fifteen (15) foot-pounds per inch at thirty-two degrees Fahrenheit (32°F) when tested in accordance with the ASTM D 256 Charpie Impact Test. The material shall be listed by the NSF for potable water service and shall be precompounded. In-plant blending shall not be allowed.

The pipe shall be homogeneous throughout and free of visible cracks, holes, foreign inclusions or other injurious defects. It shall be uniform in color, opacity, density and other physical properties.

Butt fusion of the pipe and fittings shall be performed in accordance with the pipe manufacturer’s recommendations as to equipment and technique. The fusion operation shall be performed by an individual who has demonstrated the ability to fuse polyethylene pipe in the manner recommended by the pipe supplier. The pipe supplier shall supply a representative to instruct the Contractor's crew on butt fusion and installation and witness the first twenty joints.

G. Material Limitations

Copper and ductile-iron pipe are the only pipe materials allowed on water service connections. Galvanized pipe, asbestos-cement pipe and the use of lead-tipped gaskets shall be prohibited.

Article 2.3 Construction

A. Excavation and Backfill

The Contractor shall provide all excavation, backfill, and compaction necessary to install pipe in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

B. Materials Delivery

Pipe and appurtenances shall be handled in such a manner to ensure delivery to the trench in a sound, undamaged condition. Particular care shall be taken not to damage the pipe, pipe coating, or lining. Before installation, the pipe and appurtenances shall be examined by the Engineer for defects.

The pipe shall not be strung out along the shoulders of the road for long distances if it causes inconvenience to the public. The amount of pipe strung at the job site shall be at the discretion of the Engineer.

Rubber gaskets shall be protected from freezing temperatures or direct sunlight.

C. Installation

Installation shall be in accordance with the requirements of ANSI/AWWA C600. The interior of the pipe and accessories shall be thoroughly cleaned of foreign
matter before being lowered into the trench. The pipe shall be kept clean during laying operation by plugging.

Pipe and appurtenances shall be carefully lowered into the trench by means of derrick, ropes, belt slings, or other suitable equipment. Under no circumstances shall any of the pipe or appurtenances be dropped or dumped into the trench. Care shall be taken to avoid abrasion of the pipe coating. Poles used as levers or skids shall be of wood and shall have broad, flat faces to prevent damage to the pipe and coating.

The trench bottom shall be graded to provide uniform support for the pipe barrel. Water shall be kept out of the trench by pumping, if necessary, until the jointing is completed. When Work is not in progress, open ends of the pipe, fittings, and valves shall be securely plugged so that no trench water, earth or other substances will enter the pipes or fittings. Where any part of the coating or lining is damaged, the repair shall be made by the Contractor at his expense and in a manner satisfactory to the Engineer. At a sufficient distance, prior to encountering a known obstacle or tie-in to an existing pipe, the Contractor shall expose and verify the exact location of the obstacle or pipe so that proper alignment and/or grade may be determined before the pipe sections are laid in the trench and backfilled. The connections shall be made by using specials and/or fittings to suit actual conditions. All connections larger than two inch (2”) diameter made under pressure shall be made by AWWU forces.

Pipe ends left for future connections shall be plugged or capped, and restrained, as shown on the Drawings or as directed by the Engineer. The Contractor shall install vertically an eight foot (8’) wood post, directly over the end of pipe.

Cutting of pipe shall be done in a neat and workmanlike manner without damage to the pipe.

Thrust restraint systems of the type shown on the Standard Details shall be installed where the pipeline terminates or changes alignment, utilizing a tee, cross, bend, or similar fitting.

All ductile iron pipe, fittings, valve boxes, and hydrants shall be encased in one layer of polyethylene encasement in accordance with Section 60.07 - Polyethylene Encasement.

Water mains and services shall be constructed to meet all separation requirements of 18 AAC 80.020. Variance from the separation requirements requires a waiver from the Alaska Department of Environmental Conservation. The Contractor shall stagger the joints for the water pipe such that no joint shall be closer than nine feet (9’) from the centerline crossing of water and sanitary sewer pipes. In addition, where water and sanitary sewer or storm sewer mains and services intersect, the vertical separation between the water and pipelines shall be eighteen inches (18”) minimum between exterior pipe surfaces.
D. Alignment and Grade

Contractor shall lay the pipe in the trench so that after the line is completed, the bottom of the pipe conforms accurately to the grades and alignment given by the Engineer. A maximum two-tenths foot \((2/10\text{'} \text{ or } 0.2')\) deviation from design elevation and alignment will be allowed. The pipe shall be generally straight to visual observation as determined by the Engineer.

The Contractor shall check both line and grade and record measurements in a field book for each piece of pipe and appurtenance laid. The Contractor shall have instruments such as a transit and level for transferring alignment and grades from offset hubs. He also shall have in his employ a person who is qualified to use such instruments and who shall have the responsibility of placing and maintaining such construction guides. The Contractor will furnish to the Engineer a copy of the surveyor’s notes for the newly installed pipe and appurtenances. The practice of placing backfill over a section of pipe to provide a platform for instruments shall be subject to the approval of the Engineer and shall be accomplished in accordance with Division 20, Section 20.13, Article 13.3 - Construction.

All adjustments to line and grade shall be done by scraping away or filling the earth under the body of the pipe and not by blocking or wedging up. Deflections from a straight line or grade, as required by vertical curves, horizontal curves, or off-sets shall not exceed eighty percent \((80\%)\) of the manufacturer’s recommendations.

If the alignment requires deflection in excess of the above limitations, the Contractor shall furnish special bends to provide angular deflections within the limits allowable. Short radius curves and closures shall be formed by shorter lengths of pipe, bevels, or fabricated specials.

E. Jointing of Metal Pipe

The Contractor has the option of using either mechanical or push-on joints. All joints shall conform to the requirements of ANSI/AWWA C600.

The Contractor will be required to use mechanically restrained joints and fittings on all hydrant leads. The Engineer has the option of checking any or all mechanical joints to assure proper torque as specified by the manufacturer.

Two electrical continuity straps shall be installed on each side of every joint for pipes less than twelve inches \((12')\) in diameter. Straps are to be welded to a clean, dry surface. All welds and uncoated surfaces are to be coated with coal tar pitch to the satisfaction of the Engineer.

Whenever flange connections are shown on the Drawings, called for in the Specifications, or required in the Work, the flange and fittings shall conform to the requirements of AWWA C110/ANSI A21.10 for two hundred fifty pound \((250\#)\) pressure ratings.
Contractor shall field demonstrate to the Engineer the installation and/or construction of each new restrained joint or restraining system. Contractor shall provide AWWU with a minimum of 48 hours notice, excluding non-working days, to coordinate the review of the field demonstration. The Contractor shall certify that the restrained joint system is installed in accordance with the manufacturer's instructions. If Contractor fails to install the restrained joint system in accordance with manufacturer's instructions, in the opinion of the Engineer, Contractor shall remove the disapproved system and replace with a new restrained joint system.

Contractor shall be responsible for access to the field demonstration location and all trench excavation, dewatering, and backfill operations prior to, during, and after the restrained joint system is reviewed by the Engineer. The cost for coordinating and providing access for review of Contractor's installation and/or construction of the restrained joint system shall be incidental to the bid item under construction.

**Article 2.4 Flushing and Testing**

Prior to any tests performed, all newly installed water facilities, including fire lines, shall be open-bore flushed. Water and sewer main and service trenches shall be substantially filled and compacted. The Contractor, at his option, shall perform the disinfection, hydrostatic testing and continuity testing in any order of sequence. The Contractor is made aware that in the event repairs are made on the system in order to pass the hydrostatic test, and these repairs are made subsequent to disinfection of the system, then the open-bore flush and the disinfection will be null and void and shall be repeated to the satisfaction of the Engineer after the repairs are made. Disinfection will not be allowed until all open-bore flush pipes are removed and the water system is sealed. Costs for repeat disinfection and flushing shall be incidental to the bid item for Furnish and Install Water Main.

Continuity tests will not be performed until all excavation has been completed and backfilled. AWWU's representative must be present for all testing and flushing.

A request to supply water for flushing, testing, and disinfecting shall be scheduled in writing with the Engineer at least 24 hours prior to obtaining AWWU-supplied water. The request for flushing, testing, and disinfecting will be subject to water availability. In the event of high water demand or low water availability within the AWWU water system, meeting Contractor's schedule may not be possible.

Contractor shall submit, in writing, for the Engineer to review and approve, a schedule and procedure for the testing and flushing of all newly installed pipe. When, in the opinion of the Engineer, the testing and flushing schedule and procedure is deficient, inadequate, improper, or conditions are such that the impact to existing water service areas are adversely affected by service interruptions, Contractor will be notified in writing by the Engineer. Such notification shall be accompanied by a statement of the corrective action to be taken. Contractor shall adhere to the testing and flushing schedule and comply with such instruction as directed by the Engineer.
A. Flushing

All newly installed water facilities shall be open-bore flushed to remove any foreign matter. Open-bore flushing shall be accomplished prior to hydrostatic testing and disinfection at each extremity of the main, including all stub-outs and dead ends. The Contractor shall furnish, install and remove all fittings and pipes necessary to perform the flushing, at no additional cost to the Municipality. Under no circumstances will open-bore flushing through hydrants or reduced outlets be permitted. Fire hydrant auxiliary valve shall be closed during flushing.

The Contractor shall notify the Engineer and AWWU forty-eight (48) hours in advance of any flushing operations. The Contractor shall provide a plan for approval by the Engineer for the disposal of the discharge waters from the open-bore flush for seven (7) days. The governing authority shall approve the discharge location and a discharge permit is required. Flushing of newly-constructed mains may be required between the hours of 1:00 a.m. and 6:00 a.m., depending upon the availability of water, as authorized by AWWU. The Municipality will not be responsible for any cost incurred by the Contractor for flushing.

B. Hydrostatic Testing

A hydrostatic test (Pressure Test) will be conducted on all newly constructed water mains, fire hydrant leads and stub-outs after open-bore flushing in the presence of an AWWU representative in accordance with the requirements of ANSI/AWWA C600 unless hereinafter modified. The Contractor shall furnish all necessary assistance, equipment, labor, materials, and supplies (except the test pressure gauge) necessary to complete the test to the satisfaction of the Engineer. The Contractor shall suitably valve-off or plug the outlet to the existing or previously-tested water main at his expense, prior to making the required hydrostatic test. Prior to testing, all air shall be expelled from the pipe. If permanent air vents are not located at all high points, the Contractor shall, at his expense, install corporation cocks at such points so the air can be expelled as the line is slowly filled with water.

All main valves, fire hydrant auxiliary valves, fire hydrant main valves, and plugs shall be tested. All intermediate valves within the section being tested will be closed and reopened as directed by the Engineer during the actual test. Only static pressure will be allowed on the opposite side of the end valves of the section being tested.

All hydrostatic testing will be performed through test copper. Use of fire hydrant and service connections for testing will not be allowed.

The hydrostatic pressure shall be one hundred fifty (150) psi. The duration of each hydrostatic pressure test shall be thirty (30) minutes. After the required test pressure has been reached, pumping will be terminated. If the pressure remains
constant for 30 minutes without the aid of a pump, the results of the test shall be considered satisfactory as approved by the Engineer.

The Contractor shall notify the Engineer forty-eight (48) hours, (two (2) working days) prior to any test and shall notify the Engineer two (2) hours in advance of the scheduled time if the test is to be canceled. In the event the Engineer has not been notified of cancellation and the Contractor is not prepared for the test as scheduled, the Contractor shall reimburse the Engineer for all expenses incurred. These will include, but not be limited to, salaries, transportation and administrative costs.

C. Disinfection

Chlorine shall be used for disinfection per the current edition of ANSI/AWWA C-651. Chlorine shall be applied by one of the following methods: (1) liquid chlorine gas-water mixture, (2) direct chlorine gas feed, or (3) calcium hypochlorite and water mixture. Calcium hypochlorite shall be comparable to commercial products known as HTH, Perchloren or Machochlor. The chlorinating agent shall be applied at the beginning of the section through the test copper pipe adjacent to the feeder connection, insuring treatment of the entire line. Under no conditions shall the chlorinating agent be introduced through a fire hydrant. Water shall be fed slowly into the new line with chlorine applied in amounts to produce a dosage a minimum of twenty-five parts per million (25 ppm). Application of the chlorine solution shall continue until the required dosage is evident at all extremities of the newly laid line. Contractor may submit alternate disinfection methods to the Engineer for review.

Calcium hypochlorite shall be mixed into a solution of water and injected or pumped into the water main. During the chlorination process, all intermediate valves and accessories shall be operated. Valves shall be manipulated so that the strong chlorine solution in the line being treated will not flow back into the line supplying the water. Hydrostatic testing of a water line containing the chlorine mixture will not be allowed.

A residual of not less than ten parts per million (10 ppm) chlorine shall be retained in all parts of the water main after twenty-four (24) hours. After which this residual shall be flushed from the line at its extremities until the replacement water tests are equal chemically and bacteriologically to those of the permanent source of supply. The Contractor shall provide a plan for approval by the Engineer for disposal of chlorinated waters from the disinfection of the system. The governing authority shall approve the de-chlorination discharge method and location. In no instance shall a water main be chlorinated before open-bore flushing.
CHLORINATION

<table>
<thead>
<tr>
<th>Pipe Diameter (ID)</th>
<th>Dosage (oz.) per 100 feet</th>
</tr>
</thead>
<tbody>
<tr>
<td>4&quot;</td>
<td>.34</td>
</tr>
<tr>
<td>6&quot;</td>
<td>.76</td>
</tr>
<tr>
<td>8&quot;</td>
<td>1.34</td>
</tr>
<tr>
<td>10&quot;</td>
<td>2.10</td>
</tr>
<tr>
<td>12&quot;</td>
<td>3.02</td>
</tr>
<tr>
<td>14&quot; and larger</td>
<td>D^2 x 3.02</td>
</tr>
</tbody>
</table>

1. D is the inside diameter in feet.
2. One Heaping Tablespoon ≅ ½ oz.

The above table is to be used as a guide for chlorinating water mains by the calcium hypochlorite and water mixture method. The Contractor shall use a dosage per one hundred feet (100') that results in a minimum chlorine solution of twenty-five parts per million (25 ppm).

This dosage takes into account that Contractors most frequently used granular HTH, which is sixty-five percent (65%) pure. If another chlorinating agent is used, the dosage must be adjusted. Caution should be exercised against producing too high a concentration of chlorine in the line, as approved by the Engineer.

D. Continuity Tests

The Contractor shall perform electrical conductivity tests on all ductile iron mains in the presence of a representative of the Engineer. Continuity testing shall also be performed on all water service connections and extensions greater than two inches (2") in diameter.

The Contractor shall maintain a circuit of six hundred (600) amperes DC current for a period of fifteen (15) minutes. Input current shall not exceed ten percent (10%) of the return circuit. All equipment necessary to maintain the circuit shall be supplied by the Contractor.

All continuity tests will be through wires connected to the main and brought to the surface. The use of water service thaw wires, fire hydrants and valves as substitutes for wires will not be accepted. All wires brought to the surface to complete the continuity test shall be removed to a depth of two feet (2') below finished street grade upon completion of the tests.
E. Test and Air Vent Copper Pipe Removal

The Contractor shall, upon acceptance of testing, remove all test and air vent copper pipe and close the corporation stop at the main with a copper disc and flare nut installed, in the presence of the Engineer.

Article 2.5 Measurement

Measurement for furnishing and installing water main shall be per linear foot of horizontal distance of the various sizes as set forth in the Bid Schedule. Measurement will be from station to station as staked in the field and as shown on the Drawings, except where the grade exceeds twenty-five percent (25%), in which case measurement will be by actual pipe length.

Article 2.6 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Unless specifically identified for payment under a separate pay item, the unit price bid to Furnish and Install (size) (type) Water Main shall include all labor, equipment and materials to furnish and install a functional potable water main including, but not limited to, the following incidental items: delivery of non-serviceable portions of removed pipe, valves, and fittings at a Contractor-furnished disposal site; delivery of serviceable portions of removed pipe, valves, and fittings to the Owner, when directed by the Engineer; installation of all pipe, tees, crosses, bends, caps, plugs, adapters, reducers, thrust restraint systems, and other fittings; installation of thrust blocks; adjustment to finish grade; cleaning and flushing; hydrostatic testing; provisions coordinating the supply of water as required for flushing and hydrostatic testing; disinfecting; continuity testing; protection and/or restoration of all existing utilities; maintenance of existing water distribution system flows; shoring and/or protection of existing light poles; maintenance and restoration of existing drainage patterns; restoration of existing driveways; signage, mail boxes, newspaper boxes, trees and shrubs located on private property; landscaping, utility markers, survey monumentation; removal and replacement of miscellaneous public or private improvements; preparation of off-roadway areas for topsoil and re-seeding; cleanup, and miscellaneous items required to complete the Work as shown on the Drawings.

Where the Work includes disconnecting existing water services from and existing water main and reconnecting them to a new water main, the disconnection and reconnection of those existing water services will be considered incidental to the price bid for installation of the new water main.

Trench excavation and backfill shall be paid for under Division 20, Section 20.13 - Trench Excavation and Backfill.
Payment shall be made on the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install (Size) (Type) Water Main</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 60.03  FURNISH AND INSTALL VALVES

Article 3.1  General

The Work under this Section consists of the performance of all Work required for furnishing and installing valves, including valve boxes and marker posts.

Article 3.2  Material

A.  Gate Valves

Gate valves shall be iron body, fully bronze mounted, double disc, parallel or resilient seat valves as manufactured in accordance with the requirements of AWWA C500 "Gate Valves for Water and Sewer Systems." All valves shall be nonrising stem type with an O-ring seal and a two inch (2”) square operating nut, and shall open counterclockwise. Valves shall be mechanical joint ends.

Gate valve bonnet bolts shall be Type 316 stainless steel with a minimum tensile strength of 75,000 PSI and shall conform to ASTM F593 and F594. All bolts shall be stamped with the grade marking on the head of the bolt, and shall be “T-316”, “316”, or “F593”.

B.  Butterfly Valves

Butterfly valves shall be of the rubber-seated tight-closing type. They shall meet or exceed the performance requirements of AWWA C504 for operational pressures of 150 psi working pressure and 300 psi hydrostatic pressure.

Mechanical joint valve ends shall be per AWWA C110/ANSI 21.10 and AWWA C111/ANSI 21.11 of the latest revision, and “Short-Body” in accordance with the requirements of Table 2 of ANSI/AWWA C504. Accessories (bolts, glands, and gaskets) shall be supplied by the valve manufacturer.

Butterfly valve actuator bolts that are exposed shall be Type 316 stainless steel with a minimum tensile strength of 75,000 PSI and shall conform to ASTM F593 and F594. All bolts shall be stamped with the grade marking on the head of the bolt, and shall be “T-316”, “316”, or “F593”.

Valves must use full ANSI/AWWA C504 Class 150 B valve shaft diameter and full Class 150 B underground service operator torque rating throughout entire travel to provide capability for operation in emergency service.

Valve body shall be high-strength cast iron ASTM A126 Class B. For valves with the rubber seat mounted on the disc, the mating surface in the body shall be 304 or 316 steel. For valves containing the rubber seat in the body, the method of seat retention shall be in accordance with the requirements of ANSI/AWWA C504, except that no retaining fasteners or other hardware shall be permitted in the flow stream.
Valve operators, unless otherwise required by the Contract Documents, shall be of the traveling nut type, sealed, gasketed, and lubricated for underground service and capable of withstanding on overload input torque of four hundred fifty (450) foot-pounds at full open or closed position without damage to the valve or valve operator. The number of turns to operate the valve shall be a minimum of two (2) turns per inch of valve diameter for ninety degrees (90°) of closure travel at a maximum pull of eighty (80) pounds. All valves shall open counterclockwise and be equipped with two inch (2") square AWWA operating nut.

For butterfly valves twenty inches (20") and less, the valve shaft shall be one piece extending full size through valve bearings, disc and shaft seal. In the event that the shaft is turned down to fit connections to the operator, the limits of ANSI/AWWA C504, Section 3.3.2 shall be strictly observed. Carbon steel shafts, if used, shall have 304 or 316 stainless steel journals with static seals to isolate the interior of the disc and the shaft from the water.

For butterfly valves over twenty inches (20"), the valve shaft shall be of two-piece stub shaft type, made of 18-8 Type 304 stainless steel. Valve bearings and shaft seals for valves of all sizes shall meet the requirements of ANSI/AWWA C504 Section 3.6 and 3.7 respectively, with the following additional requirements:

1. Sleeve bearings shall have a maximum coefficient of friction of 0.1.
2. For underground service, packing shall be pressure-energized chevron or "O" ring type, not requiring adjustment and suitable for permanent duty.

C. Pressure Reducing Valves

Pressure reducing valves shall be supplied as directed in the Contract Documents.

D. Valve Boxes

Valve boxes shall be cast iron of sliding, adjustable height type with round or oval bottom hood sections to fit over the top of the valve. The top section shall be recessed to receive a close fitting "eared" lid with the word "water" cast into it. Internal diameter of the smallest section shall not be less than five inches (5"). Minimum thickness of the metal shall not be less than five-sixteenth inch (5/16"). Castings shall be smooth and the workmanship shall be acceptable to the Engineer.

Valve boxes shall be of sufficient length (ten foot [10'] sections) for the pipe cover depth on the profile drawings and in accordance with the Standard Details.

E. Markers

Valve boxes shall be marked with markers consisting of two and one-half inch (2.5") O.D. galvanized steel pipe sections, seven feet (7') in length, with three feet (3')
buried in the ground. Markers shall be shop painted "Caterpillar Yellow" and painted with stenciled two inch (2") black numerals, showing the appropriate references. Markers shall be located on the nearest property line, due north, south, east or west of the valve at a maximum distance of fifty feet (50'), unless otherwise directed by the Engineer. Markers shall not be required where valve boxes are located in paved areas. Markers shall carry the following notation:

VB

(feet) (direction)

E. Live Tap Connections

Contractor shall provide all trench excavation, backfill, and compaction necessary to assist AWWU with the live tap connections. Excavation for live tap connections shall be unclassified and Contractor shall excavate substances encountered to the depth required for the live tap connections. Variations from the depth indicated in the Drawings will not be grounds for additional compensation. It shall be Contractor's responsibility to familiarize himself with the depth of water mains for the project. Contractor shall excavate for live tap connections in such a manner that the excavation is 90° to the main water line, whenever possible. The trench shall be long enough and of sufficient width at the bottom to allow installation of the valve for the live tap connection and provide safety for AWWU Operations personnel.

Contractor shall be responsible for, and shall bear the expenses incurred, if a water main should be damaged during excavation or backfilling. AWWU will repair all damaged mains; however, Contractor shall bear the cost of all material, labor, and other expenses.

Contractor shall provide assistance, equipment, labor, materials, and supplies (except the water main line valve) necessary to complete the live tap connection. Contractor shall notify the Engineer and AWWU 48 hours (two working days) prior to installation of the live tap connection. In addition, Contractor shall obtain all necessary permits for the live tap connection and pay all associated fees.

Tie back rods and/or tie back rod and shackle assemblies will not be acceptable as restrained joints or restraining system for valves and valve/pipe joint interface.

Unless otherwise detailed on the Drawings, valve and valve/pipe interface shall be push-on rubber gasket type conforming to AWWA C111. Where specified on the Drawings, restrained joint pipe shall be EBAA Iron MEGALUG®, Romac Industries RomaGrip, or approved equal.

Contractor shall provide pipe manufacturer submittals which include thrust restraint calculations prior to construction.
Article 3.3 Construction

The Contractor shall provide all trench excavation, backfill, and compaction necessary to install valves in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

Valves or valve boxes shall be installed where shown on the Drawings. Valve box components shall be plumb and centered over the operating nut. The valve operator shall be placed on the side of the water main away from the centerline of the street or easement. On fire line installations, a valve shall be placed outside the building so that all fire hydrants will remain in service in the event water service to the building must be shut off for any reason.

Valves shall have the interiors cleaned of all foreign matter before installation. If the valve is at the end of the line, it shall be plugged prior to backfilling. The valve shall be inspected by the Engineer in the open and closed positions to ensure that all parts are in working condition.

Provisions shall be made to prevent soil infiltration into the valve box. Wrap burlap inside bottom section under the packing gland and wrap three (3) layers of non-woven geotextile fabric around the outside of the valve and base section of the valve box and secure the fabric at the top and bottom with tape. Encase the valve box with eight-mil polyethylene, encasement, taped securely in place.

The Contractor shall expose all valve boxes for prefinal and final inspection. After final inspection of the valves located in unpaved areas, sawdust shall be poured directly over the valve box lid and covered with gravel to facilitate location in the future.

Article 3.4 Measurement

The quantity to be paid shall be the actual number of valves of each class and size (including valve boxes and marker posts) furnished, installed and accepted.

Article 3.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install (Size) Gate Valve</td>
<td>Each</td>
</tr>
<tr>
<td>Furnish and Install (Size) Butterfly Valve</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.04 FURNISH AND INSTALL FIRE HYDRANTS

Article 4.1 General

The Work under this Section consists of the performance of all Work required for the furnishing and installation of "L-Base" Fire Hydrant Assemblies, including the fire hydrant leg pipe, auxiliary gate valve, valve box, joint restraint, guard rails, and fire hydrants.

Article 4.2 Materials

A. Fire Hydrants

Fire hydrants shall conform to the requirements of ANSI/AWWA C502 for Dry Barrel Fire Hydrants. Fire hydrants shall be Mueller Centurian or equal.

1. All fire hydrants shall be supplied with a five and one-fourth inch (5.25") main valve opening.

2. All single pumper hydrants shall be furnished with a six inch (6") ANSI Class 125 standard mechanical-joint end. All double pumper hydrants shall be furnished with an eight inch (8") ANSI Class 125 standard mechanical-joint.

3. All connections shall be mechanical-joint unless otherwise indicated in the Contract Documents.

4. Single pumper hydrants shall be furnished with two (2) two and one-half inch (2.5") hose connections and one (1) four and one-half inch (4.5") pumper connection. Double pumper hydrants shall be furnished with one (1) two and one-half inch (2.5") hose connection and two (2) four and one-half inch (4.5") pumper connections.

5. Unless otherwise required by the Contract Documents, all hydrants shall be furnished with a barrel length that will allow a minimum of ten feet (10') of bury.

6. The main valves shall be of the compression type, where water pressure holds the main valve closed permitting easy maintenance or repair of the entire barrel assembly from above the ground without the need of a water shutoff.

7. All fire hydrants shall be furnished with a breakaway flange which allows both barrel and stem to break clean upon impact from any angle. Traffic flange design must be such that repair and replacement can be accomplished above ground.

8. Painting and coating shall be in accordance with cited AWWA Specifications. After installation, the hydrant section from the traffic flange to the top of the operating nut shall be painted "Caterpillar Yellow."
9. Operating and nozzle nuts shall be pentagon shaped with one and one-half inch (1.5") point to flat measurements.


11. All working parts shall be bronze or noncorrosive metal in accordance with the requirements of ANSI/AWWA C502.

12. All hydrants shall be right hand opening (clockwise).

13. All hydrants shall be non-draining. Drain plugs shall not be removed.

B. Auxiliary Gate Valves

All gate valves and valve boxes shall be furnished and installed in accordance with Section 60.03 - Furnish and Install Valves.

C. Thrust-Restraint System

Unless otherwise detailed on the Drawings, Contractor shall provide push-on rubber gasket type conforming to AWWA C111. Where specified on the Drawings and/or Standard Details, Contractor shall install EBAA Iron MEALUG®, Romac Industries RomaGrip, U.S. Pipe Field LOK® Gasket System, or approved equal, on restrained joint pipe. Contractor shall ensure all restrained-joint installation areas shall include joints, fittings, and piping deflection points.

D. Guard Posts

The Contractor shall install guard posts at each hydrant installation in accordance with the Standard Details. If, in the opinion of the Engineer, the guard posts are not to be installed, they shall be delivered to the Anchorage Water and Wastewater Utility storage yard. Measurement and payment for guard posts shall be incidental to the Bid item "Furnish and Install Fire Hydrant Assembly."

Article 4.3 Construction

The Contractor shall provide all trench excavation, backfill and compaction necessary to install the fire hydrant assembly in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

The Contractor shall install the hydrant assemblies in accordance with the Standard Details.

The fire hydrant auxiliary valve shall be closed during installation and remain closed during all main line open bore flushing operations. The auxiliary gate valve shall be opened for hydrostatic pressure testing and disinfection and while the hydrant is being raised by AWWU under pressure. All fire hydrant legs shall be installed level. The fire hydrant barrel shall be installed plumb. Fire hydrants will be adjusted to final grade by the AWWU.
Operations Division. The Contractor shall provide AWWU with a minimum of seventy-two (72) hours notice, excluding non-working days, to coordinate fire hydrant adjustment. The Contractor shall be responsible for access to the hydrant location and all trench excavation, dewatering and backfill operations prior to, during, and after the fire hydrants are adjusted by AWWU personnel. The cost for coordinating and providing trenching operations are incidental to the fire hydrant installation. Any adjustments to the fire hydrant traffic flange on a Municipal Contract will be made by AWWU at no cost to the Contractor. Adjustment to other fire hydrants will be made by Anchorage Water and Wastewater Utility on a reimbursable basis.

Hydrants installed but not available for use shall be covered with burlap and securely tied.

In lieu of valve box markers for the auxiliary gate valves, the Contractor shall paint in two inch (2") black lettered stencils, the direction and distances to the nearest one-tenth foot (1/10' or 0.1’) the distance to the valve box on the face of the fire hydrant directly below the bonnet flange.

**Article 4.4 Measurement**

The method of measurement to furnish and install fire hydrants shall be as follows:

A. Single Pumper Fire Hydrants

Fire hydrants six inch (6") leg to main, six inch (6") auxiliary gate valve and valve box, guard post installation, and thrust-restraint system shall be paid for at the unit price as set forth in the Bid Schedule. The price shall include full compensation for furnishing and installing single pumper hydrants as shown in the Standard Details.

B. Double Pumper Fire Hydrants

Fire hydrants eight inch (8") leg to main, eight inch (8") auxiliary gate valve and valve box, guard post installation, and thrust-restraint system shall be paid for at the unit prices as set forth in the Bid Schedule. The price shall include full compensation for furnishing and installing double pumper hydrants as shown in the Standard Details.
Article 4.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

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<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install Fire Hydrant Assembly</td>
<td>Each</td>
</tr>
<tr>
<td>(Single Pumper)</td>
<td></td>
</tr>
<tr>
<td>Furnish and Install Fire Hydrant Assembly</td>
<td>Each</td>
</tr>
<tr>
<td>(Double Pumper)</td>
<td></td>
</tr>
</tbody>
</table>
SECTION 60.05   FIRE LINES

Article 5.1   General

The Work required under this Section consists of the performance of all Work required for the furnishing and installation of fire lines including thrust-restraint system, fittings, valves, and valve boxes.

Article 5.2   Material

Refer to Section 60.02, SubArticles 2.2.A – Ductile Iron Pipe, 2.2.D – Copper Service Pipe, and 2.2.E – Thrust Restraint System for material specifications.

Article 5.3   Construction

A.   General

A fire line that originates at a water utility main has the primary purpose of providing fire protection inside a building. No connections, other than those for additional fire protection, will be allowed on the fire line outside the building. Domestic water obtained from a fire line will be connected and metered inside the building.

Valves and valve boxes shall be installed where shown on the Drawings.

B.   Excavation and Backfill

The Contractor shall provide all excavation, backfill, and compaction necessary to install fire lines in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

C.   Materials Delivery

Refer to Section 60.02, SubArticle 2.3.B – Materials Delivery, for material delivery specifications.

D.   Installation

Installation shall be in accordance with Section 60.02, SubArticle 2.3.C – Installation, amended as follows: Thrust-restraint systems shall be installed for a distance of forty feet (40') in both directions from all fittings.

Refer to Section 60.02, SubArticle 2.3.D. - Alignment and Grade, for alignment and grade specifications.
F. Jointing of Metal Pipe

The Contractor has the option of using either mechanical or push-on joints. All joints shall be made in conformance with AWWA C-600. The Contractor will be required to use mechanically restrained joints on all hydrant leads. The Engineer has the option of checking any or all mechanical joints to assure proper torque as specified by the manufacturer.

Butterfly valves shall be used on lines sixteen inches (16") and larger. Refer to Section 60.03, SubArticle 3.2.B. - Butterfly Valves, for specifications.

Two electrical continuity straps shall be installed on each side of every joint for pipes less than twelve inches (12") in diameter. Straps are to be welded to a clean, dry surface. All welds and uncoated surfaces are to be coated with coal tar pitch to the satisfaction of, and as approved by, the Engineer.

Article 5.4 Fire Hydrants and Valve Boxes

Refer to Section 60.04, Article 4.2 - Material for Fire Hydrant Specifications and Section 60.03, Article 3.2 - Material for Valve Box specifications.

Article 5.5 Flushing and Testing

Prior to any tests performed, all newly installed fire lines shall be open-bore flushed. The Contractor, at his option, shall perform the disinfection, hydrostatic, and continuity test in any order of sequence. Hydrostatic testing of a water line containing a chlorine mixture shall not be allowed. Contractor is hereby notified, that in the event repairs are made on the system in order to pass the hydrostatic test, subsequent to completion of system disinfection and flushing, all previous tests, including open-bore flushing, shall be declared void and shall be repeated to the satisfaction of the Engineer. Costs for repeat disinfection and flushing shall be incidental to the price bid for Furnish and Install Fire Line.

A. Flushing

All newly installed fire line shall be open-bore flushed to remove any foreign matter. Open-bore flushing shall be accomplished prior to hydrostatic testing and disinfection at each extremity of the line, including all stubouts and dead ends. The Contractor shall furnish, install, and remove all fittings and pipes necessary to perform the flushing, at no additional cost to the Municipality. Under no circumstances will open-bore flushing through hydrants or reduced outlets be permitted.

It will be the Contractor's responsibility to notify the Engineer and AWWU forty-eight (48) hours in advance of any flushing operations. The Contractor shall provide a plan for approval by the Engineer for the disposal of the discharge waters from the open-bore flush. The governing authority shall approve discharge location. Spent flushing water shall not be discharged to the sanitary sewer system. The governing authority shall approve the discharge location. Flushing of newly constructed mains
may be required between the hours of 1:00 a.m. and 6:00 a.m. depending upon the availability of water as authorized by AWWU. The Municipality will not be responsible for any cost incurred by the Contractor for flushing.

B. Hydrostatic Testing

A hydrostatic test will be conducted on all newly constructed fire lines, fire hydrant leads and stubouts after open-bore flushing in the presence of an AWWU Inspector in accordance with the requirements ANSI/AWWA C600. The Contractor shall furnish all necessary assistance, equipment, labor, materials, and supplies (except the test pressure gauge) necessary to complete the test to the satisfaction of the Engineer.

All main line valves, fire hydrant auxiliary valves, fire hydrant main valves, and plugs shall be tested. All intermediate valves within the section being tested will be closed and re-opened as directed by the AWWU during the actual test. Only static pressure will be allowed on the opposite side of the end valves of the section being tested.

All hydrostatic testing will be performed through test copper or fire line riser in building. Use of fire hydrants and service connections for testing will not be allowed.

All fire lines and stubouts for future fire line connections shall be hydrostatically pressure tested at two hundred pounds per square inch (200 psi) for two (2) hours, in accordance with the Fire Underwriter's requirements as outlined in the National Fire Codes.

If the pressure decreases below the required test pressure during the two (2) hour period, the preceding portion of that test will be declared void. Cracked or defective pipe, gaskets, mechanical joints, fittings, valves, or hydrants discovered as a consequence of the hydrostatic tests shall be removed and replaced with sound material at the Contractor's expense. The test shall then be repeated until the results are satisfactory. Use of leakage tests shall not be allowed.

C. Disinfection

Refer to Section 60.02, Article 2.4 - Flushing and Testing Disinfection for specifications.

D. Continuity Tests

Refer to Section 60.02, SubArticle 2.4.D - Continuity Tests for specifications.

E. Test and Air Vent Copper Removal

Refer to Section 60.02, SubArticle 2.4.E - Test and Air Vent Copper Removal for specifications.
Article 5.6 Measurement

Measurement for furnishing and installing fire lines shall be per linear foot of horizontal distance of the various sizes as set forth in the Bid Schedule. Measurement will be from station to station as staked in the field and as shown on the Drawings, except where the grade exceeds twenty-five (25) percent, in which case measurement will be by actual pipe length.

Article 5.7 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Unless specifically identified for payment under a separate pay item, the unit price bid to Furnish and Install (size) (type) Fire Line shall include all labor, equipment and materials to furnish and install a functional fire line including, but not limited to, the following incidental items: delivery of non-serviceable portions of removed pipe, valves, and fittings at a Contractor-furnished disposal site; delivery of serviceable portions of removed pipe, valves, and fittings to the Owner, when directed by the Engineer; installation of all pipe, tees, crosses, bends, caps, plugs, adapters, reducers, thrust restraint systems, and other fittings; installation of thrust blocks; adjustment to finish grade; cleaning and flushing; hydrostatic testing; disinfecting; continuity testing; protection and/or restoration of all existing utilities; maintenance of existing water distribution system flows; shoring and/or protection of existing drainage patterns; restoration of existing driveways; signage, mail boxes, newspaper boxes, trees and shrubs located on private property; landscaping, utility markers, survey monumentation; removal and replacement of miscellaneous public or private improvements; preparation of off-roadway areas for topsoil and re-seeding; cleanup, and miscellaneous items required to complete the Work as shown on the Drawings.

Excavation and backfill shall be paid for under Division 20, Section 20.13 - Trench Excavation and Backfill.

Payment shall be made on the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install (Size) (Type) Fire Line</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 60.06   WATER SERVICE LINES

Article 6.1   General

The Work under this Section consists of the performance of Work required for furnishing and installing water service lines including fittings, key boxes, and valve boxes.

A service line provides potable water to a building or lot for domestic or commercial use.

A permit shall be purchased from AWWU permit section prior to any and all construction (either on or off property in the AWWU service area).

Twenty-four (24) hours notification shall be given to AWWU Inspector prior to making the connection available for inspection.

Before an on-property service line permit for any new subdivision can be released for construction, all property corners shall be established and identified.

Article 6.2   Material

A.   Pipe

Ductile iron pipe or soft drawn seamless copper Type K shall be used for all service lines.

B.   Key Box, Valve Box

The key box or valve box shall provide a clear and unobstructed access to a curb stop or valve to enable the AWWU operation of the curb stop or valve. Key boxes and valve boxes shall be wrapped with eight mils (8-mils) thick polyethylene encasement. Key boxes or valve boxes shall not be in contact with a gas main. Key boxes or valve boxes shall be installed in the standard location as shown in the Standard Details.

Key boxes shall be of an acceptable construction as outlined in this Article for construction and as shown in the Standard Details for Typical Water Service Connects. Key boxes shall be installed with a standard location marker as defined in the Design Criteria of the Anchorage Water and Wastewater Utility.

Valves shall be of an acceptable construction as outlined in Section 60.03, Article 3.2 - Materials and the Standard Details for Typical Valve Box. Valves shall be installed with a standard marker as defined in Section 60.03, SubArticle 3.2.E - Markers.
Article 6.3 Construction

A. Excavation and Backfill

The Contractor shall provide all excavation, backfill, and compaction necessary to install water source lines in accordance with Division 20, Section 20.13 - Trench Excavation and Backfill.

B. Service Connections

A corporation stop or main valve shall be installed at a point in the service line as close to the main water supply as possible. There shall be line pressure in the main at all times connections are being made. All service lines two inches (2") and smaller shall be constructed of seamless, soft drawn, Type K copper. All service connections larger than two inches (2") shall be made of ductile iron and shall be accomplished by AWWU connection crews. The Contractor may make the connection by special agreement with the AWWU Permits Section. All ductile iron pipe installations shall be flushed, hydrostatic tested, and disinfected as outlined in Section 60.02 - Furnish and Install Pipe.

In the event a Contractor elects to make the connection to an AWWU main water supply, it shall be installed in a manner consistent with the Standard Specifications and Standard Details. A water service line shall not cross property lines of adjoining lots. The key box shall be installed to the edge of the right-of-way or edge of permanent easement of the lot being served, no closer than five feet (5') from adjoining property lines, and shall be marked by means of a Carsonite (or equal) marker extended two feet (2') above grade, painted blue. The connection shall be inspected by an AWWU Inspector at the time the connection is made or the excavation be exposed in its entirety for his inspection. The permit shall be posted and available at the time of inspection.

No unions will be allowed in the right-of-way on newly constructed service lines.

Where water service lines intersect with sanitary sewer pipelines, the water service line shall be located to provide a minimum vertical separation of eighteen inches (18") between the water pipe and sanitary sewer pipeline, with the separation distance measured from outside of water pipeline to outside of sanitary sewer pipeline.

As-built records shall include the pipe station of service connection at the main, service length and distance to the nearest property corner.

C. Excavation

The Contractor shall excavate whatever substances that are encountered to the depth required for the connections. Depth for water service connections will be a minimum of ten feet (10') below proposed finished grade. The ten foot (10') depth below finished grade shall be maintained five feet (5') past the footings, before the
depth shall be less than ten feet (10’). Variations in depth from the depth stated above will not be grounds for additional payment. It shall be the Contractor's responsibility to familiarize himself with the depth of water mains for the project. The portion of the right-of-way that extends from the main to the key box (curb stop) will be excavated in such a manner that will allow the service connection to be installed horizontally (no slope). The Contractor shall excavate for water connections in such a manner that the excavation is ninety degrees (90°) to the street line, whenever possible. Two services shall not be installed in a single trench when separation between keyboxes is greater than twelve feet (12’). The ditch shall be long enough to allow the key box to be set at the property line.

Trenches shall be of sufficient width at the bottom to allow for laying of the particular service (minimum two and one-half feet [2.5'] for single service). Excavation of all fill materials to virgin ground is required to provide safety for workmen utilizing the trench.

The Contractor shall expose the mains to be tapped for distance of four feet (4’) in length. Excavation on both sides of the pipe shall be carried to the bottom of the pipe. Excess excavation below required level shall be backfilled and compacted with sand or gravel at the Contractor's expense as directed by the Engineer.

No water service shall be within a horizontal distance of ten feet (10’) from the sanitary sewer service, footing drain or storm service.

The Contractor shall be responsible for, and shall bear the expenses incurred, in the event that a main should be damaged during excavation or backfilling. The water Utility will repair all damaged mains; however, the Contractor shall bear the cost of all material, labor, and other expenses thereof. If approved by AWWU, the Contractor may repair the damaged main.

All on-property installations shall be constructed to the same standard as off-property installations.

D. Backfill

At such time as the Engineer may direct, but only after the service lines and appurtenances have been properly completed and inspected, the trenches and appurtenant structures shall be backfilled. The backfilled material, free from large clods, frozen material or stones, shall be placed by the Contractor in conformance with the codes and regulations of the Municipality.

The Contractor shall exercise due care in backfilling to keep the service box and thaw wire vertical and in place. In the event the service box or thaw wire is displaced, the Contractor will be required to excavate and restore the service box and thaw wire to the proper position. Any work necessary to restore the service box and thaw wire to the proper position will be performed at the Contractor's expense.
A thaw wire constructed to a #2 copper plastic or rubber coated wire shall be attached to the corporation stop on one inch (1") connections by an approved method. On one and one-half (1 1/2") and two inch (2") connections, the thaw wire shall be attached to the saddle on the main. Three inch (3") through ten inch (10") connections shall have continuity straps attached in the same manner as that of main line installation.

E. Disconnects

If an existing service line is replaced by a new service or becomes unusable due to a replat of the property, it shall be disconnected at the main, at no cost to the Municipality. The disconnect shall be witnessed by an AWWU inspector.

F. Hydrostatic Testing

All newly installed water mains and all new services shall be subject to a hydrostatic pressure test of 150 pounds of pressure. This pressure test may be performed at the same time that the hydrostatic test is performed on the new water main. A bleeder will be installed at each service line key box and extended one foot (1') above the existing ground. The bleeder will be capped after testing is complete. The bleeder may not be used for the on-property system and must be disconnected at the time of the on-property hook-up.

Article 6.4 Measurement

Measurement for Furnishing and Installing Water Service Lines shall be per linear foot of horizontal distance of the various sizes as set forth in the Bid Schedule. Measurement will be from station to station as staked in the field and as shown on the Drawings, except where the grade exceeds twenty-five percent (25%), in which case measurement will be by actual pipe length.

Article 6.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Unless specifically identified for payment under a separate pay item, the unit price bid to Furnish and Install (size) Water Service Lines shall include all labor, equipment and materials to furnish and install a functional potable water service including, but not limited to, the following incidental items: verify location of existing water services; disconnection and reconnection of customer’s existing services where the Work includes replacement of existing services; clearing and grubbing; trench excavation and backfill; excess excavation and backfill; excavation dewatering; trench support system; furnishing and installing Class C bedding; compaction of fill; installation of pipe, fittings, adapters, or other necessary appurtenances; polyethylene encasement; hydrostatic testing, flushing, disinfection, water service insulation; disposal of unusable or surplus material; seeding; protection of existing utilities; restoration of existing drainage patterns; removal and replacement of existing
culverts, guardrail, fences, landscaping, and other public or private improvements; finish grading; and cleanup.

Where the Work includes disconnecting existing water services from an existing water main and reconnecting them to a new water main, the disconnection and reconnection of those existing water services will be considered incidental to the costs bid for installation of the new water main.

Fittings and appurtenances as shown on the Drawings or not specifically identified for payment under a separate pay item but required for normal completion of water service line installation, will be considered incidental and shall be included in the linear foot cost of the water service lines.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install (Size) Water Service Line</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 60.07  POLYETHYLENE ENCASEMENT

Article 7.1  General

The Work under this Section consists of providing all operations pertaining to the furnishing and installation of one layer of polyethylene encasement on all ductile and cast iron mains and services, fittings, fire hydrants, valve boxes, etc.

Article 7.2  Material

The polyethylene encasement material for pipe shall be 8-mils thick and conform to AWWA C105/ANSI A21.5.

Article 7.3  Construction

The polyethylene encasement shall be installed in strict conformance to the methods described in the most current editions of ANSI/AWWA C105/A21.5 and the Ductile Iron Pipe Research Association's "A Guide for the Installation of Ductile Iron Pipe."

Bedding and backfill material around pipelines with polyethylene encasement shall be placed using protective measures such as shields, guards, coating systems, and/or other methods as needed to protect the polyethylene encasement from becoming torn, punctured or otherwise damaged during the Work. Damage to the integrity of the polyethylene encasement shall be either repaired or the pipeline removed and the polyethylene encasement replaced as directed by the Engineer. Costs for repair and/or replacement of damaged polyethylene encasement shall be considered incidental to the installation of the polyethylene encasement and/or the installation of the pipeline protected by the encasement.

Article 7.4  Measurement

Measurement of all sizes of polyethylene encasement for pipe shall be the same as the measurement of the pipe installed, except that polyethylene encasement installed on fire hydrant legs and barrel sections and valve boxes shall be incidental to the item under construction.

Article 7.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Polyethylene Encasement</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 60.08 TEMPORARY WATER SYSTEMS

Article 8.1 General

The Work under this Section consists of the performance of all operations pertaining to the construction, installation, maintenance and removal of temporary water service during construction of this project to current AWWU customers in the area. It is the intent that the Contractor maintains water service during the entire period of construction activities to all current customers in the project area.

The Contractor shall submit a plan for any temporary water systems to the State of Alaska, Department of Environmental Conservation (ADEC) for review and approval prior to beginning Work on such system. The plan must identify the type of system, the method of construction and the maintenance and operation procedures to be used. The plan must identify service to each existing customer except those who agree in writing to have their service temporarily disconnected. The Contractor shall obtain such agreement. To be submitted with the plan are any agreements between the Contractor and property owner regarding access and use of private property. The methods to be employed in maintaining water service are left to the Contractor. Surface piping, trailer mounted supply systems, and so forth may all be considered as long as they comply with current health standards and requirements. A copy of the ADEC approved plan shall be provided to the Engineer, along with copies of any agreements with property owners referred to above.

The Contractor shall also submit the name and phone number of a contact person and at least one alternate who shall be available on a twenty-four (24) hour basis for repair and/or maintenance of the temporary water system. In the event that the Contractor fails to repair and/or maintain the temporary system and AWWU is required to perform repairs and/or maintenance, all costs associated with said repairs and/or maintenance shall be deducted from the Contract amount.

Article 8.2 Material

The Contractor shall use only those materials and equipment listed in this Section to supply temporary water service. Temporary water service shall be supplied under the service criteria outlined in this Section. All equipment used must be specifically designed and properly disinfected for the storage, handling, and delivery of potable water.

Service shall be supplied to each structure presently served by AWWU. The following minimum criteria shall be used for service to each structure:

A. Forty (40) psi minimum, one hundred (100) psi maximum delivery pressure measured at the connection to the structure.

B. Five (5) gallons per minute flow at the above delivery pressure measured at the connection to the structure. Commercial and other business structures may require higher water flows.
C. Potable water system and water quality shall conform to 18 AAC 80 Alaska Drinking Water Standards.

D. All services to structures shall be valved to allow individual control of service to each structure.

Materials used for temporary water service shall conform to the requirements of these Specifications. The temporary water service system shall be constructed from one or more of the following materials: polyvinyl chloride (PVC), high-density polyethylene (HDPE), copper, ductile iron, cast iron or galvanized steel.

The primary water feeder pipe shall be a minimum of three inches (3”) in diameter.

Article 8.3 Construction

All temporary water service equipment shall be disinfected per ANSI/AWWA C652, Disinfection of Water Storage Facilities and ANSI/AWWA C651, Disinfection of Water Mains. All bacteriological samples required under these Specifications shall be done by a testing laboratory certified by the State of Alaska.

All temporary service equipment shall be disinfected prior to connecting to a residence or business and shall be disinfected each and every time the equipment is moved or connected to another residence per above-referenced Specifications.

The Engineer shall be notified twenty-four (24) hours prior to the installation of any temporary water system. The Engineer shall be present to inspect the disinfection process of any temporary water service system.

No residence presently serviced by the AWWU system shall be without water for a period greater than six (6) hours in any twenty-four (24) hours period. Each residence or business owner shall be notified seventy-two (72) hours before they are transferred on or off the temporary water system and before any other service interruption. Prior to constructing temporary water services on private property, the Contractor shall secure a written “Permission to Enter” from the property owner. Such permission shall hold the Municipality of Anchorage, AWWU, and its agents harmless for any claims resulting from damage or harm sustained due to the Contractor’s operation. The Contractor shall also provide a copy of each “Permission to Enter” form to the Engineer.

Following the successful installation of the temporary water system, the existing water service shall be appropriately disconnected at a main shutoff valve inside the structure. Qualified personnel who are familiar with building plumbing systems shall accomplish the disconnection of the existing water service. This Work shall be performed to prevent backfeeding water through the service connection.

Fire hydrants may be used as a water source for a temporary water system. The Contractor will be required to obtain a hydrant permit from AWWU and will be required to meet all permit conditions (winter use of a hydrant shall required special permission from AWWU). In addition, the Contractor shall provide a gate valve assembly at the fire hydrant
as a shutoff valve for the temporary water system. The Contractor shall be responsible for any damage to the hydrant and temporary service piping and shall repair such damage at no cost to the Owner.

**Article 8.4 Measurement**

Providing temporary water service as required throughout the project shall be measured as lump sum.

**Article 8.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Temporary Water System</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 60.09 REPLACE VALVE BOX

Article 9.1 General

The Work under this Section consists of performing all operations pertaining to the removal, disposal, and replacement of mainline, fire line, and/or fire hydrant valve boxes that have become separated and/or misaligned to such an extent as to require replacement, from the top of the valve to final finished grade, including the replacement of all valve box sections, lids, and dust pans.

Article 9.2 Material

All materials used in the replacing mainline and fire hydrant valve boxes shall conform to the requirements defined in Section 60.03 - Furnish and Install Valves and the Standard Details.

Backfill shall be Type II Classified material to the subgrade elevation.

Article 9.3 Construction

All construction shall be in accordance with the provision of Section 60.03 – Furnish and Install Valves.

All locations where replacing a valve box is required shall be excavated to the top of the valve and conform to the procedures outlined in Section 60.03 - Furnish and Install Valves, concerning installation of the valve box and the Standard Details.

Article 9.4 Measurement

Valve boxes replacement will be measured per unit, complete in place.

Article 9.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Replace Valve Box</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.10  RESET VALVE BOX SECTIONS BELOW FINISHED GRADE

Article 10.1  General

The Work under this Section includes all operations pertaining to the reconnection of mainline, fire line, and/or fire hydrant valve box sections that have separated below finish grade. Work under this Section also includes the requirements of the Drawings and applicable sections of this Division and Division 20 – Earthwork. All broken and/or missing valve box components are to be replaced with new materials furnished and installed by the Contractor in accordance with these specifications.

Article 10.2  Material

All materials used in the reconnection of mainline and fire hydrant valve boxes shall conform to the requirements defined in Section 60.03 - Furnish and Install Valves and the Standard Details.

Backfill shall be Type II Classified material to the subgrade elevation.

Article 10.3  Construction

All construction shall be in accordance with the provision of Section 60.03 – Furnish and Install Valves.

All locations where reconnections are required shall be excavated to the depth required to perform the reconnection. The Contractor shall be responsible for removing the liner inside the valve box casing and determining the location of the separation. Care shall be used to ensure that soil or other foreign matter does not enter the valve box standpipe.

Article 10.4  Measurement

Resetting Valve Box Section Below Finish Grade will be measured per unit, complete in place. The same valve shall not be paid for under this pay item if it is paid for under Section 60.09 - Replace Valve Box. In particular, related work includes, but is not limited to, removal of debris from inside the valve box standpipe, trench excavation and backfill, disposal of unsuitable or surplus material, mechanical compaction, adjust mainline valve box to finish grade, replace broken valve box components, and classified materials. No separate measurement for payment will be made.
Article 10.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Reset Valve Box Sections Below Finished Grade</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.11 REPLACE TOP SECTION OF VALVE BOX

Article 11.1 General

The Work under this Section consists of performing all operations for the removal, disposal, and replacement of mainline and fire hydrant valve box top section(s), lid(s), and dust pan(s) that are missing or damaged in the opinion of the Engineer. The Contractor is to provide all labor, materials and supervision required to furnish and install new valve box components needed to rehabilitate existing valve boxes.

Under this Section, rehabilitation of existing valve boxes can include the following items of Work:

- Removal and replacement of valve box lids.
- Removal and replacement of valve box dust pans.
- Removal and replacement of valve box top sections.

The valve box components to be removed and replaced for a specific valve box are identified in the Drawings. The Contractor is to reuse those components that are not to be replaced in assembly of the rehabilitated valve box.

Article 11.2 Material

Materials used in this Work shall conform to the requirements of Section 60.03, Article 3.2 - Material.

Article 11.3 Construction

The Contractor shall excavate around the valve box as needed to access the Work. All excavation, shoring, dewatering, backfill and compaction efforts required to access the Work shall be per Division 20 – Earthwork. All importation of fill and/or disposal of unsuitable material, excavation, and backfill efforts shall be considered incidental to Work, and will not be paid separately.

Upon completion of the Work, the Contractor shall restore the existing grades and surrounding area to preconstruction conditions. Any pavement, sidewalk, curb and gutter, landscaping, and/or other improvements disturbed and/or damaged by the manhole rehabilitation effort shall be restored by the Contractor to preconstruction conditions. Restoration of these conditions shall be considered incidental to the Work, and will not be paid separately.

The Contractor shall remove and replace those valve box components identified in the Drawings. The rehabilitated valve box shall be configured according to the requirements of this Division and the Standard Details.
The Contractor shall use care in protecting those component parts of the existing valve box that are to be reused in the rehabilitated valve box.

Article 11.4 Measurement

Rehabilitated valve box assemblies shall be measured as units complete in place with the components identified in the Drawings replaced and accepted by the Engineer.

All effort required to complete the Work, including excavation, shoring, dewatering, backfilling, restoration of Work area to existing preconstruction conditions, and/or other items of Work needed to complete the Replace Top Section of Valve Box effort shall be considered incidental to the completion of the Work and shall not be paid for separately.

Article 11.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove and Replace Valve Box Lids</td>
<td>Each</td>
</tr>
<tr>
<td>Remove and Replace Valve Box Dust Pan</td>
<td>Each</td>
</tr>
<tr>
<td>Remove and Replace Valve Box Top Section</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.12  ABANDON PIPELINE IN PLACE

Article 12.1  General

The Work under this Section includes all operations pertaining to the abandonment of pipeline in place. Where shown on the Drawings, or otherwise directed by the Engineer, the Contractor shall abandon an existing pipeline in place in accordance with the requirements of this Section.

Article 12.2  Material

Sand slurry shall consist of a mixture of water and sand with an approximate ratio of seven (7) gallons of water per cubic foot of sand. Sand may consist of native material with a particle size distribution such that one hundred percent (100%) of the material passes the No. 4 U.S. Standard Sieve and contains no lumps, frozen material, organic matter, or other deleterious material.

Article 12.3  Construction

Wherever existing pipe is to be abandoned in place, the Contractor shall empty the line of all water, fill the pipe full with sand slurry, and plug the ends. Placement of the sand slurry shall be by means of a tremie pipe or other method that shall enable uniform placement of the sand slurry throughout the length of the pipe being abandoned. The Contractor shall demonstrate the entire pipe to be abandoned has been filled prior to the installation of end caps. Validation shall include placement of a predetermined volume of sand slurry into the pipe to be abandoned.

In the event the pipeline to be abandoned is cracked or crushed, the Contractor shall excavate to the next joint of pipe and install the plug. Crushed pipe sections or portions thereof shall be removed and disposed of by the Contractor.

All excavation, shoring, dewatering, disposal of unsuitable material, backfilling, and compactive effort required for completion of this Work shall conform to the requirements of Division 20 – Earthwork.

During the execution of this effort, the Contractor shall maintain vehicular traffic and pedestrian access as required in Division 10 - Standard General Provisions.

The Contractor shall restore the Work area to preconstruction conditions.

The Contractor shall notify the Engineer twenty-four (24) hours in advance of abandoning each main and shall provide safe access for the inspection of the process.

Article 12.4  Measurement

Measurement of quantities of pipeline to be abandoned in place shall be per lineal foot of pipeline to be abandoned for each nominal pipeline size. Length shall include pipeline that is removed due to damaged ends.
Removal and disposal of pipeline sections that have damaged ends and cannot be plugged in place shall be considered incidental to the Abandon Pipeline in Place scope of Work identified in this Section.

Any excavation, shoring, dewatering, disposal of unsuitable material, backfilling, compactive effort, maintenance of vehicular traffic and/or pedestrian access, paving, landscaping, or restoration of existing preconstruction conditions necessary to complete the Abandon Pipeline in Place scope of Work identified in this Section that is not specifically address by a separate bid item shall be considered incidental to the Work completed under this Section. Costs incurred for completion of these incidental Work items are considered including in the unit cost bid for completion of the Work in this Section.

**Article 12.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Abandon Pipeline in Place</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>(Pipeline Nominal Size) (Type of Pipe)</td>
<td></td>
</tr>
</tbody>
</table>
SECTION 60.13  CONNECT TO EXISTING WATER SYSTEM

Article 13.1  General

This Section consists of all Work necessary for furnishing all material, labor, and equipment necessary for locating, excavating, and assisting Municipal crews in making a live tap into an existing water main.

Article 13.2  Material

The Contractor shall obtain the live tap permit to initiate the connection process. The live tap permit will be issued by AWWU at no cost to the Contractor.

AWWU will supply the gate valve and valve box for the live tap.

AWWU will furnish the tapping machine and personnel to operate the tapping machine.

All materials used in the construction of connections to the existing water main shall conform to the requirements of Section 60.02 - Furnish and Install Pipe.

Article 13.3  Construction

The Contractor shall be responsible for trench excavation and backfill in accordance with Division 20, Section 20.14 - Trench Excavation, Backfill and Compaction for Service Connections. Excavation shall meet all OSHA standards.

The Contractor shall provide all necessary equipment and manpower to assist AWWU personnel in moving piping, valves, tapping machines and miscellaneous items into and out of the trench during the entire time AWWU personnel are working to complete the installation of the water line tap.

Connections to existing water mains shall utilize existing stubs, tees, crosses and valves. New valves may not be installed downstream of existing valves unless an active service or branch exists downstream of the existing valve.

The Contractor may choose to perform a valve leakage test of the existing valve in the presence of an AWWU inspector prior to making a connection. The quantity of water lost per hour shall be recorded and added to the completed main during pressure testing. Contractor shall remove existing valves found to be unacceptable and replace with a new valve.

Where no existing point of connection exists a live tap will be performed by AWWU for new mains up to twelve inches (12") in diameter. New mains larger than twelve inches (12") require the installation of a tee or cross.

AWWU will provide the staff, tapping machine, connection valve and valve box.

AWWU staff will complete the installation of the water main tap and new connection.
Contractor shall backfill around the tapped water main and new valve, and install the AWWU-furnished valve box assembly.

Contractor shall be responsible for all shoring, dewatering, disposal of unsuitable material, backfilling, and compaction effort.

During the execution of this effort, the Contractor shall maintain vehicular traffic and pedestrian access, as required in Division 10 – Standard General Provisions.

The Contractor shall restore the Work area to preconstruction conditions, as required by Division 10 – Standard General Provisions. Landscaping, paving and concrete work shall conform to Division 75 – Landscaping Improvements, Division 40 – Asphalt Surfacing, and Division 30 – Portland Cement Concrete, respectively, and the Standard Details.

The Contractor shall provide seventy-two (72) hours notice to the Engineer prior to anticipated flow interruption and/or physical connection to existing water systems to coordinate “turn-offs” with municipal crews. The actual connections shall be made only during periods of low water demand, as determined by the Owner.

The Contractor shall notify any property owners or residents, seventy-two (72) hours prior to interruption of any utility services. Disruption of water service to any structure shall be limited to six (6) hours in any twenty-four (24) hour period when authorized by the Engineer. Where the existing main line service to structures will be disrupted in excess of six (6) hours, the Contractor shall furnish and install a temporary water system. If the water service is disrupted in excess of six (6) hours, with or without the temporary water system, a penalty of $150 will be assessed per structure per violation within the twenty-four (24) hour period.

Construction of connections to existing water mains shall be in accordance with this Division and Section 60.02 - Furnish and Install Pipe.

The valve box shall be installed and adjusted to final grade by the Contractor.

**Article 13.4 Measurement**

Connect to existing water main shall be measured per each unit, complete in place.

**Article 13.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Connect to Existing Water Main</td>
<td>Each</td>
</tr>
<tr>
<td>(Identify Location, Type of Pipe</td>
<td></td>
</tr>
<tr>
<td>and Nominal Pipe Size)</td>
<td></td>
</tr>
</tbody>
</table>
SECTION 60.14   REMOVE AND SALVAGE EXISTING FIRE HYDRANT

Article 14.1   General

The Work under this Section consists of removing and salvaging serviceable portions of existing fire hydrant assemblies identified in the Drawings and delivering them to AWWU’s Operations Building at 325 East 94th Court, Anchorage, Alaska.

Article 14.2   Material

Materials used in this Work shall conform to the requirements of Section 60.02, Article 2.2 - Materials.

Article 14.3   Construction

The Contractor shall excavate, expose and remove the fire hydrant assemblies identified in the Drawings to be salvaged.

The hydrant assembly components to be removed and salvaged at each hydrant location where the hydrant is to be salvaged include:

- Hydrant
- Hydrant shoe
- Hydrant gate valve
- Hydrant gate valve box
- Hydrant leg

Upon inspection of the exposed hydrant assembly components, the Engineer may determine that one or more of the components are not salvageable. These items are to be transported by the Contractor to a disposal site approved for disposal of construction debris.

The Contractor shall install a plug in the branch connection on the tee in the water main where the hydrant assembly is removed. If the water main is to remain in active service, the plug shall be tested for leakage according to Section 60.02, Article 2.4 – Flushing and Testing, prior to the water main being covered with fill. Disinfect the existing water main at the locations where the hydrant assemblies are removed per AWWA C651.

Excavation, shoring, dewatering, disposal of unsuitable material, backfilling, and compaction, shall all conform to the requirements of Division 20 – Earthwork.

During the execution of this effort, the Contractor shall maintain vehicular traffic and pedestrian access as required in Division 10 – Standard General Provisions.

The Contractor shall restore the Work area to preconstruction conditions as required by Division 10 – Standard General Provisions. Landscaping, paving and concrete work shall conform to Division 75 – Landscaping Improvements, Division 40 – Asphalt Surfacing and Division 30 – Portland Cement Concrete, respectively, and the Standard Details.
**Article 14.4 Measurement**

Removing, disposing of or salvaging, and delivery of existing fire hydrant serviceable portions will be measured per each fire hydrant removed and salvaged in accordance with this Section.

**Article 14.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove and Salvage Existing Fire Hydrant</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.15  RELOCATE WATER MAIN

Article 15.1  General

The Work under this Section consists of providing all operations pertaining to relocating water mains. In the preparation of the Drawings, efforts have been made to determine exact elevations of live utilities; however, elevations of utilities shown are not represented as exact and are shown to include approximate location only. The Engineer shall have the final say as to whether the main is raised or lowered.

Article 15.2  Construction

Where a water main crosses the location of a sewer, the water main shall be raised or lowered sufficiently to permit a minimum (outside diameter) vertical distance of eighteen inches (18") from the sewer line. If the water main elevation is below the sewer line elevation, refer to Division 50, Section 50.13 – Polyethylene Encasement. The Contractor may employ either of the following methods for raising or lowering a water main. He may raise or lower lengths of the water main as necessary on either side of the proposed sewer to allow the main to pass under or over the sewer, providing the deflection at any joint does not exceed the pipe manufacturer’s recommendations, or the water main may be raised or lowered using four (4) pipe bends no to exceed twenty-two and one-half degrees (22 ½º). In special cases only, and when approved by the Engineer in advance, forty-five degree (45º) bends may be used. The method of lowering and materials to be used shall be approved by the Engineer prior to commencing Work. The Contractor shall give forty-eight (48) hours notice to AWWU and the Engineer prior to any planned water shutoff.

Water lines two inches (2") in diameter and smaller shall not be construed as water mains.

Any necessary lowering of water lines two inches (2") or smaller shall be included under the conditions set forth in the General Provisions for the moving and relocation of utilities occupying space within the area of construction. With the approval of the Engineer, the Contractor may lower water lines two inches (2") in diameter or smaller, but separate payment shall not be made for such lowering. The cost shall be included in the unit bid price as specified in Division 20, Section 20.13 - Trench Excavation and Backfill.

Article 15.3  Measurement

Raising or lowering existing water mains will be measured as units complete in place without regard to the diameter of the water main or length required to be lowered.
Article 15.4 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Relocate Water Main</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.16  RAISE OR LOWER WATER SERVICE

Article 16.1  General

The Work under this Section consists of providing all operations pertaining to raising or lowering existing water services when the grade(s) of such services interfere(s) with the construction of new sanitary or storm sewers. The Work includes, but is not limited to, trench excavation and backfill, compaction, furnishing trench backfill, disposal of unsuitable or surplus material, and water service line piping.

Article 16.2  Materials

Materials to be used in the Work shall conform to Section 60.06, Article 6.2 - Material.

Article 16.3  Construction

Where a conflict in grade occurs between new storm and/or sanitary sewers, and an existing water service connection, the Contractor shall excavate the water service connection from the point of interception to a sufficient distance to raise or lower the water service such that the grade conflict will be eliminated. In no case will the length of raising or lowering of the water service exceed fifty feet (50').

If the clearance between the raised or lowered water service and the storm/sanitary sewer is less than three feet (3’), insulation board (R-20) shall be installed in accordance with Section 60.10, Article 1.4 - Insulation. However, in no case shall the vertical separation distance between the service connection and the storm drain and/or sanitary sewer be less than eighteen inches (18") without an ADEC separation waiver.

All excavation, backfill, and pipe laying shall be performed in accordance with the applicable provisions of Division 20 - Earthwork and this Division. Any materials needed to complete the raising or lowering of a water service shall be provided by Contractor and considered incidental to the Contract.

The existing water service shall be disinfected according to AWWA C651 prior to being place back in service.

Article 16.4  Measurement

Measurement for raising or lowering water service lines will be measured as units complete in place.

Fittings and appurtenances not specifically identified for payment under a separate pay item, but required for normal completion of raising or lowering water service lines will be considered incidental and shall be included in the unit cost of the Work.

Disinfection of the raised or lowered water service line shall be considered incidental and shall be included in the unit cost of the Work.
Any excavation, shoring, dewatering, disposal of unsuitable material, backfilling, compactive effort, maintenance of vehicular traffic and/or pedestrian access, paving, landscaping, or restoration of existing preconstruction conditions necessary to complete the Raise or Lower Water Service scope of Work identified in this Section that is not specifically addressed by a separate bid item shall be considered incidental to the Work completed under this Section. Costs incurred for completion of these incidental Work items are considered included in the unit cost bid for completion of the Work in this Section.

**Article 16.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Raise or Lower Water Service</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.17   FURNISH AND INSTALL GALVANIC ANODES

Article 17.1   General

The Work under this Section consists of the performance of all Work required for furnishing and installing galvanic anodes for added protection of water pipe from corrosion. The Contractor shall install galvanic anodes in accordance with these specifications and in conformity with the detail shown on the Drawings, unless otherwise approved.

Article 17.2   Definitions

In these Contract Documents, the following words or expressions shall have the meaning given below:

- AWG: American Wire Gauge
- BDC: Bottom Dead Center of the Pipe
- HMWPE: High Molecular Weight Polyethylene
- NACE: National Association of Corrosion Engineers

Article 17.3   Materials

A.   Anodes

Anodes utilized for typical galvanic anode system installation shall be prepackaged magnesium style anodes weighing seventeen (17) pounds. Anode composition shall consist of the following:

<table>
<thead>
<tr>
<th>Element</th>
<th>Amount (%)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Cu</td>
<td>0.001</td>
</tr>
<tr>
<td>Si</td>
<td>0.01</td>
</tr>
<tr>
<td>Zn</td>
<td>2.5-3.5</td>
</tr>
<tr>
<td>Mn</td>
<td>0.2-0.5</td>
</tr>
<tr>
<td>Ni</td>
<td>0.001</td>
</tr>
<tr>
<td>Al</td>
<td>5.5-6.5</td>
</tr>
<tr>
<td>Fe</td>
<td>0.005</td>
</tr>
<tr>
<td>All Others</td>
<td>0.01</td>
</tr>
<tr>
<td>Magnesium</td>
<td>Balance</td>
</tr>
</tbody>
</table>
Anodes shall be packaged in a low resistive backfill consisting of seventy-five percent (75%) gypsum, twenty percent (20%) bentonite, and five percent (5%) sodium sulfate.

Anodes shall be provided with #8 AWG stranded copper, single-conductor cable with HMWPE insulation. Lead wire cable shall be rated for six hundred (600) volts and designed for direct burial applications.

B. Pipe Connection Lead Wire

Lead wires shall be of sufficient length for splice-free routing between the anode and the pipe and shall be #8 AWG stranded copper, single-conductor cable with HMWPE insulation. Lead wire cable shall be rated for six hundred (600) volts and designed for direct burial applications.

C. Thermite Welding Equipment and Materials

Equipment and materials used to bond the #8 AWG HMWPE to the pipeline shall be “CADWELD” type as manufactured by ERICO Products, Inc. of Cleveland, Ohio, or approved equal. Thermite weld caps, designed to protect the CADWELD bonds from corrosion, shall be Royston “Handy Cap 2” or approved equal.

Article 17.4 Installation

A. General Requirements

Excavation, shoring, dewatering, disposal of unsuitable material, backfilling, and compaction, shall all conform to the requirements of Division 20 – Earthwork.

During the execution of this effort, the Contractor shall maintain vehicular traffic and pedestrian access as required in Division 10 – Standard General Provisions.

The Contractor shall restore the Work area to preconstruction conditions as required by Division 10 – Standard General Provisions. Landscaping, paving and concrete work shall conform to Divisions 75 – Landscaping Improvements, Division 40 – Asphalt Surfacing, and Division 30 – Portland Cement Concrete, respectively, and the Standard Details.

B. Anode Installation

The following is a list of general procedures utilized for typical installation of galvanic anodes:

1. Anode Placement

Anodes shall be installed twelve to thirty-six inches (12” to 36”) from the side wall of the pipe, to a centerline depth in-line with the approximate horizontal plane of the pipe’s BDC. Anodes may be placed on either side of the pipeline, one anode per every second pipe section (joint).
2. Lead Wire Connection to Pipe

The #8 AWG HMWPE lead wires shall be attached to the top dead center of the pipe. Lead wire connection to the pipe shall utilize exothermic weld connection methodology as outlined above and on the drawings. Contractor shall follow CADWELD manufacturer’s instructions for use.

3. Backfilling

Extreme care shall be taken not to damage the anodes or direct buried lead wires during backfill procedures.

Article 17.5 Measurement

Measurement for furnishing and installing anodes shall be per each anode installed. The price shall include full compensation for furnishing and installing anodes as described herein and as shown on the Drawings.

Any excavation, shoring, dewatering, disposal of unsuitable material, backfilling, compactive effort, maintenance of vehicular traffic and/or pedestrian access, paving, landscaping, or restoration of existing preconstruction conditions necessary to complete the Furnish and Install of Galvanic Anodes scope of Work identified in this Section that is not specifically addressed by a separate bid item shall be considered incidental to the Work completed under this Section. Costs incurred for completion of these incidental Work items are considered included in the unit cost bid for completion of the Work in this Section.

Article 17.6 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install Anode</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.18  ABANDON PRIVATE WATER WELL

Article 18.1 Description

The Work under this Section consists of furnishing all material, labor, and equipment necessary to abandon existing private water wells as described in this Section.

The depths to the bottom of the boring, depth to static groundwater and locations of the wells to be abandoned are described in the Special Provisions.

Article 18.2 Materials and Construction

The Contractor shall use the following procedure when abandoning the wells:

1. Remove the existing well pump and appurtenances. The well pump and appurtenances shall be carefully removed to avoid damage and delivered to the property owner after removal.

2. Backfill the well casing to ten feet (10’) above the screen with disinfected sand or gravel. Sand shall be used as backfill adjacent to water bearing strata consisting of sand, and gravel shall be used as backfill adjacent to water bearing strata consisting of gravel. Disinfected sand or gravel is defined as sand or gravel washed in a one part per million chlorine/water solution prior to the backfilling operation. The Contractor shall provide proof to the Engineer that any imported sand or gravel has been disinfected prior to backfilling the casing.

3. If the aquifer is pressurized, place a seal over the top of the disinfected sand or gravel to seal the aquifer. The seal may consist of bentonite chips or other suitable means, as approved by the Engineer.

4. Backfill the next section of well casing for a minimum distance of ten feet (10’) with concrete or cement grout. If necessary, the depth of the concrete or cement grout may have to exceed ten feet (10’) to ensure the lower aquifer is thoroughly sealed. The concrete or cement grout shall be placed from the bottom upward through a pipe or tremie in such a way as to avoid segregation or dilution of the material. The concrete or cement grout shall be allowed thirty-six (36) hours to cure prior to proceeding with the next step.

5. Backfill the next section of well casing to fifteen feet (15’) below the ground surface with gravel. Disinfected gravel is not required in this zone.

6. Excavate the area adjacent to the top of the well to a depth of five feet (5’) and cut the casing at this level. Then backfill the remaining ten feet (10’) of well casing with bentonite, concrete, or cement grout. Weld a metal cap on top of the well casing to ensure the well is sealed.
7. Backfill the area within a two foot (2') minimum radius from the center of the well casing to a level which is two inches (2") above the top of the sealed well casing with concrete or cement grout to preclude the downward migration of water along the outside of the casing. Then backfill the remainder of the excavated hole with native soils.

8. As part of this bid item, the Contractor shall be responsible for topsoil and reseeding all lawn areas damaged by the Contractor during the well abandonment operation. In addition, the Contractor shall be responsible for the repair and/or replacement of all existing utilities, driveways, trees, utility markers, survey monuments, fences, retaining walls, buildings, sidewalks, gardens, landscaping, and other private improvements damaged by the Contractor as a result of the well abandonment operation.

9. The Contractor shall provide a log of the well abandonment to the Engineer prior to receiving final payment for this Work. The log shall describe the materials used in the abandonment and the depths below existing grade each type of material was used, in addition to any other pertinent information regarding the abandonment.

The Contractor may employ, at his/her option, an alternate method of abandoning the wells that conforms to the requirements of ANSI/AWWA Standard A 100. In the event the Contractor elects to obtain approval to employ an alternate method, he/she shall first secure the approval of the ADEC and then submit a Substitution Request form to the Engineer for approval. The substitution request shall clearly identify the method the Contractor wishes to employ; clearly reference applicable sections of ANSI/AWWA Standard A 100 which allow the Contractor's proposed method of abandonment; and, include written approval from the ADEC specific for these particular walls.

Excavation, shoring, dewatering, disposal of unsuitable material, backfilling, and compaction, shall all conform to the requirements of Division 20 – Earthwork.

During the execution of this effort, the Contractor shall maintain vehicular traffic and pedestrian access as required in Division 10 – Standard General Provisions.

The Contractor shall restore the Work area to preconstruction conditions as required by Division 10 – Standard General Provisions. Landscaping, paving and concrete work shall conform to Division 75 – Landscaping Improvements, Division 40 – Asphalt Surfacing, and Division 30 – Portland Cement Concrete respectively, and the Standard Details.

**Article 18.3 Measurement**

Measurement for payment shall be per each well abandoned in accordance with the requirements of this Section or ADEC requirements if an alternate method of well abandonment is employed and approved by the Engineer.

Any excavation, shoring, dewatering, disposal of unsuitable material, backfilling, compactive effort, maintenance of vehicular traffic and/or pedestrian access, paving,
landscaping, or restoration of existing preconstruction conditions necessary to complete the Abandon Existing Private Water Well scope of Work identified in this Section that is not specifically addressed by a separate bid item shall be considered incidental to the Work completed under this Section. Costs incurred for completion of these incidental Work items are considered included in the unit cost bid for completion of the Work in this Section.

**Article 18.4  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Abandon Private Water Well</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.19   ADJUST KEY BOX

Article 19.1 General

The Work under this Section consists of providing all materials, equipment and labor and performing all operations necessary for adjusting existing key boxes to finished height and/or finished grade. All broken and/or missing keybox components are to be replaced with new materials furnished and installed by the Contractor in accordance with these specifications.

Article 19.2 Material

All materials used in the key box adjustment shall conform to the requirements defined in Section 60.06 - Water Service Lines and the Standard Details.

Article 19.3 Construction

Key boxes to be adjusted will be identified by the Engineer. In all cases the maximum height of the adjusted key box will be flush with the final ground surface. If excavation is required to adjust the key box, the ground surface will be restored to its original condition unless otherwise indicated in the Drawings. The Contractor shall be responsible for ensuring that the valve box is vertical, clean, to proper grade, and readily accessible for operation of the valve.

Any damage to a key box resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor’s expense. Adjustments to key boxes to be lowered will include cutting excessive length of key box, threading, and installing threaded unions to complete adjustments. Only threaded joints will be allowed in the Work. “Quick-connect” style connections assembled with set screws will not be allowed in the Work.

Where key box is located in concrete slab, adjustment will include cutting concrete, installing pavement riser and lid, and restoring disturbed area to original condition.

Contractor shall adjust the valve box to finish grade prior to placement of asphalt pavement. After-the-fact cutting of new asphalt for adjustments is not accepted. Any adjustment(s) requiring cutting of new asphalt shall not be paid and shall be deducted from the quantity.

Article 19.4 Measurement

Adjusting key boxes will be measured per unit, complete in place.
**Article 19.5  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 – Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Key Box</td>
<td>Each</td>
</tr>
<tr>
<td>Adjust Key Box (Concrete Slab or Asphalt Paving)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 60.20  ADJUST VALVE BOX TO FINISH GRADE

Article 20.1  General

The Work under this Section consists of providing all operations pertaining to adjustment of existing mainline or hydrant valve boxes to finish grade, including the replacement of any and all broken valve box sections, lids, and dust pans.

Article 20.2  Material

All materials used in the adjustment of mainline valve boxes shall conform to the requirements of the utility company having jurisdiction over the water system.

Article 20.3  Construction

All valve box adjustments will be accomplished as directed by the Engineer. During the adjustment of the valve boxes, the top section will be replaced with a new top section, dust pan, and lid marked “water,” per the water utility specifications. Any salvaged top sections will be identified by the Engineer. All salvaged top sections will be delivered to the Street Maintenance Storage Yard by the Contractor. Any damage to a mainline valve box resulting from construction under this contract shall be repaired or the damaged portion replaced at the Contractor's expense. The Contractor shall be responsible for ensuring that the valve box is vertical, clean, to proper grade, and readily accessible for operation of the valve.

Contractor shall adjust the valve box to finish grade prior to placement of asphalt pavement. After-the-fact cutting of new asphalt for adjustments is not accepted. Any adjustment(s) requiring cutting of new asphalt shall not be paid and shall be deducted from the quantity.

Article 20.4  Measurement

Mainline valve box adjustments will be measured per unit, complete in place.

Article 20.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
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</thead>
<tbody>
<tr>
<td>Adjust Valve Box to Finish Grade</td>
<td>EACH</td>
</tr>
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MUNICIPALITY OF ANCHORAGE
STANDARD SPECIFICATIONS

DIVISION 60
WATER SYSTEMS
STANDARD DETAILS
60-1  MJ Cap and Plug
60-2  Thrust Block
60-3  Typical Valve Box
60-4  Single Pumper “L” Base Fire Hydrant Assembly
60-5  Double Pumper “L” Base Fire Hydrant Assembly
60-6  Fire Hydrant Guard Posts
60-7  Water Service Connect - 1”
60-8  Water Service Connect - 1-1/2” and 2”
60-9  Irrigation System
60-10 Connecting Ductile Iron Pipe to Ductile Iron Pipe
60-11 Typical Pipe Angle Marker
60-12 Relocate Water Main (Storm Drain)
60-13 Anode Detail
60-14 Adjust Service Key Box
MJ CAP

MJ Plug

NOTES:
1. MECHANICAL JOINT RESTRAINT EBAA IRON MEGALUG® OR EQUAL.
2. COST OF THIS FITTING TO BE INCLUDED IN BID PRICE OF PIPE.
NOTE:

1. MINIMUM THICKNESS OF PRE-CAST CONCRETE THRUST BLOCKS SHALL BE 6-INCH OR PER THE CONTRACT SPECIFICATIONS, AND IN CONFORMANCE WITH DIVISION 30.

2. THRUST BLOCK MAY NOT BE USED IN LIEU OF THRUST RESTRAINT

3. VIRGIN/ORIGINAL GROUND

<table>
<thead>
<tr>
<th>PIPE SIZE</th>
<th>PLUG</th>
<th>90° BEND</th>
<th>45° BEND</th>
<th>22 1/2° BEND</th>
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<tbody>
<tr>
<td>6”</td>
<td>2.0</td>
<td>2.0</td>
<td>1</td>
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<tr>
<td>8”</td>
<td>2.5</td>
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<tr>
<td>12”</td>
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<td>14”</td>
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<td>4.5</td>
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<td>16”</td>
<td>10.5</td>
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<td>6</td>
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<tr>
<td>24”</td>
<td>24</td>
<td>24</td>
<td>13</td>
<td>13</td>
</tr>
</tbody>
</table>
NOTES:

1. VB ADJ. SLEEVE & LID TO BE EJIW 3669 SERIES OR EQUAL.

2. VB RISER PIPE TO BE (TYLER) 5" DIA., 10 FOOT SECTION ONLY SINGLE HUB SOIL PIPE OR EQUAL. (TO BE PLUMBED STRAIGHT)

3. VB BTM SECTION TO BE EJIW 3669 SERIES OR EQUAL.

4. VALVE BOX DUST PAN SHALL BE DUCTILE IRON.

5. VB RISER PIPE AND BTM SECTION TO BE WRAPPED W/ 8 MIL OF POLYETHYLENE ENCASEMENT.

WRAP ONE (1) LAYER OF GEOTEXTILE FABRIC AROUND OUTSIDE OF VALVE AND UP AROUND TOP OF VB BTM SECTION. TAPE AND SECURE GEOTEXTILE FABRIC AROUND TOP OF BTM SECTION AND EACH VALVE END (PER THE ENGINEER).

WRAP BURLAP INSIDE VB BOTTOM (BTM) SECTION AND AROUND AND UNDER PACKING GLAND. SECURE WITH TAPE OR WIRE. (PER THE ENGINEER).
STENCILED DISTANCE AND DIRECTION TO FIRE HYDRANT GATE VALVE
(EXAMPLE: 19’N)

(2 EA.) 2–1/2” HOSE CONNECTIONS

(1 EA.) 4–1/2” PUMPER CONNECTION

BARREL SECTION

FINISH GRADE

MJ WITH MEGALUG®
(RESTRAINED JOINT)

18” (MAX. LENGTH) OF
HYD. LEG CONNECTION,
6” D.I.P. (MAX. PIPE)

6” GATE
VALVE

LEG VARIES

6” SHOE

GUARD POSTS
(SEE STANDARD DETAIL 60–6)

4” x 4’ (WIDE)
EXTRUDED
POLYSTYRENE
INSULATION
(CUT INSULATION TO
FIT AROUND BARREL
AND VALVE BOX)

DRAIN PLUGS (EA. SIDE)
(TO BE INSTALLED)

3” (MIN.)
9” (MAX.)

SINGLE PUMPER 'L' BASE
FIRE HYDRANT ASSEMBLY

NOTES:
1. HYDRANT BARREL SHALL BE INSTALLED PLUMB AND THE LEG SHALL BE LEVEL.
2. DRAIN PLUGS TO BE PROVIDED BY CONTRACTOR.
3. ALL HYDRANTS SHALL BE PAINTED CATERPILLAR YELLOW.
4. HYDRANT GATE VALVE BOX TO BE INSTALLED ACCORDING TO DETAIL FOR
   TYPICAL VALVE BOX.
5. ALL PIPE AND FITTINGS FROM THE MAIN TO THE HYDRANT SHOE SHALL BE
   RESTRAINED BY USE OF MEGALUG® AND/OR FIELD LOK® GASKETS OR EQUAL.
6. ALL BACKFILL MATERIAL AROUND HYDRANT BARREL SHALL BE NFS.
STENCILED DISTANCE AND DIRECTION TO FIRE HYDRANT GATE VALVE (EXAMPLE: 19'N)

(2 EA.) 4-1/2" PUMPER CONNECTION

(1 EA.) 2-1/2" HOSE CONNECTIONS

BARREL SECTION

GUARD POSTS (SEE STANDARD DETAIL 60-6)

FINISH GRADE

MJ WITH MEGALUG® (RESTRAINED JOINT)

10' (MIN.)

3" (MIN.) 9" (MAX.)

4" x 4' (WIDE) EXTRUDED POLYSTYRENE INSULATION (CUT INSULATION TO FIT AROUND BARREL AND VALVE BOX)

18" (MAX. LENGTH) OF HYD. LEG CONNECTION, 8" D.I.P. (MAX. PIPE)

8" GATE VALVE

LEG VARIES

8" SHOE

DRAIN PLUGS (EA. SIDE) (TO BE INSTALLED)

NOTES:
1. HYDRANT BARREL SHALL BE INSTALLED PLUMB AND THE LEG SHALL BE LEVEL.
2. DRAIN PLUGS TO BE PROVIDED BY CONTRACTOR.
3. ALL HYDRANTS SHALL BE PAINTED CATERPILLAR YELLOW.
4. HYDRANT GATE VALVE BOX TO BE INSTALLED ACCORDING TO DETAIL FOR TYPICAL VALVE BOX.
5. ALL PIPE AND FITTINGS FROM THE MAIN TO THE HYDRANT SHOE SHALL BE RESTRAINED BY USE OF MEGALUG® AND/OR FIELD LOK® GASKETS OR EQUAL.
6. ALL BACKFILL MATERIAL AROUND HYDRANT BARREL SHALL BE NFS.
NOTES:
1. GUARD POSTS WILL BE FURNISHED & INSTALLED BY THE CONTRACTOR AND APPROVED BY THE ENGINEER.
2. GUARD POSTS SHALL BE INSTALLED PLUMB AND LOCATED TO ALLOW UNRESTRICTED ACCESS TO PUMPER AND HOSE CONNECTIONS.
3. GUARD POSTS NOT INSTALLED SHALL BE DELIVERED TO AWWU OPERATIONS DIVISION (PER THE ENGINEER).
NOTES:
1. USE MUELLER CORPORATION STOP NO. 15050 OR EQUAL FOR STEEL PIPE.
2. USE MUELLER CORPORATION STOP NO. 10016 OR EQUAL FOR WOOD STAVE PIPE.
3. USE MUELLER 1-INCH CORPORATION STOP NO. 15000 OR EQUAL FOR DUCTILE PIPE.
4. USE MUELLER CURB STOP NO. H-15214 OR EQUAL FOR COPPER TO COPPER CONNECTIONS.
5. KB ROD TO BE ATTACHED TO CURB STOP WITH 9/64" x 4" BRASS COTTER PIN, NO SUBSTITUTIONS (MANUFACTURERS COTTER PIN SHALL NOT BE USED).
6. WRAP AROUND STAINLESS STEEL SERVICE SADDLE SHALL BE USED ON ALL PIPE.
7. THAW WIRE TO BE PLACED PARALLEL TO THE SERVICE LINE AND SHALL NOT COME IN CONTACT WITH THE SERVICE LINE AT ANY LOCATION.
NOTES:
1. USE MUELLER CORPORATION STOP NO. 15025 OR EQUAL.
2. USE MUELLER CURB STOP NO. H15184 ORISEAL OR EQUAL FOR COPPER TO IRON CONNECTIONS.
3. WRAP AROUND STAINLESS STEEL SERVCE SADDLE SHALL BE USED ON ALL PIPE.
4. KB ROD TO BE ATTACHED TO CURB STOP WITH 9/64 X 4” BRASS COTTER PIN, NO SUBSTITUTIONS (MANUFACTURERS COTTER PIN SHALL NOT BE USED).
5. USE MUELLER CURB STOP NO. H15214 ORISEAL OR EQUAL.
6. THAW WIRE TO BE PLACED PARALLEL TO THE SERVICE LINE AND SHALL NOT COME IN CONTACT WITH THE SERVICE LINE AT ANY LOCATION.
IRRIGATION FACILITIES ENCLOSURE

6. FOR SECURITY AND PROTECTION, IT IS RECOMMENDED THAT THE ENCLOSURE BE FABRICATED WITH 1-1/2" X 1-1/2" ANGLE IRON (FRAME) COVERED W/ FLAT EXPANDED METAL (MESH) PRIMED AND PAINTED. ENCLOSURE SHALL BE SIZED TO ENCLOSE ALL ABOVE FINISH GRADE APPURTENANCES AND SHALL PROVIDE ADEQUATE DRAINAGE TO KEEP THE BACKFLOW PREVENTER FROM BECOMING SUBMERGED.
1. MECHANICAL JOINT (MJ) LONG SOLID SLEEVE SHALL BE USED TO CONNECT SAME SIZE (O.D.) DUCTILE IRON PIPE (D.I.P.) TO D.I.P. ONLY.
2. SEE MANUFACTURERS RECOMMENDATIONS FOR DIMENSION "W"
3. ALL D.I.P. FITTINGS SHALL CONFORM TO THE REQUIREMENTS OF AWWA C110/ANSI A21.10 (SEE SECTION 60.02 – FURNISH AND INSTALL PIPE)
3/8" - 5/8" TOP OF PAVEMENT

6" 12"

2" ALUMINUM CAP SEE RIGHT

VALVE BOX TOP SECTION & LID

5/8" X 36" REBAR HAMMER INTO GROUND

2" ALUMINUM CAP

NOTE: BEND SHOWN ON ALUMINUM CAP SHALL MATCH ACTUAL INSTALLED WATER MAIN BEND.
NOTES:

1. ALL PIPE AND FITTINGS SHALL BE RESTRAINED BY USE OF MEGALUG® AND/OR FIELD LOK® GASKETS OR EQUAL.

2. RELOCATED WATER MAIN SHALL HAVE A MINIMUM SEPARATION OF THIRTY-SIX INCHES (36") BETWEEN STORM AND WATER. IF LESS THAN THIRTY-SIX INCHES (36") OF SEPARATION CANNOT BE OBTAINED THEN FOUR INCHES (4") OF INSULATION IS REQUIRED. IF EIGHTEEN INCHES (18") OF SEPARATION CANNOT BE MAINTAINED BETWEEN WATER AND SEWER OR STORM AN ADEC WAIVER IS REQUIRED.

3. RIGID BOARD INSULATION SHALL BE HIGH DENSITY EXTRUDED POLYSTYRENE, MIN. 60 P.S.I., EQUIVALENT TO R-20 PER FOUR INCH (4") THICKNESS. INSULATION SHALL BE BE POSITIONED NO LESS THAN OR EQUAL TO FOUR INCHES (4") FROM STORM SEWER.

4. ALL BACKFILL MATERIAL AROUND RELOCATED WATER MAIN SHALL BE NFS AND COMPACTED TO 95% MAX. DENSITY.

5. ALL MATERIALS USED TO RELOCATE WATER LINE SHALL BE APPROVED BY THE AWWU ENGINEER.
NOTES:
1. HIGH POTENTIAL MAGNESIUM ANODES SHALL BE PREPACKAGED IN A CLOTH BAG WITH A BACKFILL MIXTURE OF 75% GYPSUM, 20% BENTONITE AND 5% SODIUM SULFATE. THE ANODES SHALL HAVE A 20 lb. BARE WEIGHT AND APPROXIMATELY 70 lb. PACKAGED WEIGHT.
2. ANODES SHALL BE INSTALLED ON EVERY OTHER JOINT OF PIPE (A MAXIMUM OF 36 FEET OF BURIED PIPE BETWEEN ANODES). AN ANODE IS REQUIRED ON THE FIRST AND LAST JOINT OF PIPE.
3. CONTRACTOR SHALL PROVIDE COORDINATES OR PIPE STATIONING FOR EACH ANODE INSTALLED.
4. TWO #2 AWG THAW WIRES ARE REQUIRED FOR PIPE SIZES LESS THAN 12". TWO #2 AWG JOINT BOND WIRES ARE REQUIRED FOR PIPE SIZES 12" AND LARGER, WHEN ANODE INSTALLATIONS ARE REQUIRED.
5. ALL CABLES SHALL BE SINGLE CONDUCTOR, STRANDED COPPER, WITH TYPE HMWPE INSULATION RATED FOR 600 VOLTS.
6. SPLIT–BOLT CONNECTIONS SHALL NOT BE ALLOWED ON ANY UNDERGROUND CONDUCTORS. IF SPLICES ARE REQUIRED, COMPRESSION CONNECTIONS (BURNDY OR APPROVED EQUAL) SHALL BE USED. COMPRESSION CONNECTIONS SHALL BE SEALED WITH A HEAT SHRINK SLEEVE RATED FOR BELOW GRADE USE.
7. EXOTHERMIC WELDS SHALL BE MADE IN STRICT ACCORDANCE WITH THE MANUFACTURER’S RECOMMENDATIONS (NOTE: COPPER CONDUCTOR SLEEVES ARE REQUIRED FOR #10 WIRE BY SOME MANUFACTURERS). THE EXOTHERMIC WELD AREA SHALL BE COATED WITH COAL TAR MASTIC.
8. AT FIRE HYDRANT LOCATIONS, INSTALL ONE ANODE (12"–36" AWAY FROM THE PIPE) AT THE MIDPOINT BETWEEN THE TEE FROM THE MAIN LINE PIPE AND THE HYDRANT SHOE.
KEY BOX TOP (NOT INSTALLED)  
KEY BOX TOP CENTER PLUG (TO BE RELOCATED)  
ASPHALT PAVEMENT, CONCRETE WALK OR DRIVE  
CENTER PLUG RELOCATED (SEE NOTE 1)  
4" MIN.  
6" MAX.  
1/4" TO 1/2"  
PIECE COUPLER  
STANDARD SERVICE KEY BOX (KB)  

VB ADJ. SLEEVE W/ TOP EJW 3669 SERIES OR EQUAL  
INSTALL THAW WIRE LONG ENOUGH TO EXTEND 2' ABOVE VALVE BOX (VB) ADJ. SLEEVE TOP. WRAP EXCESS WIRE AROUND KEY BOX INSIDE VB.  

NOTES:  
1. REMOVE KEY BOX TOP AND INSTALL PIPE COUPLING ON STANDARD KEY BOX, REMOVE CENTER PLUG FROM KEY BOX TOP AND INSTALL INTO TOP OF PIPE COUPLER.  
2. TYPICAL INSTALLATION WHEN KEY BOX FALLS WITHIN ASPHALT PAVEMENT, CONCRETE WALK OR DRIVEWAY.  
3. TO BE INSTALLED AND APPROVED BY THE ENGINEER.  
4. ALL BACKFILL MATERIAL AROUND VALVE BOX SHALL BE NFS AND COMPACTED TO 95% MAX. DENSITY.
## STANDARD CONSTRUCTION SPECIFICATIONS
### FOR MUNICIPAL CONSTRUCTION SURVEYS
#### DIVISION 65
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SECTION 65.01 GENERAL

Article 1.1 Scope of Work

The Contractor shall furnish all labor and materials necessary to perform all surveying and staking essential for the completion of construction in conformance with the Drawings, Specifications, and Contract Documents. The Contractor shall perform all the necessary Work and calculations required to accomplish the Work in accordance with this Division.

This Section establishes a minimum standard of field survey specifications and procedures to properly control Municipal construction projects. The Contractor shall insure that commonly accepted practice of survey methods and procedures are followed. Errors or damages resulting from the Contractor's survey shall be corrected or made whole at the expense of the Contractor. The Owner shall not be held liable for any additional expense. Any method conflicting with these survey specifications must be approved by the Engineer prior to its use.

An Alaskan Registered Professional Surveyor, subcontracted to the Contractor shall perform all surveying, monumentation, staking and cross section for quantities pay item measurements. All personnel involved in measuring and recording survey data shall be directly employed by the Surveying Subcontractor and shall not be employed by the Contractor or any of the other Subcontractors for the duration of the project. Failure to adhere to this specification will result in non-payment for all Work affected by non-compliance.

The Contractor shall notify the Engineer twenty-four (24) hours in advance prior to beginning Work. All requests for information or determinations concerning the project shall be directed to the Engineer.

Contractor shall conduct a preconstruction survey to establish the existing road centerline and gutter lip profiles as applicable, within five working days prior to beginning construction staking. Contractor shall submit the survey field notes and a centerline profile plot drawn on vellum drawing paper at the same scale as the Drawing scale to the Engineer. The Engineer will have five days to review the survey notes and profile drawings prior to the start of construction.

Article 1.2 Payment - General

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.
SECTION 65.02 CONSTRUCTION SURVEYING

Article 2.1 Project Control

The Owner may provide project horizontal and vertical control monuments to facilitate construction staking or the Owner may not have provided horizontal and vertical control monuments for a project. Regardless, the Contractor shall recover project survey control monuments shown on the Drawings or establish project survey control monuments to ensure the project is properly located and constructed according to the Contract Documents.

Survey control monuments may be shown on the Drawings. Prior to construction, the Contractor shall locate these monuments to ensure they have not been destroyed. In the event the Contractor is unable to locate certain monuments, the Contractor shall notify the Engineer immediately and provide five (5) working days for the Engineer to reestablish the missing monumentation.

The Contractor shall have no basis for a claim requesting additional compensation for costs incurred due to missing survey control which is shown on the Drawings, unless the Engineer fails to reestablish said control within five (5) working days after written notification from Contractor. The Contractor may be entitled an extension of time as the Engineer may determine. Claim for extension of time shall be in accordance with Division 10, Section 10.05, Article 5.23 - Delays and Extension of Time.

The Contractor shall notify the Engineer immediately if a discrepancy exists between the field conditions and the Contract Documents. Project staking, which would be directly affected by the discrepancy, shall cease until further notice by the Engineer. Work unaffected by the discrepancy shall continue uninterrupted.

The Contractor is responsible for preserving, protecting and replacing all monuments and lot corners, line stakes, grade stakes, reference points, and hubs. In the event of their loss or destruction, the Contractor shall pay all costs for their replacement.

A. Monuments

1. General Description

A monument is defined as a material object used to physically identify a measured point on the earth’s surface, representing a land boundary that was determined by a land survey. The term “monument” will be deemed generic to identify public land corners, private property corners and public agency vertical and horizontal control monuments. If a question arises as to the validity of a found object being a monument, it should be submitted to the Engineer for clarification prior to disturbance or removal.
2. Existing Monument and Lot Corner Search

Contractor shall perform a monument search and make a record of the monument and lot corner search in the survey control field book, before commencement of construction staking. The monument search shall include both centerline and property monumentation.

Contractor shall locate and verify all project survey control monuments shown on the Drawings to ensure that they have not been disturbed or destroyed. In the event the Contractor is unable to locate any survey control monument that is shown on the Drawings, the Contractor shall notify the Engineer immediately. The Engineer shall have five working days to reestablish the missing monument or make a determination whether the project can be accurately staked without the missing monument.

The field book record of the monument and lot corner search shall state which monuments were found and which were not found. Contractor shall obtain record plats within the construction limits to assist in the search.

The Contractor shall replace all monuments and lot corners that are missing upon completion of construction unless the Contractor can show that the monument or lot corner was searched for and none existed prior to construction.

The requirement to search for existing monuments and lot corners is governed by a separate pay item in the Bid Schedule shall be measured and paid as identified in Article 2.15 – Method of Measurement and Article 2.16 – Basis of Payment.

3. Requirement to File Record of Monument

The State of Alaska Statute (AS 34.65.040) requires A RECORD OF MONUMENT to be filed with the State District Recorders Office immediately after establishment of survey control and prior to clearing and grubbing and/or excavation work for all applicable monuments. Monuments for which a record of monument shall be filed are defined as follows:

- U.S. public lands survey monument established by a cadastral land survey.
- Alaska state land survey monument established by a cadastral land survey.
- Municipality of Anchorage land survey monument established by a cadastral land survey.
- Exterior boundary monument controlling a record survey.
• Geodetic control monument established by a federal, state, or municipal agency.

Proof of recording shall be submitted to the Engineer in the form of a copy of the monument of record bearing the State District Recorders stamp before the monument is disturbed or removed.

A second RECORD OF MONUMENT shall be filed for each monument after the monument has been replaced (refer to AS 34.65.040). The record of monument shall be filed within five (5) working days of the date the monument was installed. Proof of recording shall be submitted to the Engineer in the form of a copy of the record of monument bearing the State District Recorder’s stamp.

B. Requirement to Establish Monuments

1. General

The Contractor shall replace any monument that exists within the construction limits if it is disturbed or removed due to project activity. All monumentation disturbed or removed shall be replaced with the same type monument or monument approved by the Engineer. All monuments that are replaced shall be crowned with a self identifying cap bearing the surveyor’s license number, year set, the lot, block and subdivision name stamped into the cap. No plastic monument caps are allowed. Should a physical impediment prevent a monument from being reestablished at its original location, one or more reference monuments shall be established. The establishment of reference marks shall be coordinated with the Engineer.

2. Centerline Monumentation for Road Improvement Projects

Projects which include paving or repaving of the road surface shall establish monuments installed in a monument case at all project centerline PCs, PTs, angle points, and street intersections. Monuments established to identify street intersections, angle points, and PCs/PTs of curves shall be center punched and stamped with the following information:

• centerline stationing
• year set
• surveyor’s license number
• the initials “M.O.A.”

Monuments that are located in gravel road surfaces, fill slopes, back slopes or ditches shall be installed six inches (6”) below the finished surface.

Existing subdivision lot corner monuments located within paved portions of a public use easement shall be replaced with a like monument installed flush with the top of finished pavement grade.
3. Utility Projects Within the Road Right Of Way

Maintenance and utility projects including storm drains, traffic signalization/channelization and gravel surface re-grading and reshaping projects, do not require the establishment of new monumentation. However, in accordance with SubArticle B.1 - General. above, the Contractor is responsible for replacing any existing monuments disturbed or removed during the Work.

4. Standard Monument and Monument Case Specifications

The standard monument is a five-eighths by thirty inch (5/8" x 30") iron rebar with a two inch (2") aluminum cap attached. The monument case shall conform to AASHTO M-105, Class 30A or DOT/PF Standard Drawing M-16.01. The case shall be coated with coal-tar pitch varnish. The top of the case shall be installed flush with the pavement surface. The top of a monument installed in a case shall be four-tenths feet (0.4') below the top of the case.

5. Request to Install Additional Monumentation

The Owner may request that additional monumentation be established and installed. Additional monumentation is extra to the project and not identified in the Contract Documents. The monuments would be established and installed according to SubArticles B.1 - General and B.4 - Standard Monument and Monument Case Specifications above.

This Work is governed by a separate pay item in the Bid Schedule and is separate from the lump sum construction survey pay item listed in Article 2.16. The measurement for this pay item is identified in Article 2.15 Method of Measurement.

C. Project Control Accuracy

1. Horizontal Control

The maximum permissible linear error allowed in establishing horizontal control is 1:10,000 feet. The maximum error allowed in unadjusted angular closure shall be calculated by the formula "15 times the square root of N." The term "N" signifies the number of transit setups in a traverse and "15" signifies fifteen seconds.

2. Vertical Control

Vertical datum shall originate from the MOA Benchmark Network or NGS Vertical Level Line System. All level circuits run to establish temporary bench marks shall have an accuracy no less than the value computed by the equation (three-hundredths feet (0.03') times the square root of the distance
in miles). Foresights and backsights shall be balanced. The maximum sighting distance shall not exceed three hundred feet (300’). All leveling circuits establishing TBMs will be adjusted utilizing recognized standard surveying adjustment methods. Side shots to establish an elevation on TBMs will not be allowed.

A minimum of two known bench marks shall be utilized when establishing TBMs to verify correct elevation information. A sufficient number of TBMs shall be set to control a project with a maximum spacing of eight hundred feet (800’) between marks. Typically, a TBM should not be greater than two hundred feet (200’) outside the construction limits of the project. All TBMs shall be located and be comprised of sufficient materials such that their integrity will not be compromised throughout the life of the project.

D. Construction Centerline

1. Establish Centerline

The construction centerline location and stationing shall conform to that shown on the Drawings. Any errors found in the line shall be corrected and shown on the specific plan view with reference to the centerline stationing. If control points do not exist they shall be established and referenced so that the line can be readily re-established when required. A minimum of two reference points shall be established to reference each project control point or monument. Each reference point shall be visible to the other reference point. The method of referencing control points shall be done in accordance to the Standard Details of these specifications. Reference points shall be placed at locations where there is the least possibility of their being disturbed during the construction period. Measurements and sketches of the reference points shall be kept in the horizontal control survey field book.

2. Check Existing Ground Profile

A centerline profile shall be run prior to establishing construction grade stakes. The existing ground elevations shall be checked against the existing profile elevations shown on the Drawings to verify design grade relative to the existing ground conditions. The Contractor shall review the centerline profile information and immediately notify the Engineer of any elevations that do not match the plan profile information. The Engineer will direct the Contractor how to proceed.

Article 2.2 Field Notes

The Owner will supply the Contractor with survey field books. Field books will only be issued to the Contractor. No survey Subcontractor will be allowed to check out field books. The Owner has the right to inspect and take possession of the field books at any time throughout the project. Each book shall be indexed and its contents referred to by page number prior to returning them to the Owner. All field books containing field note
information shall be sealed and signed by an Alaskan Registered Professional Land Surveyor on the title page of each field book. The date, weather conditions, survey crew personnel, and instruments used shall be shown at the beginning of each day’s notes. As a general rule, field notes for each phase of the Work shall be placed in a separate series of field books. All field books used for the project shall be submitted to the Owner upon completion of the project.

Field notes shall be neatly logged as follows:

- observations recorded directly in field book.
- notes shall be in pencil.
- notes shall be complete and reduced.
- sketches and traverse data shall be graphic.
- stationing shall increase from the bottom to the top of the page.
- notes shall be precise and sufficiently detailed.

Refer to Section 65.02, Article 2.13 – Electronic Data Collection and Radial Surveys for procedures for logging field notes with the use of electronic data collectors.

Pegging of notes and erasures of information will not be acceptable. A line shall be drawn through those portions of the notes in error leaving the original note legible. The correction shall be noted above the original entry. Corrections shall be initialed and dated. Where appropriate, a note of explanation shall be included.

Field notes shall conform to the note format shown in the Standard Details. All survey Work will be stopped until the notes are brought into conformance with this requirement. A copy of each day's field book notes shall be reduced and delivered to the office of the Engineer by 12:00 Noon the following work day. The Engineer may issue a stop work order at the Contractor's expense until the field notes are delivered within this time frame.

Failure on the part of the Contractor to keep and maintain complete and accurate field notes, as required by this Section, shall be sufficient reason to withhold payment for those items of Work where survey is required. No final project payment will be made to the Contractor until the field books have been submitted and approved by the Engineer.

**Article 2.3 Party Chief’s Daily Diary**

The survey party chief shall keep a factual daily diary of all Work performed by the survey crew on the project. As a minimum, the diary shall contain the following information:

- date.
- crew.
- type and location of Work performed.
- Work accomplished.
- orders from the Engineer.
- signature of Party Chief.
This record shall be kept on the project site and submitted to the Engineer upon request. At completion of the project this dairy shall become the property of the Owner.

**Article 2.4 Clearing and Grubbing Stakes**

The Contractor shall stake the clearing and grubbing limits as shown on the Drawings and/or as directed by the Engineer. If possible, stakes shall be adjusted to avoid sharp breaks in the width of the clearing line. The staking of clearing limits shall be approved by the Engineer prior to the start of the clearing operations.

Distances shall be measured to the nearest foot and standard lath/flagging shall be placed to clearly designate the intended limits. Intervals for placement of lath/flagging shall vary based on the terrain and foliage density, with a minimum of fifty feet (50’) and no greater than one hundred feet (100’) between lath. In areas of heavy timber, clearing stakes shall be placed to avoid leaving trees on the clearing line. If, as the Work progresses, revisions are required to the originally staked clearing distances, the revisions shall be duly noted in the field notes.

**Article 2.5 Cross Sections**

The Contractor shall perform all cross sections necessary for determination of excavation and fill or backfill quantities, including intermediate and/or re-measure cross sections as may be required. Cross sections shall be required before excavation activity begins unless otherwise specified. When clearing and grubbing work is included in the contract the original cross sections shall be taken immediately after grubbing work is complete. Cross sections measured for pay quantities shall clearly identify in the field notes whether the Work was done before excavation or after excavation. When both usable and unusable excavation are a part of the project, the limits of usable or unusable materials shall be clearly identified in the cross sections, in the field book.

**A. Methods and Procedures**

1. **Equipment**

   Cross sections may be accomplished with 1) an engineer's level, 2) a self compensating surveyor's level, or 3) an electronic (laser) level, or 4) by electronic data collection and radial survey method. Neither radial methods nor electronic leveling shall be employed without prior approval from the Engineer. When radial methods or electronic leveling methods are used the survey shall comply with or exceed the accuracy established in this article. Conditions under which these methods may be used shall be discussed at the initial pre-construction meeting with the Engineer. For radial methods see Article 2.13 - Electronic Data Collection and Radial Surveys.

2. **Procedure and Accuracy**

   When an engineering level, self compensating surveyor's level, or an electronic (laser) level is used, cross sections shall be taken perpendicular to
the centerline along tangents and on radial lines along curves. A right angle prism shall be used to determine perpendicul ars. The height of the instruments (H.I.'s) shall be recorded to the nearest hundredth of a foot (0.01'). All cross sectioning work shall be part of a closed level loop. If only one TBM is used the level set-up shall be broken and a different instrument height obtained before closing into the same TBM. The maximum allowable error for level loops used for cross sectioning shall be three hundredths of a foot (0.03'). Cross section readings shall be recorded to the nearest tenth of a foot (0.1'). Horizontal measures shall be recorded and accurate to the nearest tenth of a foot (0.1'). Work shall not be paid for if it does not meet the stated accuracy requirements.

3. Original Ground Measures

Cross section measures of original ground shall be taken at each fifty foot (50') station as indicated on the Drawings. Intermediate stations shall be measured by cross section wherever grade breaks occur. Additional cross sections shall be taken at stations to include quantities measurement of retaining walls, drainage structures, etc. Elevation shots for original ground cross sections shall be taken at the centerline of construction according to the Drawings and as a minimum, at the following points perpendicular to and on each side of the centerline:

- grade breaks
- edge of pavement
- curb and gutter
- shoulder of road
- toe of slope
- centerline of ditch
- top of bank
- all other physical features within the project limits.

In areas where overbreak or slides are anticipated, sections shall be extended out from centerline to include the anticipated disturbed ground area.

4. After Excavation Measures

Cross sections shall be taken at the same stations as the original ground cross sections. Elevation shall be for the bottom, sides and top of excavation at the following points on each side and perpendicular to the centerline:
• centerline
• grade breaks
• toe of excavation
• top edge of cut
• original ground at a minimum of ten feet (10') beyond the limits of excavation.

Work not meeting these requirements shall not be accepted by the Engineer for payment.

B. Notification Prior To Cross Section Work

The Contractor shall notify the Engineer twenty-four (24) hours prior to conducting any survey measurements involving pay quantities. The Contractor shall obtain approval of the excavation from the Engineer prior to taking cross sections and shall provide the Engineer the opportunity to be present during the survey. Pay quantity Work done without the Engineer's notification and approval, or any Work covered up before proper remeasure is made, shall be just cause for non-payment.

Article 2.6 Slope Stakes

Slope stakes shall be required for each cross section station and at additional intervals such as points of curvature and tangency of curves, street intersections, vertical curve intermediate stations to include the high or low point of the curve, and at grade breaks. The stakes are to be set at points where the cut or fill slopes intersect the surface of original ground.

Staking notes shall record the location of the slope stake in relation to the construction centerline, the existing elevation shot at the catch point, the planned elevation that the slope stake is identifying, what level of the design prism the catch point is identifying (i.e., top of unclassified fill, top of subbase, etc.), the percent of slope for cut/fill, the distance to point slope staked, and the station of the slope stake.

The information to be shown on a slope stake is as follows:

• distance from the catch point to the point being staked.
• percent of slope of the cut/fill.
• amount of cut/fill.
• stake's location in reference to the centerline.
• centerline station of the slope stake written on the back of the stake.

The use of hand levels for setting slope stakes shall be limited to one turning point up or down from the instrument to the catch point. Hand level turning points shall be clearly noted in the field book.

A reference stake shall be set for each slope stake. The reference stake shall be set a minimum of ten feet (10’) and a maximum of fifteen feet (15’) beyond the slope stake. The reference stake shall re-state the slope stake information in the event the slope stake is
disturbed or destroyed. A hub shall be driven flush with the ground at the reference stake and all elevations and distances referenced to the hub.

**Article 2.7 Grade Stakes**

A. **Cut or Fill Stakes**

Vertical cut/fill stakes shall be used where the design prism does not contain sloped shoulders and ditches and a slope stake would not be needed. The cut/fill stake shall be comprised of a standard wooden hub driven flush with ground surface and accompanied by a guard lath with the following information written on it:

- amount of cut or fill
- distance to the point of cut/fill from the hub
- description of the cut or filled type, i.e. subgrade, top classified
- offset distance from construction centerline to the cut/fill point
- centerline station written on the back of the lath of cut/fill point
- elevation of the top of hub.

Cuts shall be given to the nearest tenth of a foot (0.1’). Elevations of the top of hubs shall be given to the nearest hundredth of a foot (0.01’). Stakes shall be required at each fifty foot (50’) station identified on the Drawings and at additional intervals such as points of curvature and tangency of curves, street intersections, vertical curve intermediate stations to include the high or low point of the curve, and at grade breaks. A record of the cut/fill, the design grade, the distance offset from centerline, the centerline station and the type of cut/fill being staked shall be written in the survey field book.

B. **Finish Grade**

Grade hubs shall be set to verify that the road prism is at the correct elevation prior to the placement of leveling course material. Wooden hubs, painted or topped with colored whiskers, shall be set at the top of classified fill, within two hundredths of a foot tolerance (0.02’). Stationing shall be fifty feet (50’) on tangent and twenty-five feet (25’) on curves unless the Engineer approves otherwise. All grade breaks, vertical curve intermediate points to include the high/low point of the curve, PC and PT of horizontal curves, and street intersections shall be staked.

Hubs shall be established on the centerline of the road prism as a minimum where poured curb and gutter is incorporated into the designed road prism. Otherwise, hubs shall be established at the shoulder of the designed road prism, as well as the centerline of the road prism.

When parking aprons are staked, hubs shall be set on a fifty foot (50’) grid pattern unless approved otherwise by the Engineer. The field book shall contain the centerline station, the design finish grade elevation of the point staked, the elevation of the hub, and a description of the material being staked.
Article 2.8 Drainage Facilities

The location, type, size, length, and invert elevations for drainage facilities are given on the Drawings. Minor changes in locations and grades to meet existing field conditions may be made where necessary, but only with the approval of the Engineer. If the planned design grade is found to be unworkable in the field, the Engineer shall be notified immediately and all grade staking of the facility shall cease until further notice from the Engineer.

A. Storm Drains, Cleanouts, Outfalls, Catch Basins, Oil and Grease Separators, Culverts

A ground line profile shall be run directly above the centerline of the pipe before trenching occurs. The line and grade for storm drain pipe shall be given from reference hubs offset from each manhole, catch basin, angle point, outfall or cleanout. Reference hubs for culvert installation shall be offset from the pipe ends on the extended centerline of the culvert. One reference hub is required at each end of a culvert. Guard stakes shall be provided for each hub and shall identify the following information:

- station
- size, length and type of pipe
- the amount of cut or fill from the top of the hub to the invert at the end of the pipe
- the horizontal distance from the reference hub to the center of a manhole, cleanout, catch basin, angle point in a pipe, outfall or end of a culvert pipe.

For each structure, the field book shall show the location, type, and size of the structure with a staking diagram showing all distances and pertinent elevations. Two (2) reference hubs shall be set for each manhole, cleanout, catch basin, angle point, and outfall. The reference hubs shall be offset no greater than twenty-five feet (25') from the facility they are referencing.

B. Headwalls

Headwalls for storm drains and culverts shall be staked by setting a hub accompanied by a guard stake on each side of the storm drain or culvert. The hubs shall be on line with the face of the headwall, or as directed by the Engineer. An elevation shall be established on the hubs and written on the guard stake along with the offset distance to the center of the headwall.

C. Dikes and Ditches

Dikes/ditches shall be staked to the alignment, grade and slopes shown on the Drawings. Dikes/ditches shall be slope staked to the shoulder or flow line of the improvement with distances referenced to the improvement centerline. The criteria outlined in Article 2.6 – Slope Stakes shall govern the establishment of slope stakes for this Work.
D. Riprap and Slope Protection

All rip rap and slope protection shall be staked as soon as possible after the pipe, fill, channel change or dike has been constructed. Slope stakes shall be set if needed. See Article 2.6 – Slope Stakes for slope staking criteria.

E. Curb and Gutter

Reference stakes shall be set at even fifty foot (50’) stations on tangents as shown on the Drawings. Horizontal curves shall be staked on even twenty-five foot (25’) stations. All grade breaks, PVCs, PVTs, low points and high points on vertical curves shall also be staked. A hub and tack shall be set at an offset distance of three feet (3’) to the top back of curb. A lath will be set behind the hub and tack with the offset distance marked below the offset and the station marked on the back of the lath. The cut and fill will be to the top back-of-curb within three hundredths of a foot (0.03’). All radius points at curb returns will be staked and additional stakes set breaking up the arc of the curve between curb returns. If valley gutters are to be built, they shall be staked and referenced.

Article 2.9 Water Systems

The Contractor shall stake in the field the alignment and grade for Work to be done under the Contract. Two (2) offset hubs and lath shall be set for each tee, hydrant, water service, valve, angle point, and grade break in the alignment. The lath shall identify the feature being staked and state the elevation of the hub, the offset distance to the center of the feature, and the station of the feature as shown on the Drawings. The offsets shall be set at a reasonable distance to protect them from disturbance.

The Contractor shall be responsible for, and pay all costs for, the transfer of the control points from the reference hubs to such hubs or batter boards as required for the prosecution of the Work. An original ground line profile directly above the water line shall be run prior to excavation. The ground line profile refers to the elevation of the ground directly above the centerline of pipe and the grade line refers to the elevation of the bottom of pipe, except where otherwise noted. The field notes shall record the profile, the hub elevations, offset of the hubs, and the station of the feature being staked.

Article 2.10 Sanitary Sewer Systems

Line and grade for sanitary sewer pipe shall be given from a minimum of two reference hubs for each manhole, outfall or cleanout. Guard stakes shall be provided for each hub showing the information necessary to construct the facility. The minimum information to be shown on the reference stakes and in the field book is as follows:

- centerline of pipe station.
- size and type of pipe.
- cut or fill from the hub to the invert at the end of the pipe.
- offset distance from the hub to the end of the pipe or center of the structure.
Article 2.11 Major Structures

Construction survey procedures shall be reviewed by the Engineer prior to commencing any construction staking. The Engineer’s review and approval of survey procedures is required prior to commencing construction activities for major structures including bridges, docks, piers, piling foundations, drainage control facilities and large buildings.

Horizontal and vertical control for the project shall be verified by the Contractor prior to any construction activity. The Contractor shall verify existing field elevations where planned foundations, pilings, piers and support structures are to be placed prior to any construction activity. The Contractor shall verify depth of water and existing ocean or lake bottom elevations for all dock and pier construction prior to commencing pile driving and excavation activity. If any discrepancies are found between the Contract Documents and existing conditions the Contractor shall inform the Engineer immediately.

Article 2.12 Miscellaneous Construction

The Contractor shall provide sufficient stakes for adequate control of all structures and incidental construction not specifically covered above. A staking diagram with respect to centerline and measurements for pay quantities shall be maintained in the field notes. Other items such as horizontal and vertical control shall be shown in the field book and shall be governed by procedures established in previous articles of this Specification.

Article 2.13 Electronic Data Collection and Radial Surveys

Data gathered by electronic data collection or by radial methods shall be submitted in AutoCAD drawing file format to be determined by the Engineer. The Contractor shall be guided by the following specifications:

A. A standard field book shall be used to record the date of survey, weather conditions, instrumentation and data collector used, crew, project description and sketches, listing of horizontal and vertical control points used and established, and other information needed to set up the reconstruction of the survey.

B. A printout of the unedited output from the data collector or a copy of the radial field book entries to include: code descriptors, horizontal circle information, vertical circle information based on zenith, and slope distance expressed in feet. A sheet containing the explanation of the codes used to identify the various shots.

C. A printout of the reduced and adjusted data represented by point number, station left or right of centerline, elevation, descriptor and coordinates of the point.

D. A plot drawing, showing the control points used, points occupied and the radial observations expressed by point number.
E. All cross section data shall be submitted in an unedited points file so it can be independently run through a DTM program by the Engineer.

F. A cross section plot of each station shall be submitted to the Engineer for verification showing the following information:

- centerline or control line and station.
- point of elevation and offset from centerline.
- design grade road template with superimposed before and after excavation surfaces.
- quantity of cut or fill expressed in cubic yards.
- summary table of each sections cut or fill and total amount of quantities expressed in cubic yards.

**Article 2.14 As-built Surveys and Record Drawings**

As-built survey measurements shall be required for all constructed facilities and improvements to confirm the dimensions, lines, grades, locations, or materials as shown on the Drawings. Survey measurements shall be taken, field notes shall be kept, and accuracy shall be attained in accordance with this Division. As-built information shall be marked on a clean set of full-size paper copy Drawings and be submitted to the Engineer at the completion of construction activity. When Record Drawings are to be submitted on the original mylar Drawings, the As-builts recorded on the paper copy Drawings shall be transferred to the mylar and both the paper copy and mylars submitted to the Engineer. The Drawings shall be clearly stamped "Record Drawings." No final project payment will be made to the Contractor until the Record Drawings have been submitted to and approved by the Engineer.

The following abbreviations shall be used on the Record Drawings to denote a deviation from the Drawings:

- **ASB "As-Built"** - The actual horizontal, vertical, dimension, or quantity measured by survey after it has been constructed.
- **F.C. "Field Change"** - Revision or change of original design made in the field.
- **"DELETED"** - Not constructed.

Minimum requirements for construction of Record Drawings:

- When original mylar Drawings are used for Record Drawing purposes, all As-built information shall be in drafting ink and all information shall conform in size, type, and scale to the original. No stick-on information adhesives shall be accepted on the original mylars submitted for filing of Record Drawings.
• When paper copies are used for record Drawing purposes, As-built Work shall be marked in red ink or red pencil to clearly identify the changes to the original design.

• A straight line drawn through stationing, elevations, and notes shall show a change, deletion, or omission and shall be followed with the appropriate symbol.

• Storm sewer, water, sanitary sewer, gas lines, or any construction that has been deleted or relocated will be crosshatched.

• Crossed out information should still remain legible.

• The scale of new gas lines, water, sewer, or any new construction not shown should conform to the scale of the drawings.

• Reference information used to prepare Record Drawings, such as change orders, and field books, shall be noted on the drawings.

• Profile changes will be made with elevations or stationing only. The profile line need not be re-drawn unless the change is significant.

• As-builts for water, sewer, gas lines, and storm drain systems shall be accurate within three-hundredths feet (0.03’) vertically and one-half feet (0.5’) horizontally. As-built Information shall be referenced to existing subdivision survey control and/or monumented centerline of the right-of-way control.

• As-builts for structures shall be accurate to within one-half inch (1/2”) vertically and horizontally.

• The name of the Record Drawing preparer, the employer, and the date of the preparation shall appear in the appropriate title block on each Record Drawing sheet.

The construction of Record Drawings is incidental to other Work and no measurement or payment shall be made.

**Article 2.15 Method of Measurement**

The method of measurement for surveying services shall be a lump sum cost item on the bid schedule. The lump sum cost for Construction Survey Measurement shall include all project control, project staking, and quantities measurement for payment by unit price Work as required by the Contract Documents.

The measurement for Existing Monument and Lot Corner Search shall be a lump sum bid item measured at the time of completion of the establishment of project staking of centerline control. Contractor shall submit field book notes to the Engineer for the Owner’s review and approval of the pay item.
For bid schedule item, Two-Person Survey Crew, the measurement will be the cost per hour for a crew. The item, Two-Person Survey Crew, shall be used only for extra, additional, or unanticipated Work required for changes in the project as directed by the Engineer. Additional survey Work requiring one survey person shall be paid at forty-five forty-five percent (45%) of the bid amount per hour of a two-person crew. The item One-Person Survey Crew shall be used only for extra, additional, or unanticipated Work required for changes in the Project as directed by the Engineer.

For bid schedule item "Survey Monument Installed," the measurement shall be the cost to purchase the materials and install a monument per Article 2.1, SubArticle B.4 – Standard Monument and Monument Case Specifications. When the bid schedule contains an item "Survey Monument Installed in Monument Case," the measurement shall be the cost to purchase the materials and install a monument in a monument case, per Article 2.1, SubArticle B.4 – Standard Monument and Monument Case Specifications.

Computer time is incidental to other Work and will not be measured. Certified payrolls and daily time records are required for all Work to be measured by the hour and survey monuments installed.

**Article 2.16 Basis of Payment**

Payment for this item shall be in accordance with Division 10, Section 10.07 - Measurement and Payment and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
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<tbody>
<tr>
<td>Construction Survey Measurement</td>
<td>Lump Sum</td>
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<tr>
<td>Survey Monument Installed in Monument Case</td>
<td>Each</td>
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<tr>
<td>Survey Monument and Lot Corner Installed</td>
<td>Each</td>
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<tr>
<td>Two-Person Survey Crew</td>
<td>Hour</td>
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<tr>
<td>Existing Monument and Lot Corner Search</td>
<td>Lump Sum</td>
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MUNICIPALITY OF ANCHORAGE
STANDARD SPECIFICATIONS

DIVISION 65
CONSTRUCTION SURVEY
STANDARD DETAILS
STANDARD CONSTRUCTION SPECIFICATIONS
MISCELLANEOUS
DIVISION 65
INDEX OF STANDARD DETAILS

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</table>

**Description**

- Begin retracement & establishment of "P-line"
- Complete "P-line" & determine falling at Reeve Blvd.
- Tie property corners from "P-line"
- Establish & Construction from "P-line"
- "
- Slope stakes S.E. corner Third & Post
- R.P. 20+00 @ Nechima, 26+50 & 31+0335
- R.P.'s & S.I.'s
- Luminaires
- R.P. & S.I. 3rd & Post
- Stake Load Center
- Curb & Gutters Staking
- Stake Load Center & Luminaires
- J-Boxes, Post & Third, Concrete St. & Third grades
- P.P. Elevations
- Storm Drain MH
- Before & after Ex X-5
- As Built Curb
- Curb & gutter Rt. Side 17+18 to Nechima
- Restake 24G on Nechima St.
- Curb & gutter 3rd Ave.
- Curb & gutter Post Rd.
West 42nd. Ave. (Place)  
Horizontal Control  
Nov. 13, 1986 Clear±30°F  
T: 2000 Layout Rod  

3" φ Brass Mon.  
1" below surface  
Slightly scarred  

30.00 30.00  

Green Valley Unit No. 3 Blk. 1  
All these #4 rebar were  
bent and in poor condition  

1320.20 @ 90°-50-04  
1320.06 Mea. W-West 42nd  
(1321.11 Rec.)  

There were no lot corners  
found along this property  
line, west end of block  

Green Valley Subd.  
Unit No. 3 Blk. 2  

3 1/4" B. C.  
0" Below Asphalt  
6" I.D. Case  

30.00 30.00 30.00 30.00 30.00 30.00  

1. 270° 00' 58"  
2. 180° 01' 42"  
M. 270° 00' 53"  
4. 00° 09' 46"  
M. 270° 00' 56"  

14 13 12 11  

TNI  
5/8" x 30" RB  
φ 2" AL CAP  
Set: Jan. 14, 1867  
THOMAS CARTER CLINE  

Legend:  
△ #4 Rebar  ○ #5 Rebar  

NOTE: Ref. CofA FB 1898  
pg. 2-5  
for & control Turnagain Blvd.
TBM LOOP FOR 100th AVE. TOPO
Note: We peg our level immediately before beginning this loop, check ±0.0005 in 400'
Wild NA\(^3\) #502261

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<th>-</th>
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### UNIVERSITY DRIVE
#### STAKE EXCAVATION LIMITS

**EXCAVATION LIMITS (FROM C.L.)**

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*Sunny 60°F, Party Chief Chainm., RT & Prism 100' Rag Tape*
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<th>STA.</th>
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Grade Stakes: 5/24/07
Party: Chief - NI-2
Chairsman: Philly Rod
Instrumentman: Rag Tape
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149.63 RT.

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No B.TOP & Rd 142° 54' ✓

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27° Rd No B.TOP

143.59

Elev. from Pg. 11

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C-04° FG 6 15

27° Rd 143° 48' ✓

27°
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## Construction Stakes

### Open Ditch

**Goldenview Dr. 164th to 162nd**

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**Party Chief:** 7-8-85  
**Instrm.:** Sunny 65°  
**Chainm.:** NI 2  
**Philly Rod:**

---

**Spike in PP A-256-A B9 page 7**

---

**Scale:** NTS  
**Detail #:** 65-11  
**Section #:** 65-11  
**Detail #:** 65-11  
**Scale:** NTS  
**Municipality:** Anchorage  
**Revised:** 10/07  
**Approved:** NTS  
**Details:**

---

**Offset:**

---

**1st:**

---

**2nd:**

---

**3rd:**

---

**4th:**

---

**5th:**

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<tr>
<td>6.69' 18'&lt;br&gt;84.79F - 0.57</td>
<td>5.69' 18'&lt;br&gt;85.79C - 0.07</td>
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<td><strong>7.56'</strong>&lt;br&gt;18'</td>
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Note: Hub/Tack set 0.3 offset to TBC.

TBM STA  + 91.48 RT.  - 8738 N. Bolt ELEV.

Party Chief: NI-11
Instrm.: 25' Glass Rod
Chainm.: 100' Rag Tape

F.M. E Side Timothy - 100' of Sundew Cir
MOA FB 1985-4
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<tr>
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<td>General</td>
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<td>Article 19.2</td>
<td>Materials</td>
<td>55</td>
</tr>
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<td>Article 19.3</td>
<td>Construction</td>
<td>55</td>
</tr>
<tr>
<td>Article 19.4</td>
<td>Measurement</td>
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<tr>
<td>Article 19.5</td>
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SECTION 70.01    GENERAL

Article 1.1    Scope of Work

The Work covered by these Specifications consists of providing all plant, labor, equipment, supplies, materials, transportation, handling and storage, and performing all operations in connection with the adjustment and/or construction of miscellaneous facilities as provided in this Division.

Article 1.2    Applicable Standards

The latest revision of the following standards of the American Society for Testing and Materials (ASTM) and the American Association of State Highway and Transportation Officials (AASHTO) are hereby made a part of these Specifications.

ASTM A-112    Specification for Zinc-Coated (Galv.) Steel Tie Wires
ASTM A-120    Specification for Black and Hot-Dipped Zinc-Coated (Galv.) Welded and Seamless Steel Type for Ordinary Uses
ASTM A-121    Specification for Zinc-Coated (Galv.) Steel Barbed Wire
ASTM A-153    Specification for Zinc-Coated (Hot Dip) on Iron and Steel Hardware
ASTM A-227    Specification for Hard-Drawn Steel Spring Wire
ASTM A-307    Specification for Low-Carbon Steel Externally and Internally Threaded Standard Fasteners
ASTM A-392    Specification for Zinc-Coated Steel Chain Link Fence Fabric
AASHTO M-133    Specification for Preservatives and Pressure Treatment Processes for Timber
AASHTO M-145    Classification of Soils
AASHTO M-180    Specification for Corrugated Sheet Steel Beams for Highway Guardrail
SECTION 70.02  ADJUST GAS VALVE KEY BOX TO FINISH GRADE

Article 2.1  General

The Work under this Section consists of providing all operations pertaining to adjustment of existing gas valve key boxes to finish grade. The Contractor must contact ENSTAR’s Distribution Department prior to starting any Work that includes or is adjacent to the gas valve key box.

Article 2.2  Material

ENSTAR will furnish all materials to adjust gas valve key boxes to finish grade.

Article 2.3  Construction

The Contractor shall adjust gas valve key boxes in accordance with the applicable Standard Details, unless otherwise directed by the Engineer. Any damage to gas valve key boxes resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor's expense. All gas valve key box adjustments will be accomplished as requested by the Engineer. The Contractor shall be responsible for ensuring that the gas valve key box is vertical, clean, to proper grade, and readily accessible for operation of the valve.

Contractor shall adjust the service key box to finish grade prior to placement of asphalt pavement. After-the-fact cutting of new asphalt for adjustments is not accepted. Any adjustment(s) requiring cutting of new asphalt shall not be paid and shall be deducted from the plan quantity.

Article 2.4  Measurement

Adjustment of gas valve key boxes will be measured per unit, complete in place.

Article 2.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Gas Valve Key Box to Finish Grade</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.03 ADJUST GAS VALVE MANHOLE TO FINISH GRADE

Article 3.1 General

The Work under this Section consists of providing all operations pertaining to adjustment of existing gas valve manholes to finish grade. The Contractor must contact ENSTAR's Distribution Department prior to starting any Work that includes or is adjacent to the gas valve manhole.

Article 3.2 Material

ENSTAR will furnish all materials, except mortar, to adjust gas valve manholes to finish grade.

Article 3.3 Construction

The Contractor shall adjust gas valve manholes in accordance with applicable Standard Detail, unless otherwise directed by the Engineer. Any damage to gas valve manholes resulting from construction under this Contract shall be repaired or the damaged portion replaced at the Contractor's expense. The Contractor shall be responsible for ensuring that the valve box is vertical, clean, to proper grade, and readily accessible for operation of the valve.

Contractor shall adjust the gas valve manhole to finish grade prior to placement of asphalt pavement. After-the-fact cutting of new asphalt for adjustments is not accepted. Any adjustment(s) requiring cutting of new asphalt shall not be paid and shall be deducted from the plan quantity.

Article 3.4 Measurement

Adjustment of gas valve manholes will be measured per unit, complete in place.

Article 3.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Gas Valve Manhole to Finish Grade</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.04 ADJUST ELECTRIC/TELEPHONE MANHOLE

Article 4.1 General

The Work under this Section consists of providing all operations and materials required for the preparation and adjustment of electric/telephone manhole lids, frames, and rings to finish grade.

Article 4.2 Materials

All concrete and cement used in the adjustment of electrical/telephone manholes shall conform to the requirements for manholes as specified in Division 55, Section 55.05 - Manholes and Catch Basin Manholes.

The Contractor may utilize Neenah Manhole Adjusting Rings P1979-077, Part No: 19790053, Catalog 1797-01, or an approved equal, for adjusting the electrical and telephone manhole to finish grade.

Article 4.3 Construction

All manholes to be adjusted shall be inspected by the Contractor, the Engineer, and the applicable utility's representative to verify size, condition, and any necessary replacement of the existing lids. Inspection, replacement, and cost of lids will be considered incidental to the Contract and no separate payment shall be made. Manholes may be adjusted by installing grade rings and/or grouting. Manhole adjustment by grouting shall consist of bringing the manhole grade ring and lid to final grade, then grouting underneath the ring. The Contractor shall have an assortment of adjustment rings of various thicknesses on the project site to preclude after-the-fact asphalt cutting for adjustment.

After-the-fact cutting of new asphalt for adjustments will not be accepted; rings will be inventoried before authorization to pave is given. Any utility adjustments requiring cutting of new asphalt will not be paid and will be deducted from the plan quantity.

The Contractor shall contact the appropriate utility at least forty-eight (48) hours prior to beginning the overlay operation, and to schedule a representative of that utility to be on site to supervise the manhole adjustments to finish grade.

Prior to placement of any grade ring adjustment, the existing seat should be cleaned and all loose material shall be blown out or wire brushed to ensure a proper fit.

Article 4.4 Measurement

Adjustments of electric/telephone manholes to finish grade shall be measured per unit, complete in place, and adjusted to the required grade.
Article 4.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Electric Manhole (Type)</td>
<td>Each</td>
</tr>
<tr>
<td>Adjust Telephone Manhole (Type)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.05  ADJUST ELECTRICAL VAULT

Article 5.1  General

The Work under this Section consists of performing all operations pertaining to materials, equipment, and personnel required for the preparation and adjustment of a high voltage electrical vault to finish grade. The high voltage electrical vault is typically located within a sidewalk adjacent to a building or in an alleyway and is specifically located on the Drawings.

Article 5.2  Materials

All Portland Cement Concrete utilized in the adjustment of the electrical vault shall conform to the requirements as specified in Division 55, Section 55.05 - Manholes and Catch Basin Manholes. The joint sealing compound utilized to seal the joint between the electrical vault's lid and walls shall be premolded plastic gasket or an approved equal.

Article 5.3  Construction

No later than forty-eight (48) hours prior to commencement of Work on adjustment of the electrical vault, Contractor shall contact the Utility Line Superintendent of ML&P. This vault contains energized high-voltage circuits and all Work in and immediately surrounding the vault shall be monitored and supervised by a Journeyman Power Lineman with a current State of Alaska Certificate of Fitness. The Contractor shall be responsible for protecting the Contractor's personnel and the general public from the open vault as well as from the hazardous high voltages present within the vault.

The electrical vault lid to be adjusted typically contains two manhole frames and covers and forms an integral part of the sidewalk, alleyway, or other finished surface. The vault lid shall match the final finish grade of the finished surface in which it is installed. Any proposed adjacent curb shall be in accordance with Standard Detail 30-1 as identified on the Drawings. To lower the vault lid, the Contractor shall remove a portion of the lid in order that the top of the vault lid match the proposed top back of curb elevation with a two percent (2%) transverse slope. The existing area of contact between the vault lid and vault walls is typically a rabbet joint and not a flat surface.

Prior to removal of the electrical vault lid, Contractor, Engineer, and an ML&P representative shall inspect and verify the condition of the vault lid and vault structure. After verification of condition, Contractor shall submit a drawing detailing how the vault lid will be lowered. The drawing shall be approved in writing by ML&P. The vault lid shall be adjusted by cutting and removing a portion of the vault lid. The vault lid shall be cut to match the existing vault wall rabbet joint. The rabbet joint shall be sealed to provide a watertight seal.

Prior to replacement of the vault lid, the vault lid and vault structure shall be inspected by Contractor, Engineer, and an ML&P representative to verify adjustments. Any Work, personnel, and/or materials required to properly correct problems shall be at Contractor's
expense. After ML&P’s written receipt of approval, Contractor shall reset the vault lid at the correct adjusted elevation and grade.

Contractor may propose an alternate adjustment method. This alternate method must be submitted to an ML&P representative in writing. ML&P shall have sole discretion on the approval of the Contractor’s proposed alternate method. If an alternate method is approved, no added or separate payment shall be made.

**Article 5.4 Measurement**

The method of measurement for all Work in this Section shall be a lump sum. The lump sum cost for adjusting the electrical vault to finish grade shall include all labor, materials, and equipment. The bid item shall include all required usable and unusable excavation, classified fill and backfill material, compaction, concrete cutting and removal, vault lid removal and replacement, traffic control, and required personnel.

**Article 5.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Electrical Vault</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.06 ADJUST UTILIDUCT LID

Article 6.1 General

The Work under this Section consists of all operations pertaining to the adjustment, either up or down, of an existing ACS/ML&P concrete utiliduct lid as directed by the Engineer.

Article 6.2 Materials

All Portland Concrete Cement utilized in the adjustment of the utiliduct lid shall conform to the requirements as specified in Division 55, Section 55.05 - Manholes and Catch Basin Manholes.

Article 6.3 Construction

The utiliduct to be adjusted typically consists of a dual channel concrete structure with a structural concrete top/lid. Each utiliduct lid section is typically approximately five feet (5') in width and eight feet (8') in length. Prior to adjustment of the utiliduct lid, Contractor, Engineer, and a representative from both ACS and ML&P shall inspect and verify the condition of the utiliduct lid and utiliduct structure. After verification of condition, Contractor shall utilize the method of adjustment in accordance to the Utiliduct Lid Adjustment Detail available from ML&P. The utiliduct lid shall match the finished surface in which it is installed.

Contractor may submit an alternative method adjustment to the Engineer detailing how the utiliduct lid will be adjusted. The alternative method of adjustment shall not reduce the existing load rating of the utiliduct and utiliduct lid. The alternative method of adjustment of the utiliduct lid shall be designed, stamped, and signed by a registered professional engineer licensed by the State of Alaska. Contractor shall submit an ACS and ML&P approved substitution request in accordance with Division 10, Section 10.05, Article 5.7 - Materials. All costs associated with preparing the design of an alternative utiliduct lid adjustment and obtaining the necessary utility approvals prior to submitting the substitution request shall be considered incidental to this item and no additional payment will be made.

No later than forty-eight (48) hours prior to commencement of Work on adjustment of the utiliduct, Contractor shall contact both the Outside Plant Construction Supervisor of ACS and the ML&P Line Superintendent. The utiliduct contains telephone and energized high-voltage circuits. All Work in and immediately surrounding the utiliduct shall be monitored and supervised by a Journeyman Power Lineman with a current State of Alaska Certificate of Fitness. The Contractor shall be responsible for protecting Contractor’s personnel and the general public from the open utiliduct, as well as the hazardous high-voltages and telephone lines present within the utiliduct. Should ACS’s and/or ML&P’s cables be damaged, ACS and/or ML&P will install new cables at Contractor’s expense.

Contractor shall adjust the utiliduct lid to finish grade prior to placement of asphalt pavement. After-the-fact cutting of new asphalt for adjustment(s) will not be accepted. Any adjustment requiring cutting of new asphalt will not be paid and will be deducted from the plan quantity.
Article 6.4  Measurement

The method of measurement for all Work in this Section shall be measured in linear feet along the top face at the centerline of the utiliduct, complete in place, and adjusted to finish grade. The bid item shall include all required material, usable and unusable excavation, classified fill and backfill, compaction, Portland Concrete Cement, traffic control, required personnel, and equipment.

Article 6.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Adjust Utiliduct Lid</td>
<td>Linear Feet</td>
</tr>
</tbody>
</table>
SECTION 70.07   REMOVE PIPE

Article 7.1  General

The Work under this Section consists of performing all operations pertaining to the removal and disposal or salvage of existing pipes (of whatever size of pipe encountered), when encountered in the excavation and/or as directed by the Engineer.

Article 7.2  Construction

Contractor shall remove salvageable pipes and deliver the pipes to a location as directed by the Engineer. Contractor shall provide a disposal site for non-salvageable material in accordance with the provisions of Division 10, Section 10.04, Article 4.9 - Disposal Sites.

Excavation required in the removal of the pipes is incidental to this bid item. Contractor shall backfill the excavation with suitable, non-frost-susceptible materials and compact it to not less than ninety-five percent (95%) of maximum density as directed by the Engineer.

Article 7.3  Measurement

Removal of pipes is measured per linear foot without regard to pipe size. Removal of electrical conduit of whatever size and type is incidental to the Contract, unless provided for elsewhere in the Contract.

Article 7.4  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Pipe</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 70.08  RESET FENCE

Article 8.1  General

The Work under this Section consists of providing all operations pertaining to removing, storing, and resetting existing fence whatever height and type of fencing material as indicated on the Drawings or as directed by the Engineer.

Article 8.2  Material

All materials which can be reused shall be salvaged from the existing fence. Those materials which cannot be salvaged or are damaged by the Contractor's operations shall be replaced, at the Contractor's expense, with new materials which as nearly as possible duplicate the kind and quality of materials in the original installation.

Nails, staples, fastening wires or devices, and all materials required for the construction of such anchors, end posts or other portions of the fence which can be replaced more efficiently than they can be moved, shall be furnished by the Contractor.

If the property owner elects to replace any of the existing fencing materials with other materials in better condition, he shall furnish and deliver such materials to the site of the Work, upon the approval of the Engineer and the Contractor.

Article 8.3  Construction

The fence shall be set in close conformity with the property line shown on the Drawings or as directed by the Engineer. Posts and anchors shall be set at the same depth and spacing as in the original fence. Wire shall be drawn taut but care shall be taken to avoid over-stressing the salvaged materials. Permanent anchors, end posts or other parts which cannot be economically moved shall be replaced by equivalent construction. If any new materials require painting, they shall be painted to match the original materials as nearly as possible. If a match cannot be attained to the satisfaction of the Engineer, the entire fence will be painted. The reset fence shall be placed in at least as good condition as the existing fence before it was moved.

Article 8.4  Measurement

Resetting fence will be measured by length in linear feet, complete and accepted in its final position.
Article 8.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove and Reset Fence</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Reset Fence</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 70.09  RESET PARKING METERS

Article 9.1  General

The Work under this Section consists of performing all operations pertaining to furnishing all equipment, materials, and personnel to remove and reset existing parking meter post assemblies.

Article 9.2  Construction

All posts shall be placed in excavated holes. Depth of embedment shall be as shown on Drawings, unless otherwise directed.

Surplus excavated material shall be disposed of by the Contractor and shall be incidental to this bid item.

All materials and finished parking meter installations are subject to inspection and acceptance in place by the Anchorage Parking Authority. Contact the Anchorage Parking Authority twenty-four (24) hours prior to removal.

Article 9.3  Measurement

Removal and resetting of existing parking meters shall be measured per each parking meter post assembly, reset and accepted in final position. Parking meter and post components damaged or destroyed due to the Contractor’s operation shall be replaced by the Contractor at no additional expense to the Owner. One post equipped with two or more parking meters shall be considered a single parking meter assembly.

Article 9.4  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove and Reset Parking Meter</td>
<td>Each</td>
</tr>
</tbody>
</table>
 SECTION 70.10 TRAFFIC MARKINGS

Article 10.1 General

The Work under this Section shall consist of performing all operations pertaining to furnishing all materials; placing painted and methyl methacrylate traffic markings and applying glass spheres thereto; and retroreflective preformed patterned pavement tape traffic markings. Contractor shall provide all Work in accordance with these specifications; at the locations shown on the Drawings; the Manual of Uniform Traffic Control Devices (MUTCD), and the Alaska Traffic Manual.

Article 10.2 Materials

A. Paint for Traffic Markings - General Requirements

1. The Contractor shall furnish the name of the company that will manufacture the paint and the location of the plant from where shipments will be made. No material shall be shipped by the manufacturer until it has been sampled, tested, and approved.

2. Traffic Lane Paint shall conform to the current State Specifications.

B. Glass Spheres for Reflectorizing Highway Pavement Markings

Reflective Glass Beads shall conform to the current State Specifications, and shall be supplied with a moisture-resistant coating.

C. Retroreflective Preformed Patterned Pavement Tape for Traffic Markings

1. General Requirements:

   a. Contractor shall furnish retroreflective preformed patterned pavement tape traffic markings which are manufactured and formulated from new material and are free from defects and imperfections that might adversely affect the serviceability of the finished product. Contractor shall furnish pavement traffic markings free from dirt and other foreign material such as, but not limited to, surface oils or existing road marking material.

   b. The preformed patterned markings shall consist of white or yellow films with clear and/or yellow-tinted microcrystalline ceramic beads incorporated to provide immediate and continuing retroreflection. These films shall be manufactured without the use of lead chromate pigments or other similar, lead-containing chemicals.

   c. Preformed words and symbols shall conform to the applicable shapes and sizes as outlined in the Manual on Uniform Traffic Control Devices for Streets and Highways (MUTCD) and the Alaska Traffic Manual.
d. The preformed markings shall adhere to asphaltic cement concrete and Portland cement concrete by a pre-coated pressure sensitive adhesive. The pavement traffic markings shall be capable of application on new, dense and open-graded asphalt concrete wearing courses during the paving operation in accordance with the manufacturer’s instructions. Where the asphalt concrete wearing course is Class A asphalt concrete pavement, the pavement traffic markings shall be applied in grooves cut into the pavement. After application, the markings shall be immediately ready for traffic. Contractor shall furnish the proper surface preparation adhesives to be applied at the time of application, all equipment necessary for proper application, and manufacturer’s recommendations for application that will assure effective product performance.

e. The markings shall be highly durable, retroreflective, pliant polymer materials designed for longitudinal, transverse, and symbol/legend markings subjected to high traffic volumes and severe wear conditions such as shear action from crossover or encroachment on typical longitudinal configurations such as edge lines and lane lines and typical transverse configurations such as stop bars and crosswalks.

f. Thickness: The patterned material without adhesive shall have a minimum caliper of 0.065 inches (65 mils) at the thickest portion of the patterned cross-section and a minimum caliper of 0.020 inches (20 mils) at the thinnest portion of the cross-section.

![Thickness Diagram]

0.065 in. (65 mils)    0.020 in. (20 mils)

g. Contractor shall furnish retroreflective preformed patterned pavement traffic markings 3M Stamark Tape Series 380I ES manufactured by:

3M Traffic Safety Systems Division
3M Center, Bldg 0225-05-S-08
St. Paul, MN 55144
Phone: 800-553-1380
Email: www.3M.com/tss

3M Alaska
11151 Calaska Circle
Anchorage, AK 99515
Phone: 907-522-5200
Fax: 907-868-5283

or an approved equal.

2. Performance Requirements:

a. Retroreflectivity. The retroreflective preformed patterned pavement traffic markings shall maintain a minimum retroreflectivity of 100
The retroreflectivity of the areas in question or zones of measurement will then be measured using a portable retroreflectometer conforming to the ASTM E-1710 test method for CEN-prescribed geometry of 1.05° observation and 88.76° entrance angles.

b. Color Stability. The preformed patterned pavement traffic markings shall consist of white or yellow films with pigments selected and blended to conform to standard highway colors.

c. Adhesion. The retroreflective preformed patterned pavement traffic marking shall not have a loss of area or significant movement due to non-adhesion greater than a cumulative ten percent (10%) on any three hundred foot (300') segment of the installation throughout the warranty period. A loss or significant movement greater than a cumulative ten percent (10%) shall constitute an adhesion failure of the material in that segment.

The definition of and the responsibility for adhesion failures are:

- Loss of traffic markings due to excessive wear or damage to the pavement surface such as localized or widespread shaving or scraping of a substantial depth of the top surface of the pavement caused by snow plowing is not an adhesion failure.

- If adhesion failure occurs during the warranty period, then the Contractor shall provide labor and material to replace the failed pavement traffic markings at the discretion of the Engineer.

d. Durability. The retroreflective preformed patterned pavement traffic marking shall not have a loss of area due to lack of durability greater than a cumulative ten percent (10%) on any three hundred foot (300') segment of the traffic marking installation throughout the warranty period. A loss greater than a cumulative ten percent (10%) shall constitute a durability failure of the material in that segment.

The definition of and the responsibility for durability failures are:
- Loss of traffic markings due to excessive wear or damage to the pavement surface such as localized or widespread shaving or scraping of a substantial depth of the top surface of the pavement caused by snow plowing is not a durability failure.

- If a durability failure occurs during the warranty period, then the Contractor shall provide labor and material to replace the failed pavement traffic markings to meet the requirements for retroreflectivity, color stability, adhesion and durability at the discretion of the Engineer.

3. Warranty Requirements

Contractor shall furnish retroreflective preformed patterned pavement tape traffic markings that carries a manufacturer’s twenty-four (24) month warranty. Contractor shall provide a copy of the manufacturer’s warranty to the Engineer a minimum of 5 working days before commencing application of the traffic markings. Manufacturer’s warranty shall provide that all longitudinal pavement traffic markings shall maintain the performance requirements for retroreflectivity, color stability, adhesion and durability as detailed in SubArticle 2 - Performance Requirements, above.

The warranty shall state that the manufacturer and/or Contractor shall repair or replace, at the sole discretion of the Engineer and at no additional cost to the Municipality, all pavement traffic markings that fail to meet the terms of the warranty.

D. Methyl Methacrylate Pavement Markings

1. General Requirements:
   a. Contractor shall furnish Methyl methacrylate traffic markings which are manufactured and formulated from new material and are free from defects and imperfections that might adversely affect the serviceability of the finished product. Contractor shall furnish Traffic markings free from dirt and other foreign material such as, but not limited to, surface oils or existing road marking material, and shall cure to a tough serviceable film within the time specified by the manufacturer.

   b. Methyl methacrylate traffic markings which are a spray-applied, ambient temperature curing, 2-component system for application on either asphalt or cement concrete surfaces. Traffic markings shall be composed of a Part “A” methyl methacrylate based resin and Part “B” benzoyl peroxide in liquid plasticizer. The mix ratio shall be four (4) parts of “A” to one (1) part of “B.”

   c. Glass beads for drop-on applications recommended in writing by the traffic marking material manufacturer and approved by the Engineer.
d. Contractor shall furnish Methyl methacrylate traffic markings Dura-Stripe Type V manufactured by:

TMT – PATHWAY
1675 Commercial Street N.E.
Salem, Oregon 97303
Phone: 800-835-3357
FAX: 800-774-8464

or an approved equal.

Article 10.3 Construction

A. General

This Work shall be done as soon as possible after paving is completed to facilitate traffic.

B. Paint Color

All pavement markings shall conform to the colors shown on the Drawings.

C. Preparation of Surface

Paint will not be applied to pavements which are excessively dirty, damp, or cold. Paint shall not be applied when the pavement temperature is less than forty degree Fahrenheit (40°F). All dirt, oil, grease, and other foreign matter shall be removed from the areas of the pavement upon which the traffic markings are to be painted by a method approved by the Engineer.

D. Types of Lines

The type and color of the lines shall be as shown on the Drawings.

E. Width of Lines

The width and spacing of all lines shall be shown on the Drawings.

F. Application

1. Paint

   a. The paint shall be applied with atomizing spray type striping machine, approved by the Engineer. The markings shall have clear-cut edges, true and smooth alignment and uniform film thickness. The wet film thickness shall be seventeen (17) mils with a nominal variation not to exceed two (2) mils.
b. The wet film thickness of the in-place paint shall be measured as follows:

Convenient to the location where the road service lines will be placed, test lines shall be laid to adjust the pavement-marking machine. In the path of the test line laid without glass spheres, place a weighted sheet of aluminum foil eighteen by eleven inches \((18'' \times 11'')\), thumbtacked to a three-quarter inch \((3/4'')\) plywood board. Immediately after the motorized striper (spraying a four inch \((4'')\) strip along the eighteen inch \((18'')\) dimension of aluminum foil) passes over the aluminum foil, quickly roll it up, slip an elastic band over the roll, and weigh it to the nearest 0.1 gram within thirty (30) seconds from the net weight of paint on the foil and the weight per gallon of the sample, calculate the film thickness using the following formula:

$$\text{Film thickness, in} = \frac{A \times 231}{453.6 \times 18 \times 4 \times B}$$

$$= \frac{A \times 0.007073}{B}$$

Where:  

- \(A\) = Weight of paint on foil in grams.  
- \(B\) = Weight per gallon of sample in pounds.

2. Glass Beads

Glass beads shall be applied over the wet painted stripes in a uniform pattern at the rate of five pounds \((5 \text{ lbs})\) of glass beads per gallon of paint. The bead dispensers shall be of a type that will mechanically and automatically give such performance. Glass beads shall be applied to all painted traffic markings by the drop-on method.

3. Retroreflective Preformed Patterned Pavement Tape Pavement Markings

a. Contractor shall prepare the roadway areas to receive the retroreflective preformed patterned pavement tape traffic markings in accordance with these Specifications and the manufacturer’s recommendations. Contractor shall submit a current copy of manufacturer’s recommendations at least five \((5)\) working days prior to application of traffic markings.

b. Contractor shall be certified by the manufacturer as an approved installer of retroreflective preformed patterned pavement tape traffic markings, or Contractor shall provide the services of a manufacturer’s representative \((\text{Manufacturer’s Representative})\). In the latter case, Contractor shall ensure the Manufacturer’s Representative observes all aspects of the application of the pavement marking materials. In all cases, Contractor shall cooperate with the Manufacturer’s
Representative and the Engineer to ensure the retroreflective preformed patterned pavement tape traffic markings are placed according to these Specifications and the manufacturer’s recommendations. Manufacturer’s Representative shall be present continuously for each day of striping for each project.

c. Contractor shall apply retroreflective preformed patterned pavement tape traffic markings with equipment designed and capable of properly applying and inlaying or grooving in pavement markings at the point and time of application approved by the Manufacturer’s Representative for the type of pavement marking being applied.

4. Methyl Methacrylate

a. Contractor shall prepare the roadway areas to receive the methyl methacrylate pavement markings in accordance with this Section and the manufacturer’s recommendations. Contractor shall submit a current copy of manufacturer’s recommendations at least 5 working days prior to application of traffic markings.

b. Contractor shall apply methyl methacrylate pavement markings at a minimum thickness of sixty (60) mils. The thickness is measured without glass beads.

c. Contractor shall provide a manufacturer’s representative to be present on the first day of striping for each type (sprayed or extruded) and additional days as required by the Engineer.

d. Contractor shall not apply striping to new asphalt/P.C.C. until the asphalt/P.C.C. has cured to the satisfaction of manufacturer’s representative or the Engineer.

e. The minimum application rate of beading on sprayed markings is twenty pounds (20 lbs) of beads per gallon and twelve pounds (12 lbs) of beads per one hundred (100) square feet for extruded markings.

f. The surface temperature of the roadway shall be in the range of 30° to 105° Fahrenheit for stripe application. Contractor shall thoroughly clean and dry the roadway surface.

g. Contractor shall apply methyl methacrylate stripe material with equipment designed and capable of properly mixing at the point and time of application in accordance with the manufacturer’s recommendations.

H. Pavement Marking Removal

Pavement markings shall be removed to the fullest extent possible from the pavement by any method that does not materially damage the surface or texture of the pavement or surfacing. Sand or other material deposited on the pavement as a result of removing traffic stripes and markings shall be removed as the Work
progresses. Accumulations of sand or other material which might interfere with drainage or might constitute a hazard to traffic are not permitted.

Pavement markings no longer applicable which may create confusion in the minds of motorists shall be removed or obliterated before any change is made in the traffic pattern. Painting over markings is prohibited.

Pavement markings shall be removed by such methods that will cause the least possible damage to the pavement or surfacing. Any damage to the pavement or surfacing caused by pavement marking removal shall be repaired by the Contractor at his expense by acceptable methods.

Where blast cleaning is used for the removal of pavement markings or for removal of objectionable material, and such removal operation is being performed within ten feet (10’) of a lane occupied by public traffic, the residue, including dust shall be removed immediately after contact between the sand and the surface being treated. Such removal shall be by a vacuum attachment operating concurrently with the blast cleaning operation, or by other approved methods.

I. Preliminary Spotting

The Contractor will provide the necessary control points at intervals including all changes of direction and changes in the basic configuration of striping such as at the beginning and ending of no-passing zones on a two-way, two-lane roadway. These points shall be used in preliminary spotting of lines before striping is commenced. The Contractor shall be responsible for preliminary spotting of the lines to be painted and he must obtain approval from the Engineer for all spotting before striping may begin. Preliminary spotting is required for all longitudinal striping.

J. Inlaid Protected Markings

Apply with certified extrusion equipment. Grind the slot to a depth of two hundred and fifty (250) mils and the dimensions specified, using a grinder capable of grinding the slot to the specified depth and width in a single pass. After grinding, obtain approval before placing marking material.

1. Dispose of asphalt grinding according to applicable federal, state, and local regulations.

2. Depth of Inlay Slot. Depth of material shall be measured from the peaks created by the groves to the visible surface of the markings. Minimum two hundred and fifty (250) mils to maximum two hundred and ninety (290) mils.

3. Thickness of Inlaid Marking Material. Fill inlay area completely from the bottom of the inlay to the surface of the pavement. Do not overfill the slots.
K. Tolerances of Lane Striping

The Contractor shall keep his work within the following allowable tolerances:

1. Length of Stripe. The longitudinal error within a forty foot (40’) length of lane line shall not be more than plus or minus six inches (±6”).

2. Width of Stripe. The width of stripe shall not vary more than plus or minus one-half inch (±1/2”).

3. Lane Width. The width of lanes shall not vary more than plus or minus four inches (±4”) from the widths shown on the Drawings, measured from the edge of pavement or edge of traveled way to center of lane line or between the centers of adjacent lane lines.

4. Stripes on Tangents. Stripes on tangents shall not vary more than plus or minus one inch (1”) laterally within a distance of one hundred feet (100’) when using the edge of the stripe as reference.

5. Stripes on Curves. Stripes on curves shall be uniform in alignment with no apparent deviations from the true curvature.

6. All Stripes. All stripes shall remain within four inches (4”) from the planed alignment when measured to the center of the stripe.

7. Inlay Slot. The inlay slot shall be a minimum of two hundred and fifty (250) mils and a maximum of two hundred and ninety (290) mils.

Traffic markings not within the above tolerances will be considered unacceptable under this Section and shall be replaced by the Contractor at no additional cost to the Municipality.

Article 10.4 Measurement

Traffic markings shall be measured by linear foot of traffic marking of the specified width and color complete in place and accepted. Traffic markings consisting of words and symbols shall be paid per item complete in place and accepted.

Removal of traffic stripes and pavement markings as well as repair of any damaged pavement or surfacing caused by the pavement marking removal operations shall be incidental to other items of Work.

Payment for traffic markings is full compensation for preparing and cleaning of pavement, application of painted traffic markings and applying glass beads or spheres thereto, application of retroreflective preformed patterned pavement tape pavement markings, furnishing paint, glass beads, retroreflective preformed patterned pavement tape pavement markings, and all other material and equipment necessary to complete the Work described in this Section.


**Article 10.5  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Traffic Markings (Type) (width)</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Traffic Markings (Type) (words &amp; symbols)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.11  STANDARD SIGNS

Article 11.1  General
This Work shall consist of furnishing and installing signs, guide markers, object markers and mileposts. The sign location and type of installation will be as shown on the Drawings or as directed by the Engineer. Work under this Section shall also include removal and relocation, as well as removal and disposal of existing signs, mileposts, and markers. Work under this Section shall also include removal and resetting of sign post assemblies to original location or as directed by the Engineer.

Article 11.2  Materials

Fabricate all standard regulatory, warning, and guide signs for permanent installation with Type IX reflective sheetings that conform to ASTM D4956 and single-span aluminum panel substrates, unless designated otherwise on the Drawings.

All orange construction and maintenance signs shall be fabricated with Type IV (encapsulated lens) reflective sheeting.

All new standard signs for permanent installation shall be of new materials. All sign layouts shall be in accordance with "Alaska Sign Design Specifications." Any sign delivered or installed which does not conform to these specifications shall be replaced by the Contractor at no additional cost to the Municipality.

Concrete for sign post foundations shall conform to Class B-3 per Division 30, Section 30.01, Article 1.4 - Mix.

In the following specifications, a sign’s height and width refers to an installed sign’s vertical and horizontal dimensions, respectively, and to the length of the sides for diamond shaped signs.

A. Aluminum Sheet

Contractor shall provide sheet aluminum sign panels in one of the following alloys: 6061-T6, 5052-H36, or 5052-H38 that conform to ASTM B-209. The thickness of the aluminum sheet shall be 0.125 inches unless otherwise specified. Alloy and temper designations shall be verified by mill certification.

Treat the aluminum base metal sheets with a conversion coating for aluminum conforming to ASTM B-921 or ASTM B-449, Class 2 standards. The cleaned and coated base metal shall be handled only by mechanical device or by operators wearing clean cotton or rubber gloves. After the cleaning and coating operation, the panels shall be protected at all times from contact or exposure to greases, oils, dust, or other contaminants.

Use single piece sign panels for all signs up to 48 inches by 72 inches. For signs with one or both dimensions larger than the base 48 inch by 72 inch sign panel, assemble multiple single-piece sign panels according to the following:
1. For signs up to 48 inches high, assemble the single-piece aluminum panels with the 72 inch dimension set horizontally.

2. For signs between 48.01 and 72.01 inches high, assemble the single-piece aluminum panels with the longer dimension set vertically.

3. For signs between 72.01 and 96.01 inches high, assemble two rows of single-piece aluminum panels with the 72 inch dimensions set horizontally.

The dimensional tolerance of the panels shall be one-sixteenth inch (1/16”). Metal panels shall be cut to size and shape and shall be free of buckles, warp, dents, cockles, burrs, and any other defects resulting from fabrication. All possible fabrication, including shearing, cutting and punching of holes shall be completed prior to the base metal preparation.

B. Sheet Reflective Materials

Use reflective sheetings that are part of a matched component system made by a single manufacturer. The system shall include the sheetings, process colors, clear coatings, sealants, electronically cuttable films, protective overlay films, and recommended application equipment.

The sheetings shall also pass all performance requirements specified in ASTM D4956 for type IX reflective sheetings, when tested according to the methods specified therein, including the supplementary fungus resistance requirement.

The sheeting manufacturer shall furnish third party test results that verify their sheeting materials meet all performance requirements of ASTM D4956. If the results of the accelerated outdoor weathering test are not yet available, furnish the results of the supplementary artificially accelerated weathering test and provide the date the regular test results will be available.

Furnish reflective sheetings with a class 1 adhesive backing that meets the requirements of ASTM D4956.

Fabricate signs according to the manufacturer’s written recommendations, using the process colors, coatings, sealants, and films made by the manufacturer of the reflective sheetings, and the application equipment recommended by the sheeting manufacturer.

Apply the reflective sheetings with no splices to those aluminum panels that can be oriented to fit on a rectangle with the smaller dimension equal to or less than forty-eight inches (48”). For all other sign panels, apply the reflective sheetings to form butt splices oriented to most efficiently utilize the sheeting material, except no splices are allowed within two inches (2”) of the edge of a sign and in the length of reflective sheetings.
When making the butt splices, match the adjacent pieces as recommended by the manufacturer to assure uniform day color and night appearance. Provide a gap up to one-sixteenth inch (1/16") wide between the pieces of reflective sheeting.

Seal all cut edges of the reflective sheetings with sealant recommended by the sheeting manufacturer, including legends.

C. Letters, Numerals, Arrows, Symbols, Border

Letters, numerals, arrows, symbols, border, and other features of the sign messages shall be of the type, size, and series as specified by the Alaska Traffic Manual or the Alaska Sign Design Specifications.

Completed letters, numerals, and other units shall be formed to provide continuous stroke width with smooth edges and shall present a flat surface free of warp, blisters, wrinkles, burrs, and splinters.

Fabricate the legend on signs using one of the following processes. For signs fabricated using the two screened processes, apply a clear coat over the entire face of each sign using a manufacturer recommended product.

1. For signs with a black legend, apply opaque black ink to form the legend on the reflective sheeting using the silk screened process.

2. For signs with a white legend on a colored background, apply transparent ink to all areas of the white reflective sheeting, except the legend, to form the background using the reverse silk screened process.

3. Apply electronically cut colored films that include adhesive to the reflective sheeting, similar to 1 and 2.

4. Cut the legends from the requisite color of type IX reflective sheetings and apply them to the reflective sheeting. Orient all elements of the legend in the same direction on the reflective sheeting before cutting them out.

D. Frames

All rectangular signs, over fifty-three inches (53") measured along the horizontal axis, and all diamond shape signs sixty inches by sixty inches (60" x 60") and larger shall be framed unless otherwise specified. The frames shall be constructed of aluminum as indicated on the Drawings. All framing dimensions shall have a one-eighth inch (1/8") tolerance unless otherwise specified.

The frame shall be affixed to the sign with three-sixteenth inch (3/16") diameter aluminum rivets. The maximum rivet spacing shall be twelve inches (12") on centers. No rivets shall be placed closer than three-eighths inch (3/8") from the edge of the aluminum face sheet.
All joints of the aluminum frame may be welded with an inert gas shielded - arc welding process using 4043 electrode filling wire in accordance with good shop practice. The width of the fillet shall be equal to the wall thickness of the smallest framing member being welded.

E. Test Procedures and Inspections

1. Adherence

The test panel, after a 72-hour curing time, shall be immersed in 95°F. ±3°F water for a period of 24 hours. Immediately after removal from the bath, the reflective sheeting shall be sufficiently bonded so that it cannot be readily removed from the aluminum surface with a one inch (1") round nose spatula. If the sheeting can be peeled rather than chipped from the surface, the bond is considered unsatisfactory.

2. Solvent Resistance

Test shall be in accordance with Federal Specification L-S300B 4.3.6

3. Accelerated Weathering

Test shall be in accordance with Federal Specification L-S 300B 4.3.9.

4. Resistance to Heat, Cold, and Humidity

Test shall be in accordance with Federal Specification L-S-300B 4.3.10.

5. Tensile Strength and Elongation

Test shall be in accordance with Federal Specification L-S 300B 4.3.15.

F. Post Materials

Provide sign posts that conform to the following specifications for installing the post mounted signs specified in the Drawings.

Perforated Steel Posts.

1. For sign posts, install perforated steel tubes that conform to ASTM A-653. Provide tubes fabricated from 0.105 inch thick (12 U.S. Standard Gauge) sheet steel zinc coated on both sides to minimum coating thickness designation G-90. Furnish tubes formed with square cross sections and sheet steels rolled from structural grade steel with 50 ksi yield strength.

2. All tubes shall be perforated along the centerline of each side for their entire length with seven-sixteenth inch (7/16") diameter holes on one inch (1") centers. All perforations shall be free from burrs.
3. Furnish perforated tubes that are straight and feature a smooth, uniform finish without splices. Consecutive one-quarter inch (1/4") size tubes shall telescope freely for a minimum length of ten feet.

4. Furnish the tube sizes specified in the Drawings in lengths that will provide one-piece sign posts when the signs are installed according to the Standard Details that apply, regardless of ground cross section.

**Article 11.3 Construction**

All sign post foundations shall be cast in excavated holes. Depth of embedment shall be as shown on the Standard Details unless otherwise directed by the Engineer.

Surplus excavated material shall be disposed of along the adjacent roadway or as directed by the Engineer.

Cut each perforated tube to provide the sign mounting-height specified in the Section 70 detail that applies. Adjust each tube length to account for the height of the signs, the difference in elevation between the mounting height reference and the top of the foundation, and the one foot length inserted into the foundation. Remove all burrs from the cut end.

Guide Marker reflectors shall be installed after the posts have been set in place.

Sign panels shall be attached to posts, electroliers, traffic signal standards, bridge rails, piers, and abutments with fastening hardware of the types and sizes shown on the Standard Details. All fastening hardware shall be furnished by the Contractor.

Existing signs and mile posts that are removed and relocated shall be placed on a new base and shall conform to the Drawings or as directed by the Engineer.

Contractor shall salvage and deliver the removed signs to the Municipal Traffic Sign Shop, 343-4384. Contractor shall remove sign from post before removing and delivery to the Municipal Traffic Sign Shop. Contractor shall remove the sign(s) and associated hardware without damaging the post or sign face. Contractor shall replace sign components damaged or destroyed due to Contractor’s operations at no cost to the Municipality.

Inspection: All materials and finished signs are subject to inspection and acceptance in place. All surfaces exposed to weathering shall be free of any defects in the coating that may impair the serviceability or detract from the general appearance or color match. The finished signs shall be clean and free from all chatter marks, burrs, sharp edges, loose rivets, delaminated reflective sheeting, and aluminum marks. No repairs shall be made to the face sheet. All signs not conforming to these Specifications shall be rejected and shall be replaced by the Contractor at no additional expense to the Municipality.

**Article 11.4 Measurement**

The quantity of Standard Regulatory, Warning, and Guide Signs for permanent installation to be paid for shall be the total square footage of legend bearing sign and panel erected in
place. No deductions in quantity for corner rounding shall be made. Nominal dimensions for sign sizes indicated on the Drawings shall be used for the purpose of calculating sign pay quantities.

Removal and relocation of existing signs shall be measured per each sign, completed and accepted in final position. Sign components damaged or destroyed due to the Contractor’s operation shall be replaced by the Contractor at no additional expense to the Owner. Object Markers and Guide Markers shall be measured per each, complete in place. One post equipped with two reflectors shall be considered a single marker. One signpost equipped with two or more signs is a single sign assembly.

**Article 11.5  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Standard Sign</td>
<td>Square Foot</td>
</tr>
<tr>
<td>Remove and Relocate Existing Signs</td>
<td>Each</td>
</tr>
<tr>
<td>Remove and Relocate Mile Posts</td>
<td>Each</td>
</tr>
<tr>
<td>Remove and Reset Sign Assembly</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.12 TRAFFIC MAINTENANCE

Article 12.1 General

The Work under this Section consists of performing all the necessary measures to protect and control traffic during the life of the individual project including, but not limited to, furnishing, erecting, maintaining, replacing, cleaning, moving and removing the traffic control devices, construction signs, portable concrete barriers, safety fences, and flagging required to safeguard the traveling public and all administrative responsibilities necessary to implement this Work.

The Contractor shall maintain all roadways, pedestrian, transit, and bicycle facilities within the project limits, and construct and maintain such approaches, crossings, intersections, and other features as may be necessary throughout the life of the Contract. The Contractor shall also have a powered broom (pick-up sweeper type) and water truck with high-pressure capabilities to clean the paved surfaces and along the haul routes.

A Traffic Control Plan (TCP) is required for this project. The Contractor shall provide a TCP approved by the Traffic Engineer, and when the project limits include State right-of-way, the Alaska Department of Transportation and Public Facilities (ADOT&PF) Regional Traffic Engineer prior to commencement of this project in accordance with the provisions of Division 10, Section 10.04, Article 4.13 - Traffic Control Plan. The Contractor may submit approved amendments to the TCP prior to commencement of the project. All approvals shall be obtained by the Contractor and shall be considered incidental to the Contract and no separate payment shall be made. When the project limits include state right-of-way, the Contractor shall notify ADOT&PF at least 48 hours before commencing construction.

The Contractor is required to have at least one International Municipal Signal Association (I.M.S.A.) or American Traffic Safety Services Association (A.T.S.S.A.) certified person on the job site during working hours for traffic control and implementation. The person designated will supply the Traffic Engineer with a 24-hour emergency telephone number.

All traffic channelization, detours, lane closures and/or street closures shall conform to the TCP and Division 10, Section 10.04, Article 4.13 - Traffic Control Plan.

Article 12.2 Traffic Control Plan

A TCP is a drawing or drawings indicating the method or scheme for safely and efficiently routing traffic during construction. The TCP may include, but not be limited to, such items as signs, portable concrete barriers, barricades, traffic cones, special signs, warning lights, portable changeable message board signs, flaggers, pilot cars, work zone pavement markings, temporary roadways, and all other items required to direct traffic through or around the construction zone in accordance with these specifications and the Alaska Traffic Manual (ATM). These TCPs shall also address placement of traffic control devices including location, size, mounting height, and type. The details shown for signs shall include the code designation, size, and legend in accordance with the ATM and the Alaska
Sign Design Specifications (ASDS). The TCP shall also indicate the method of safely routing pedestrian and bicycle traffic through or around the construction zone.

The Contractor shall prepare a TCP for the project. The TCP shall be approved by the Traffic Engineer and, if the Traffic Engineer determines that a state route is affected by the TCP, approved by the State of Alaska, Department of Transportation and Public Facilities Regional Traffic Engineer. The Traffic Engineer will issue road closure and/or work-in-roadway permits upon approval of the TCP. Changes in the TCP resulting from emergency circumstances may be allowed during construction, provided a minimum of 48 hours are allowed for review, and the changes are approved by the Traffic Engineer. The Contractor shall respond and make field changes as the Traffic Engineer directs.

No Work shall occur within rights-of-way or easements for public streets, highways, or other public improvement projects until the Contractor has implemented an approved TCP for the Work proposed. The number of signs indicated on the TCP is a minimum. If unsafe conditions occur, the Traffic Engineer may require additional signs/devices at no additional cost to the Owner.

Article 12.3 Pedestrian Traffic

The Contractor shall provide pedestrian access in accordance with the requirements of Division 10, Section 10.04, Article 4.10 - Protection of Persons and Property, and Article 4.12 - Public Convenience and Access.

In addition, the Contractor shall provide and maintain a pedestrian traffic route through the Project for the duration of the Project or until a permanent pedestrian walkway has been completed. The route shall be signed and delineated such that it is obvious and recognizable to the pedestrian. The route shall be established in a location within the Project limits, at a distance which will help to eliminate interference between pedestrians and construction operations. The location of the route may change throughout the duration of the project, depending on locations of construction operations, and each location shall be approved by the Traffic Engineer. Safety fencing shall be required along the pedestrian route as necessary to separate work zone from the pedestrian route.

Article 12.4 Work Site Traffic Supervisor

The Contractor shall provide a Work Site Traffic Supervisor whom shall be responsible for the Contractor's maintenance of traffic operations on a 24-hour basis. The Work Site Traffic Supervisor shall meet the following minimum requirements:

1. Is currently certified as a Work Site Traffic Supervisor by the American Traffic Safety Services Association (A.T.S.S.A.); or

2. Is currently certified as a Work Zone Traffic Safety Specialist or a Work Zone Safety Specialist by the International Municipal Signal Association (I.M.S.A.); or

3. Is currently certified as a Work Site Traffic Technician by A.T.S.S.A.
If the individual is certified under Items 2 or 3 above, the Contractor shall submit, prior to Work requiring traffic control, documentation that the individual has also obtained:

1. A minimum of 12 months of supervisory level work in Work Site Traffic Control; or

2. One year of having been in responsible charge of such Work. The term “in responsible charge” shall be construed to mean having been in a position of accountability for the selection of devices and for their placement in the traffic control system, or for the continued operation of the system. Having persons that actually perform the labor under one's control would satisfy this requirement. Provide at least one reference, including current address and telephone number, for each project which "in responsible charge" experience is claimed.

The Work Site Traffic Supervisor's duties shall include the following:

1. Understand the requirements of the ATM, the Drawings, and the Specifications.

2. Prepare the TCPs and public notices and coordinate traffic maintenance operations with the Traffic Engineer.

3. Daily inspect the condition and position of all traffic control devices in use on the Project and ensure that all traffic control devices are in proper working order, clean, visible, and conform to the approved TCP in use. All devices shall be inspected during hours of darkness so that effectiveness of the device placement can be evaluated and adjustments made, if required, to afford maximum nighttime visibility and delineation. These inspections shall be documented in a bound field book. The field book shall be made available to the Traffic Engineer for review on a daily basis and shall become the property of the Traffic Engineer upon completion of the Work.

4. Supervise the repair or replacement of damaged or missing traffic control devices.

5. Review and anticipate appropriate traffic maintenance needs and ensure that the proper traffic control devices necessary for safe and efficient traffic movement are available.

6. Hold weekly traffic safety meetings with the superintendents and foremen of the Contractor and subcontractors prior to beginning construction. The Contractor shall provide the Traffic Engineer the opportunity to attend these meetings.
Article 12.5  Materials

Materials for traffic control devices shall conform to the requirements set forth below:

1. **Signs.** Permanent Construction Signs, Construction Signs, and Special Construction Signs including sign supports shall conform to the requirements of Section 70.11 - Standard Signs, ADOT&PF Standard Specifications for Highway Construction; the ATM; and the Alaska Sign Design Specifications (ASDS). The size of each Special Construction Sign shall be clearly and neatly marked on back in three inch (3") high black numerals.

2. **Portable Sign Supports.** Contractor shall provide portable sign supports that are wind resistant with no external ballasting and capable of supporting a forty-eight by forty-eight inch (48" x 48") traffic control sign such that the height of the sign above the adjacent roadway surface conforms to the ATM. The sign support shall support the traffic control sign vertically.

3. **Barricades and Vertical Panels.** Contractor shall provide barricades and vertical panels that are constructed of wood, metal, or plastic, and conform to the requirements of the ATM. Type III barricades shall have a minimum width of eight feet (8’). Contractor shall equip barricades with warning lights.

4. **Warning Lights.** Contractor shall provide Type A (low intensity flashing), Type B (high intensity flashing), or Type C (steady burn) warning lights conforming to the requirements of the ATM.

5. **Drums.** Contractor shall provide plastic drums conforming to the requirements of the ATM.

6. **Traffic Cones.** Contractor shall provide traffic cones and/or tubular markers conforming to the requirements of the ATM. The minimum height is twenty-eight inches (28”). All cones and tubular markers shall be reflectorized.

7. **Portable Changeable Message Board Signs.** Contractor shall provide portable, truck- or trailer-mounted, changeable message board signs with a self-contained power supply for the sign that has the following features:

   a. Message sign panel large enough to display three lines of nine inch (9”) high characters.

   b. Eight-character display per message line.

   c. Message modules containing at least 36 different preprogrammed messages (three line displays) to be selected by the Traffic Engineer.

   d. The capacity to create, preview, and display new messages and message sequences.
e. A waterproof, lockable cover for the controller keyboard.

f. An operator's manual, a service manual, and wiring diagram.

g. Quick release attachments on display panel cover.

h. Variable flash and sequence rates.

i. Manual and automatic dimming capability on lamp bulb matrix models.

j. Variable mounting height of at least seven feet (7') from the pavement to the bottom of the message sign panel.

k. The capacity to operate with a battery pack for two hours under full load.

8. **Portable Concrete Barriers.** Portable concrete barriers shall conform to the requirements of ADOT&PF Standard Drawing G-45. Portable concrete barriers shall be equipped with warning lights.

9. **Work Zone Pavement Markings.** Work zone pavement markings shall be either painted with glass beads or preformed marking tape (removable or non-removable).

**Article 12.6 Public Notice**

The Work Site Traffic Supervisor shall give notices of changes, delays, or lane/road closures to the following local officials and transportation organizations including, but not necessarily limited to:

1. Anchorage Chamber of Commerce
2. Alaska Travel Industry Association
3. Alaska Trucking Association
4. Alaska State Troopers
5. Alaska Court System
6. Anchorage Police Department
7. Anchorage Fire Department
8. Local Emergency Medical Services
9. Anchorage Public Transportation
10. ASD Pupil Transportation
11. U.S. Postal Service
12. Local Schools and Universities
13. MOA Parks and Recreation
14. Volunteer Fire Departments (applicable if operating in the project area)
15. Local Solid Waste Utilities
The Contractor shall also provide the Alaska State Troopers, Anchorage Police Department, and Anchorage Fire Department with a list of radio frequencies and cellular telephone numbers used on the project, and the 24-hour telephone numbers of the Work Site Traffic Supervisor and the Project Superintendent. This contact information shall be for alerting the Contractor of emergencies which will require passage of emergency vehicles through the project. When so notified, the Contractor shall use all equipment and effort necessary to expedite rapid passage.

The Engineer shall provide newspaper advertisements and radio announcements regarding the project, its location, traffic information, and the effective dates of detours and road closures. The Contractor shall provide the Engineer information regarding scheduled detours and closures as requested by the Engineer. Delays in providing requested information to the Engineer or inaccuracies in this information will be grounds for not approving requested detours and closures. The Contractor shall not be permitted to implement detours and closures in advance of published dates, unless approved by the Engineer.

Article 12.7 Traffic Control Devices

Prior to the start of construction operations, the Contractor shall erect such permanent and temporary traffic control devices as may be required by the approved Traffic Control Plan (TCP). Traffic control devices shall be operated only when they are needed and only those devices that apply to conditions that exist shall be used. Advisory speeds, when necessary, shall be provided in the TCP and approved by the Traffic Engineer.

Following the completion of Work in a closure area, all traffic control devices relating to the closure shall be removed. Sign panels that are not removed shall be entirely covered with either metal or plywood sheeting.

Contractor shall keep clean reflective sheeting on signs, drums, barricades, and other devices. Contractor shall promptly replace any devices with scratches, rips, or tears in the sheeting deemed unacceptable by the Engineer at no additional expense to the Owner.

Article 12.8 Authority of the Engineer

When, in the opinion of the Engineer, conditions are such that the safety and/or convenience of the traveling public are adversely affected, the Engineer will immediately notify the Contractor in writing. The notice will state the defects, the corrective actions required, and the time required to complete such actions. In no case shall this time exceed twenty-four (24) hours. In the event that the Contractor fails to take the corrective actions within the specified time, (a) the Engineer will immediately direct that the offending operations cease until the defects are corrected, and (b) the Engineer reserves the right to order the corrective actions be accomplished by outside forces. The Engineer will deduct the cost of Work by outside forces from any moneys due or that may become due under the terms of this Contract.
Article 12.9 Execution

The Contractor shall maintain traffic control in accordance with the approved Traffic Control Plan (TCP). The Contractor shall submit a new TCP each time traffic control is revised. Approval of each new TCP is required.

The Contractor shall furnish all traffic control devices necessary to fulfill the requirements of this specification, including construction signs, barricades, portable concrete barriers, safety fence, and flaggers. Said traffic control devices are considered incidental to this item. All such devices shall conform to the Alaska Traffic Manual (ATM). Paved road detours and gravel pedestrian detours are also incidental to this item.

The Contractor shall protect and adequately delineate open trenches, ditches, pavement edge drop-offs, and other excavations and hazardous areas with barricades as required by OSHA. Open trenches with drops of two feet (2') or greater adjacent to the roadway shall have portable concrete barriers installed with sloping end(s). All barricades and portable concrete barriers in place at night shall have warning lights installed in accordance with the ATM.

Unless otherwise provided hereinafter, the Contractor shall maintain all roadways open to traffic. Temporary closure of residential, commercial, or street approaches requires prior approval of the Engineer. The Contractor shall provide access through the project for emergency vehicles. Contractor shall properly sign and/or flag all locations requiring redirection or stopping of the traveling public.

The Contractor's equipment shall stop at all points of intersection with the traveling public unless satisfactory traffic control measures, approved in writing by the Engineer, are installed and maintained by the Contractor.

Where required on the Drawings, the Contractor shall bypass traffic over a paved detour route. When no longer required, the detour shall be removed and the approaches obliterated. Construction of the detour will be considered incidental to bid item “Traffic Maintenance.”

The Contractor shall provide flagger(s) any time operations may affect safety, or as directed by the ATM or the Engineer. Flagger(s) are required at all times when one-lane traffic is in effect. All flagging operations shall conform to the procedures outlined in the ATM.

Article 12.10 Measurement

All Work in this Section is measured by lump sum and shall consist of all labor, materials, and equipment required to provide the Work Site Traffic Supervisor, all TCPs and TCP revisions, public notices, paved traffic detours, pedestrian and bicycle detours, and all traffic control devices. Work and materials required to provide temporary asphalt surfacing and temporary painted traffic markings shall not be paid separately and shall be considered incidental to the Work described in this Section.
Article 12.11 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Traffic Maintenance</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 70.13  BOLLARDS

Article 13.1  General

The Work under this Section consists of all labor, equipment, and materials necessary to complete the construction of wood, concrete pipe, and removable steel bollards as shown on the Drawings. Where existing bollards are removed prior to reuse, Contractor shall erect "hasty" fence or snow fence to deter vandalism by motorized vehicles.

The following Applicable Standards shall be used:

AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM)
ASTM A 36M (1997a) Carbon Structural Steel
ASTM A 123 (1989a) Zinc (Hot-Dipped Galvanized) Coatings on Iron and Steel Products
ASTM A 500 (1996) Cold-Formed Welded and Seamless Carbon Steel Structural Tubing in Rounds and Shapes
ASTM A 501 Standard Specification for Hot-Formed Welded and Seamless Carbon Steel Structural Tubing
ASTM A 615/A 615M Standard Specification for Deformed and Plain Billet-Steel Bars for Concrete Reinforcement
ASTM C 150 Portland Cement
West Coast Lumber Inspection Bureau standard grading and dressing rules
Western Wood Products Association standard grading and dressing rules.

Article 13.2  Materials

A.  Wood Bollards

1.  Storage and Protection: Protect lumber from weather. Store inside whenever possible.

2.  Grading Rules: Standard grading and dressing rules of the West Coast Lumber Inspection Bureau or the Western Wood Products Association. Each piece of yard and structural lumber shall bear official grade mark of the appropriate bureau or association. Provide Common No. 1 or better Hem/Fir wood, surfaced four (4) sides unless otherwise noted on the Drawings, and kiln dried. Moisture content shall not exceed nineteen percent (19%).

3.  Preservative Pressure Treatment

   a.  Pressure-treat all wood in direct contact with ground with chromate copper arsenate, Type II (AWPA P-5) with a retention of 0.45 lbs. per
cubic foot of wood. Contractor shall submit a certificate of treatment to the Engineer for approval prior to use on the Project.

b. After treatment, wood shall be clean, of natural color and finish, non-corrosive, water repellent, paintable, odorless, dry, and non-staining.

c. Cut Wood bollards to length necessary for construction before preservative pressure treatment is applied.

4. Paint: Stain Wood bollards with two coats Color Shield ’4000’ Alkyd Flat stain, or approved equal. Color to be Russet Brown, or similar. Apply stain to clean dry surface, free of dust or dirt, in accordance with the manufacturer’s recommendations and specifications.

B. Concrete Pipe Bollards

Contractor shall use only new products in construction and installation of concrete pipe bollards. Standard products of a manufacturer regularly engaged in the manufacture of such products. The materials provided shall be of a type with proven satisfactory use for at least two years.

1. Concrete

Portland cement shall conform to ASTM C 150 Types I, II, or III.

2. Finish

Finish shall be galvanized. Exposed surfaces and edges shall be rounded, polished, or sanded. Finish shall be non-toxic, non-glare, and resistant to corrosion.

3. Galvanizing

After fabrication, hot-dip galvanized components in zinc in accordance with ASTM A 123. Remove Tailings and sharp protrusions formed as a result of the hot-dip process and burnish exposed edges.

4. Tubing

Provide Schedule 40 steel tubing of the size specified in the Drawings.

5. Paint

Prime Tubing and cover with two coats minimum of dark green powder coat paint in accordance with the manufacturer’s instructions. Top coat with two coats Yellow Carboline 139, unless guard post cover or sleeve is to be installed.
C. Removable Steel Bollards

1. Furnish hardware as necessary and as detailed for the project. Items include bolts, nuts, anchor bolts, washers, nuts, and rods (ASTM A-307). Hot-dip galvanize all bolts, nuts, washers, and plates in accordance with ASTM A-123.

2. Provide steel that conforms with ASTM A-36 structural carbon steel, shop fabricated and galvanized.

3. Painting: Etch galvanized surface with “Galvaprep.”

D. Guard Post Cover/Sleeve

Provide guard post covers molded from a durable polyethylene with ultra-violet (UV) stabilizers to ensure product life and color fastness.

Secure the polyethylene guard post cover or sleeve in accordance with the manufacturer's recommendations. Provide Carsonite SAV-T Sleeve, the guard post cover or sleeve, or approved equal.

Article 13.3 Construction

A. Acceptance of Existing Surfaces

The Contractor shall verify that finished grade and other operations affecting mounting surfaces have been completed prior to the installation of bollards. Install Bollards plumb and true in accordance with the approved manufacturer's instructions or recommendations.

B. Installation

For concrete pipe bollard, provide footing as shown on Drawings. Slope drainage from tubing at two percent (2%) grade. Place concrete inside steel pipe or tubing for full extent. Rod concrete to remove air voids. Dome top to provide clean transition from top surface to bollard sides. Do not leave exposed edge. Provide brushed finish to concrete dome.

Install bollards plumb, level and true to line. Top of a row of bollards shall be maintained at a consistent level above adjacent ground.

C. Removable Bollards

1. Install bollard base plate flush with top of paved trail. Install bollards plumb, level, and true to line. Use only three-sixteenth inch (3/16") fillet welds. Grind all edges smooth.

2. Fasteners: Padlocks for removable bollards shall be American Lock, WWE Series 3560 purchased from Action Locksmith. Cores shall be keyed to
Municipality Of Anchorage cores matched to a 645 key. Installation of the cores must be authorized by Facilities Management at 343-8270. Padlocks are incidental to this pay item.

D. Clean Up

Clean the site of all materials associated with the installation. Clean surfaces of dirt, stains, filings, and other blemishes occurring from shipment and installation. Provide cleaning methods and agents according to manufacturer's instructions or as indicated. Remove excess concrete.

**Article 13.4 Method of Measurement**

Measurement will be based on complete units in place for all bollards.

Measurement for concrete pipe bollard with sleeve shall be for a concrete pipe bollard with polyethylene guard post cover or sleeve installed in place.

Measurement for “Remove Bollard” shall be for each bollard removed and disposed of as directed on the Drawings. Footings, anchoring devices, and other items shown on the Drawings shall be considered incidental to the bid item “Removable Bollard” and no separate payment shall be made.

Any other items required for a complete and finished installation shown on the Drawings are measured separately for payment purposes.

**Article 13.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Bollard (Type) (Color as appropriate)</td>
<td>Each</td>
</tr>
<tr>
<td>Remove Bollard</td>
<td>Each</td>
</tr>
<tr>
<td>Remove &amp; Reset Bollard</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.14  REMOVE GUARDRAIL

Article 14.1  General

The Work under this Section consists of providing all operations and furnishing all equipment and materials pertaining to the removal and disposal of guardrail designated for removal on the Drawings or as directed by the Engineer.

Article 14.2  Construction

Contractor shall remove the guardrail, bolts, and supporting posts and deliver them to a location as directed by the Engineer. If guardrail and supporting posts are not salvaged, Contractor shall provide a disposal site for the removed guardrail, bolts, and supporting posts in accordance with the provisions of Division 10, Section 10.04, Article 4.9 - Disposal Sites.

Excavation and backfill required in the removal of the guardrail, bolts, and supporting posts is incidental to the bid item. Contractor shall backfill the excavation with native non organic material.

Article 14.3  Measurement

Removal and salvage or disposal of the guardrail, bolts, and supporting posts is measured per linear foot along the face of the guardrail. Delivery of guardrail and associated components to Engineer-designated location or disposal at Contractor furnished disposal site is incidental to the pay item and no additional payment will be made.

Article 14.4  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and will include full payment for all Work as described in this Section.

Payment is made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Guardrail</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 70.15 GUARDRAIL

Article 15.1 General

The Work under this Section consists of providing all operations pertaining to the construction of guardrails.

Only one type of material shall be used on any one specific guardrail installation, unless otherwise approved by the Engineer.

Article 15.2 Material

A. Steel rail elements shall conform to the requirements of AASHTO M-180, Class B, unless a lighter weight rail is specifically called for on the Drawings or in the Specifications. Terminal sections shall not be less than twelve (12) gauge.

B. The bolts and nuts shall be galvanized steel and shall conform to the requirement of ASTM A-153, Class C and ASTM A-307.

C. Guardrail posts shall be of either wood or steel as specified.
   1. Wood posts shall be grade posts and timbers, or better, as rated by the West Coast Lumber Inspection Bureau, and shall be fabricated from one of the following timber species, unless otherwise approved: a) Douglas Fir; b) Western Pine; c) Larch, or; d) Hemlock. The length and cross-section of the posts shall be as shown on the Standard Details unless otherwise noted. Timber posts shall be treated with one of the following preservative treatments: a) Creosote Oil; b) Creosote-coal tar solution; c) Creosote-petroleum solution; d) Pentachlorophenol. Preservative treatments for wood shall conform to the applicable requirements of AASHTO M-133.
   2. Steel posts shall be of the section and length as specified or as shown on the Drawings. They shall be of copper bearing steel when so specified. Steel shall conform to the requirements of ASTM A 36 for the grade specified, or, for new railroad rail posts, of ASTM A 1 for the unit weight of rail specified.

The posts shall be galvanized or shop painted as specified.

Article 15.3 Construction

The construction of guardrails shall be in conformance with the manufacturer's recommendations, the Standard Details, and as directed by the Engineer.

Article 15.4 Measurement

Guardrails will be measured per linear foot along the face of the rail, including end sections.
Article 15.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Guardrail (Gauge)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 70.16    TEMPORARY GROUP MAILBOXES

Article 16.1   General

The Work under this Section consists of performing all operations pertaining to constructing and maintaining a temporary group of mailboxes for the duration of construction.

Article 16.2   Materials

Contractor shall furnish nails, staples, fastening wires, lumber, and all materials required for construction of the mailboxes.

Article 16.3   Construction

The temporary group mailboxes shall be provided by the Contractor prior to pavement removal. Temporary group mailboxes shall conform to current U.S. Postal Service standards.

Article 16.4   Measurement

Temporary group mailboxes shall be measured by lump sum.

Article 16.5   Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Temporary Group Mailboxes</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 70.17  RELOCATE MAILBOX

Article 17.1  General

The Work covered under this Section consists of performing all operations pertaining to the removal and resetting of mailbox(es) affected by construction of this project, and shall include providing temporary mailbox(es). The Engineer will designate which mailbox(es) is/are affected.

Article 17.2  Materials

Contractor shall salvage, from the existing mailbox(es), all reusable materials. Contractor shall replace materials that cannot be salvaged or are damaged by Contractor's operations, at Contractor's expense, with new materials, which as nearly as possible duplicate the kind, quality, and capacity of the original installation.

Contractor shall furnish nails, staples, fastening wires or devices, and all materials required for the construction of such anchors, posts, or other portions of the mailbox, which can be replaced more efficiently than they can be moved.

If the property owner elects to replace any of the existing mailbox materials with other materials in better condition, he will be responsible for furnishing and delivering such materials to the site of the Work.

Article 17.3  Construction

Contractor shall provide temporary mailbox placement and access. Contractor shall set the mailbox(es) in reasonably close conformity to its original location with respect to access points or as the Engineer directs. Contractor shall set posts and anchors at the same depth as in the original mailbox(es) or as directed by the Engineer. Contractor shall replace by equivalent construction permanent anchors, posts, or other parts that cannot be economically moved. Contractor shall paint new materials requiring painting. Contractor shall paint the entire mailbox and support if the paint on the new items does not adequately match or does not meet the satisfaction of the Engineer. Contractor shall place the relocated mailbox(es) in at least as good condition as the existing mailbox(es) before it was moved, behind the curb and gutter in accordance with postal regulations, or as the Engineer directs. Contractor shall place the face of the mailbox(es) forty-four to forty-eight inches (44” to 48”) above the top back of the curb, behind the curb and gutter, or in an alternate location approved by the U.S. Post Office.

Article 17.4  Measurement

Relocating mailboxes shall be measured per unit permanently relocated and complete in place. Each unit shall consist of a stand having a single mailbox or a variable number of mailboxes. No payment shall be made for temporary mailbox placement or relocation.
Article 17.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Relocate Mailbox</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.18    CHAIN LINK FENCE

Article 18.1    General

The Work under this Section consists of providing all materials and operations pertaining to construction of chain link fencing.

Article 18.2    Materials

Material used in the construction of chain link fencing shall be in accordance with the Standard Details and the requirements of the "Chain Link Fence Manufacturers Institute," as described below.

A.  General

Posts, gate frames, braces, rails, stretcher bars, and truss rods shall be of steel; reinforcing wires shall be of high carbon steel; and gate hinges, post caps, barbed-wire supporting arms, stretcher bar bands, and other parts shall be of steel, malleable iron or equal except that ties and clips may be of aluminum.

Contractor shall form parts accurately to dimensions. All steel and iron parts shall be zinc coated after fabrication, using zinc grade "E" in accordance with Federal Specifications QQ-Z-351.

The weight of the zinc coating per square foot of actual surface shall average not less than 1.2 ounces and no individual specimen shall weigh less than 1.0 ounce. Zinc-coated surfaces shall be free from imperfectly coated spots, bruised or scaled coating, drops of zinc, sharp projections, and sal ammoniac spots.

Posts, gate frames, rails, and braces shall conform to the dimensions and weights shown in the Dimensions and Weights Table in Article 18.3 – Tables.

B.  Fabric

Fencing fabric shall be zinc coated by the hot-dip process after fabrication. The zinc coating shall be commercially uniform. It shall not have less than 1.2 ounces per square foot when tested. Fabric gauge shall be as shown in the Fencing Fabric Size Table in Article 18.3 - Tables.

C.  Gates

Gates shall be swing or sliding, single or double, as specified, complete with latches, stops, keepers, hinges, or rollers and roller tracks, and, when so specified, with provisions for three (3) strands of barbed wire above the fabric.

Gate frames shall be constructed of tubular members, and shall be constructed in a manner such as to provide a rigid frame and ample strength and shall be free from sag and twist. Where a barbed wire top is specified, the end members of gate frames shall be extended approximately one foot (1’) above the top member and
arranged for attaching three (3) uniformly spaced strands of barbed wire and furnished with bands or other suitable method for securely attaching the wire. Fabric shall be attached securely to the gate frame at intervals not to exceed fifteen inches (15”).

Hinges shall be of heavy pattern, of adequate strength for the gate, and with large bearing surfaces for clamping them in position. The hinges shall not twist or turn under the action of the gate. The gates shall be capable of being opened and closed easily by one person.

Latches, stops and keepers shall be provided for all gates. Latches shall have the plunger-bar arranged to engage the gate stop, except that for single gates with openings less than ten feet (10’) wide, a forked latch may be provided. Latches shall be arranged for locking. Center stops shall consist of a device arranged to be set in concrete and to engage the plunger of the bar latch of double gates. No stop is required for single gates. Keepers shall consist of a mechanical device for securing the free end of the gate when in the full open position.

D. Posts

Posts shall be of the lengths specified and shall be tubular, except that line posts may be H-beam. Dimension and weight shall conform to the Dimensions and Weights Table in Article 18.3 – Tables, unless otherwise specified.

E. Post Braces

Post braces shall be provided for each gate, corner, pull, and end post for use with fabric five feet (5’) or more in height, and shall consist of a round tubular brace extending to each adjacent post at midheight of the fabric, and a truss consisting of a rod not less than three-eighths inch (3/8”) in nominal diameter from the adjacent post back to the gate, corner, pull, or end post, with a turnbuckle or other equivalent provision for adjustment.

F. Post Tops

Post tops shall consist of ornamental tops or combination tops and barbed-wire supporting arms, as specified. When so specified or when a top rail is to be provided, the top shall be provided with a hole suitable for the through passage of the top rail. The post tops shall fit over the outside of the posts and shall exclude moisture from the tubular posts.

G. Barbed-Wire Supporting Arms

Barbed-wire supporting arms, when specified to be furnished, shall be at an angle of approximately forty-five degrees (45°) and shall be fitted with clips or other means for attached three lines of barbed-wire. The top outside wire shall be approximately twelve inches (12”) horizontally from the fence line and the other wires spaced uniformly between the top of the fence fabric and the outside barbed wire.
H. Top Rails

Top rails shall be round (tubular), shall be in lengths not less than eighteen feet (18’), and shall be fitted with couplings for connecting the lengths into a continuous run. The coupling shall be not less than six inches (6”) long, shall provide a substantial connection, and shall allow for expansion and contraction of the rail. Suitable ties or clips shall be provided in sufficient number for attaching the fabric securely to the top rail at intervals not exceeding two feet (2’). Means shall be provided for attaching the top rail to each gate, corner, pull, and end post.

I. Stretcher Bars

Stretcher bars shall not be less than three-sixteenth inch by three-quarter inch (3/16” x 3/4”) and shall be of lengths one inch (1”) less than the full height of the fabric with which they are to be used. The stretcher bars shall be arranged for attaching the fabric to all terminal posts by threading through the fabric, by bands, or by other positive mechanical means.

J. Ties or Clips

Ties or clips of adequate strength shall be provided for attaching the fabric to lineposts.

K. Fabric Bands

Fabric bands of adequate strength shall be provided for attaching the fabric and stretcher bars to all terminal posts.

L. Tension Wires

A bottom tension wire shall be provided unless otherwise specified. Top tension wire shall be provided, when so specified, in lieu of a top rail. The tension wires shall be of coiled spring wire not less than seven (7) gage plus or minus 0.005 inch in diameter. Ties or clips shall be provided for attaching each wire to the fabric at intervals not exceeding two feet (2’).

M. Barbed Wire

Barbed wire shall consist of two (2) strands of twelve and one-half (12.5) gauge wire with fourteen (14) gauge four (4) point barbs spaced approximately five inches (5”) apart. All wire shall be zinc coated with a minimum coating of 0.80 ounces per square foot of surface area on twelve and one-half (12.5) gauge wire.

N. Vinyl Clad Fencing

Those components specified to be vinyl-clad or coated shall have a vinyl covering ten to fourteen (10-14) mils in thickness. Fabric is to be nine (9) gauge wire.
Products are to be Colorbond II as manufactured by Colorguard Corporation, or approved equal.

**Article 18.3 Tables**

**DIMENSIONS AND WEIGHTS**

<table>
<thead>
<tr>
<th>Use and Section</th>
<th>Nominal Outside Diameter/Dimensions (Inches)</th>
<th>Weight per Foot, Nominal (Pounds)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tubular End, Corner, and Pull Posts for:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>fabric height: 6 feet and less</td>
<td>Round 2.375</td>
<td>3.65</td>
</tr>
<tr>
<td>fabric height: over 6 feet</td>
<td>Round 2.875</td>
<td>5.79</td>
</tr>
<tr>
<td>Rails and Post Braces</td>
<td>Round 1.66</td>
<td>2.27</td>
</tr>
<tr>
<td>Intermediate Posts for:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>fabric height: 6 feet and less</td>
<td>Tubular Round 1.90</td>
<td>2.72</td>
</tr>
<tr>
<td></td>
<td>C-Section 1.875 x 1.625</td>
<td>2.28</td>
</tr>
<tr>
<td>fabric height: over 6 feet</td>
<td>Tubular Round 2.375</td>
<td>3.65</td>
</tr>
<tr>
<td></td>
<td>C-Section 2.25 x 1.70</td>
<td>2.64</td>
</tr>
<tr>
<td>Gate Posts with Fabric Over 6 Feet for Gate Leaf Widths:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>leaf width: 6 feet and less</td>
<td>Round 2.875</td>
<td>4.64</td>
</tr>
<tr>
<td>leaf width: over 6 to 13 feet</td>
<td>Round 4.000</td>
<td>8.65</td>
</tr>
<tr>
<td>leaf width: over 13 to 18 feet</td>
<td>Round 6.625</td>
<td>18.02</td>
</tr>
<tr>
<td>leaf width: over 18 to 24 feet</td>
<td>Round 8.625</td>
<td>27.12</td>
</tr>
<tr>
<td>Gate Frame Members for:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>fabric height: Less than 6 feet</td>
<td>Round 1.66</td>
<td>1.83</td>
</tr>
<tr>
<td>fabric height: 6 feet and over</td>
<td>Round 1.90</td>
<td>2.28</td>
</tr>
<tr>
<td>Interior Bracing:</td>
<td>Round 1.66</td>
<td>1.83</td>
</tr>
</tbody>
</table>
FENCING FABRIC SIZE

<table>
<thead>
<tr>
<th>Recommended Usage</th>
<th>Height of Fabric</th>
<th>Mesh Size</th>
<th>Gauge</th>
<th>Nominal Diameter Coated Wire (Inches)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Heavy Industrial</td>
<td>36&quot; through 144&quot;</td>
<td>2&quot;</td>
<td>6</td>
<td>0.192</td>
</tr>
<tr>
<td>Industrial/Residential</td>
<td>36&quot; through 144&quot;</td>
<td>2&quot;</td>
<td>9</td>
<td>0.148</td>
</tr>
<tr>
<td>Light Industrial/Residential</td>
<td>36&quot; through 84&quot;</td>
<td>2&quot;</td>
<td>11</td>
<td>0.120</td>
</tr>
<tr>
<td>Tennis Court</td>
<td>120&quot; through 144&quot;</td>
<td>1-3/4&quot;</td>
<td>11</td>
<td>0.120</td>
</tr>
</tbody>
</table>

Article 18.4  Construction

A.  Grading

All trees, brush and other obstacles which would interfere with the construction of the fence shall be removed and disposed of at a Contractor-provided disposal area and shall be considered incidental to the Contract. The fence shall follow a smooth profile. Throughout the fence length the distance between the ground surface and the bottom tension wire shall not be greater than four inches (4"), nor less than two inches (2"). Where excavation is necessary to meet this requirement, the ground will be graded level not less than one foot (1') on either side of the fence and backslopes of one and one-half to one (1½:1) provided. Where backfill is necessary to meet this requirement, natural surface vegetation will be removed prior to placing fill material. The top of the fill shall be level for one foot (1') on either side of the fence line and the shoulder slopes shall be one-half foot to one foot gradient (½':1'). Grading for all specific conditions shall be such that water will not be allowed to pond in the immediate area of the fence. Where drainage is required across the fence line, the Engineer shall be consulted and channels provided in accordance with his decision.

B.  Posts

All posts shall be set in Class B Portland Cement Concrete footings. The tops of the footings shall be level with the ground, shall be crowned to provide drainage and shall be troweled smooth. The dimensions of the footings shall be as shown on the Drawings. The footings shall be allowed to cure for a period of at least seven (7) days before attaching fabric.

The Contractor shall set the posts vertical and of uniform and equal height above the ground with a maximum horizontal spacing of ten feet (10') center. On straight runs, pull posts shall be provided at intervals not to exceed five hundred (500) lineal feet. Changes in line of thirty degrees (30°) or more shall be considered corner posts. Steep slopes and abrupt changes in topography may require changes in various elements of the fence. The chain link fabric shall be stretched taut and securely fastened to end, corner, or gate posts. The top edge of the fabric shall be
fastened to the top rail, and the lower edge of the fabric shall be fastened to the bottom tension wire.

C. Fabric

Place fabric on the side specified, stretched taut, and securely fastened to the posts. Fasten fabric to end, gate, corner and pull posts with stretcher bars and fabric bands spaced at intervals of fifteen inches (15") or less. Fastening to line posts shall be with ties or clips at fifteen inch (15") intervals.

Join rolls of wire fabric by weaving a single strand into the ends of the rolls to form a continuous mesh. Horizontal splices are not permitted.

D. Top Rail

Top rails shall pass through the ornamental tops of the line posts, forming a continuous brace from end to end of each stretch of fence. Join lengths of tubular top rail by sleeve couplings. Secure top rails fastened to terminal posts by pressed steel fittings or other appropriate means.

E. Tension Wire

Provide one continuous length of tension wire between pull posts. Apply sufficient tension to avoid excess sag between the posts. Tie or otherwise fasten tension wires to end, gate, corner, or pull posts by methods approved by the Engineer.

F. General Appearance

Runs of fence shall present the same general appearance and the product of one manufacturer only will be accepted, except for items which do not influence the appearance of the completed fence. No used, rerolled, or open-seam steel will be permitted in posts, gate frames, rails or braces.

Article 18.5 Measurement

Chain link fencing will be measured per linear foot, in place, from outside to outside of end or corner posts, except for the space occupied by gates.

Gates will be measured per each, complete in place for a particular size.
Article 18.6 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Unit cost payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Chain Link Fence (Include Heights and Gage)</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Gate (Type and Size)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 70.19   SILT FENCE

Article 19.1   General

The Work under this Section consists of providing all operations pertaining to construction of temporary silt fence as shown on the Drawings and specified in this Section.

Article 19.2   Materials

Materials for silt fence shall be specified on the Drawings. Silt fence filtration fabric material shall meet the following requirements:

<table>
<thead>
<tr>
<th>Property</th>
<th>Test Method</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Thickness, mils, min. (Nominal + 5%)</td>
<td>ASTM D-1593</td>
<td>30</td>
</tr>
<tr>
<td>Tensile Strength, lbs./in., min.</td>
<td>ASTM D-882</td>
<td>69</td>
</tr>
<tr>
<td>Elongation at Break, percent, min.</td>
<td>ASTM D-882</td>
<td>350</td>
</tr>
<tr>
<td>Tear Resistance, lbs., min.</td>
<td>ASTM D-1044</td>
<td>9</td>
</tr>
<tr>
<td>Low Temperature Strength</td>
<td>ASTM D-1790</td>
<td>Pass at –20° F</td>
</tr>
<tr>
<td>Permeability, cm/sec., max.</td>
<td>AASHTO M-288</td>
<td>0.000001</td>
</tr>
</tbody>
</table>

Article 19.3   Construction

Fence described in this Section shall be installed in accordance with the Drawings, or to the satisfaction of the Engineer. Silt fencing shall remain in place and in good working condition until Work is complete under the Contract. The continued maintenance of silt fence and replacement of damaged items shall be the ongoing responsibility of the Contractor. Additional metal “T” poles shall be installed in areas where additional structural support is required. All silt fences shall be removed upon final acceptance of the Project or as directed by the Engineer.

Article 19.4   Measurement

Silt fence described in this Section will be measured per linear foot, complete and in place.

Article 19.5   Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Silt Fence</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 70.20  SOIL STABILIZATION

Article 20.1  General

The Work under this Section consists of providing all operations pertaining to placing and maintaining Soil Stabilization Matting material according to the Drawings or specified herein.

Article 20.2  Material

A.  Jute Mesh

Jute Mesh shall be cloth of a uniform, open, plain weave of undyed and unbleached single jute yarn. The yarn shall be of a loosely twisted construction and it shall not vary in thickness more than one-half its normal diameter. Jute mesh shall be furnished in rolled strips and shall meet the requirements as follows:

1. Width - forty-eight inches, plus or minus one inch (48” ±1”).
2. Seventy-eight (78) warp-end per width of cloth (minimum).
3. Forty-one (41) weft-ends per yard (minimum).
4. Weight to average 1.22 pounds per linear yard with a tolerance of plus or minus five percent (±5%).

Staples shall be U-shaped and shall be approximately six inches (6”) long and one inch (1”) wide. Machine-made staples may be of No. 11 gauge or heavier steel wire. Hand-made staples shall be made from No. 9 gauge or heavier steel wire.

B.  Glass Fiber

Glass Fiber material shall consist of glass processed from the molten state into fibrous form. A multitude of continuous glass fibers (approximately sixty [60] ends) shall be collected together and wound into a package of cylindrical shape. The glass fibers shall be lightly bound together in a ribbon without the use of clay, starch or like deleterious substances and not more than three-quarters of a percent (0.75%) of saponifiable acids. The fibers shall be of a consistency suitable for application by compressed air and shall contain no petroleum solvents or other agents known to be toxic to plant or animal life.

C.  Nylon Matting

Nylon matting shall be made from Nylon 6, with a minimum content of five-tenths of a percent (0.5%) by weight of carbon black, monofilaments fused at their intersections to form a bulky mat of open construction. Nylon matting shall be furnished in rolled strips a minimum of thirty-eight inches (38”) wide.
Staples shall be a minimum of ten inches (10") in length and shall be T-staples, U-staples, or wood stakes. Metal staples shall be 8- to 11-gauge steel.

D. Excelsior Blankets

Excelsior blankets shall consist of a machine produced mat of curled wood excelsior of eighty percent (80%) six-inch (6") or longer fiber length, with consistent thickness and the fiber evenly distributed over the entire area of the blanket. The top side of each blanket shall be covered with a photodegradable extruded plastic mesh. The blanket shall be made smolder resistant without the use of chemical additives. Excelsior blankets shall be furnished in rolled strips and shall meet the requirements as follows:

1. Width – forty-eight inches, plus or minus one inch (48" ±1")
2. Length - one hundred and eighty feet (180') average
3. Weight Per Roll – seventy-eight (78) pounds, plus or minus ten percent (±10%)

Staples shall be made of wire 0.091 inches in diameter or greater, "U" shaped. Size and gauge will vary with soil conditions.

E. Erosion Control Blankets

Erosion control blanket shall be a machine-produced mat consisting of seventy percent (70%) agricultural straw and thirty percent (30%) coconut fiber. The blanket shall be of consistent thickness with the straw and coconut fiber evenly distributed over the entire area of the mat. The blanket shall be covered on the top side with UV stabilized polypropylene netting having an approximate five-eighths by five-eighths inch (5/8" x 5/8") mesh, and on the bottom with a polypropylene netting with an approximate one-half by one-half (1/2" x 1/2") mesh. The blanket shall be sewn together one and one-half inches (1.5") on centers with biodegradable thread. The straw/coconut fiber erosion control blanket shall be SC150 as manufactured by North American Green, or Owner-approved equivalent. The straw/coconut fiber erosion control blanket shall have the following properties:

<table>
<thead>
<tr>
<th>Material</th>
<th>Content</th>
</tr>
</thead>
<tbody>
<tr>
<td>Straw</td>
<td>70% (0.35 lb./yd.²)</td>
</tr>
<tr>
<td>Coconut Fiber</td>
<td>30% (0.15 lb./yd.²)</td>
</tr>
<tr>
<td>Netting</td>
<td>Top side heavyweight UV stabilized (3 lb./1,000 ft.² approximate weight). Bottom side lightweight photodegradable (1.64 lb./1,000 ft.² approximate weight).</td>
</tr>
<tr>
<td>Stitch</td>
<td>Degradable</td>
</tr>
</tbody>
</table>
Physical Specifications (Roll)

<table>
<thead>
<tr>
<th>Specification</th>
<th>Value</th>
</tr>
</thead>
<tbody>
<tr>
<td>Width</td>
<td>6.5 feet</td>
</tr>
<tr>
<td>Length</td>
<td>83.5 feet</td>
</tr>
<tr>
<td>Weight</td>
<td>30 lbs. +/- 10%</td>
</tr>
<tr>
<td>Area</td>
<td>60 yd.$^2$</td>
</tr>
<tr>
<td>Stitch Spacing</td>
<td>1.5 inch</td>
</tr>
</tbody>
</table>

F. Coir Fiber Roll

Coir (coconut) fiber rolls shall consist of long rolls of the specified diameter of coconut fiber encased in coconut fiber netting.

Fiber shall be one hundred percent (100%) mattress grade coconut fiber, six pounds per cubic foot.

Netting shall be one hundred percent (100%) coconut (coir) two-inch (2") mesh. Yarn tensile strength shall be fifty-five (55) pounds dry, forty (40) pounds wet.

Coir fiber rolls shall be BonTerra BioLogs, or an approved equal, manufactured by:

BonTerra America  
355 West Chestnut Street  
Genesee, ID 83842  
Phone: (800) 882-9489  
E-mail: bonterra@moscow.com.

Article 20.3 Construction

This Work shall be accomplished within forty-eight (48) hours after finish grading of the subgrade or topsoil has been completed.

The rates of application shall be as shown on the Drawings.

Matting material shall not be applied on days when the wind or rain would cause undue erosion or displacement of the material. The soil shall not be disturbed more than necessary. Use of vehicles and tracked equipment will be permitted by the Engineer only if such use does not cause rutting and displacement of the subgrade or topsoil.

Article 20.4 Surface Requirements

The surface shall be smoothed and all gullies and potholes backfilled prior to applying the matting. Contractor shall remove all rocks or clods larger than two inches (2") in size and all sticks and other foreign material which will prevent contact of the matting and surface. If the surface of the subgrade or topsoil is extremely dry, watering may be required by the Engineer prior to placement of the matting. Such watering will be incidental to the Work.
In some instances it may be appropriate to track-walk the prepared surface prior to seeding and stabilization. Contractor shall provide track-walking only if directed by the Drawings or the Engineer.

**Article 20.5 Application**

Contractor shall install each product in accordance with the manufacturer’s directions.

A. **Jute Mesh**

If seeding is specified, the jute matting shall be spread within twenty-four (24) hours after the seed has been placed.

Check slots shall be installed as detailed on the Drawings and shall consist of separate full-width four foot (4’) strips of jute mesh placed at right angles to the direction of water flow immediately prior to placing the general covering of jute mesh. Check slots shall be made by burying a tight fold of jute mesh vertically in the soil and tamping and stapling in place.

Check slots shall be spaced so that one check slot, junction slot, or anchor slot of the jute mesh occurs every seventy-five feet (75’) on gradients of less than four percent (4%) and every fifty feet (50’) on gradients greater than or equal to four percent (4%). On slope drains, a check slot or an end slot shall occur every twenty-five feet (25’).

Edges of matting shall be buried around the edges of catch basins and other structures as herein described. Matting must be spread evenly and smoothly and in contact with the soil at all points.

Jute matting shall be held in place by approved wire staples, pins, spikes, or wooden stakes driven vertically into the soil. Matting shall be fastened at intervals not more than three feet (3’) apart in three (3) rows for each strip of matting with one (1) row along each edge and one (1) row alternately spaced in the middle. All ends of the matting and check slots shall be fastened at six inch (6”) intervals across their width.

B. **Glass Fiber Matting**

Glass fiber matting shall be of such consistency that it can be applied by use of a blower. Other equipment capable of spreading the continuous glass fiber strands uniformly may be used if approved by the Engineer. Equipment which cuts or breaks the glass fibers shall not be permitted.

The matting shall be spread uniformly at the locations shown on the Drawings and shall be loose enough to allow sunlight to penetrate and air to circulate, but dense enough to shade the ground, reduce rate of water evaporation, and prevent or reduce water or wind erosion.
Glass fiber matting shall be held in place by the application of a CRS-1 asphalt emulsion applied at the rate shown on the Drawings. A hand-operated boom from a spreader may be used to spray the emulsion evenly over the mulch material.

All glass fibers shall be placed and tacked with emulsion in the specified areas within twenty-four (24) hours after seeding, or as directed by the Engineer.

CRS-1 Cathionic emulsion will not be measured or paid separately and is considered incidental to glass fiber matting.

C. Nylon Matting

Matting shall be installed peaked side down. Adjacent strips are to be overlapped two to three inches (2” to 3”) and ground fastened at three to five foot (3’ to 5’) intervals. The entire perimeter of the matting shall be ground fastened in twelve inch (12”) deep trenches at three to five foot (3’ to 5’) intervals and covered with soil.

If seeding is specified, the seeding shall be accomplished within twenty-four (24) hours after placing the nylon matting.

D. Excelsior Blankets

Excelsior blankets shall be unrolled with the netting on top and the fibers in contact with the soils over the entire area. In ditches, the blankets shall be applied in the direction of flow, butted at ends and sides. On slopes, the blankets shall be applied either horizontally or vertically to the slope; ends and sides shall be butted. Staples shall be spaced approximately two linear yards apart, on each side, and one row in the center alternately spaced between each side. Use a common row of staples on adjoining blankets.

If seeding is specified, the excelsior blankets shall be placed within twenty-four (24) hours after the seed has been placed.

E. Erosion Control Blankets

Erosion control blankets shall be spread uniformly at the locations described on the Drawings and shall be loose enough to allow sunlight to penetrate and air to circulate, but dense enough to shade the ground, reduce rate of water evaporation, and prevent or reduce water or wind erosion.

Erosion control blankets shall be unrolled with the netting on top and the fibers in contact with the soils over the entire area. On slopes, the blankets shall be applied vertical to the slope; ends and sides shall be butted. Staples shall be spaced approximately two linear yards apart, on each side, and one row in the center alternately spaced between each side. Use a common row of staples on adjoining blankets.
F. Coir Fiber Roll

The Contractor shall minimize Work site disturbance. The Contractor shall protect existing plants and avoid additional disturbance that can lead to erosion and sedimentation. The Contractor shall prepare the site for installation of the coir fiber roll by removing any large rocks, obstructions, or material that may prevent the coir from making direct and firm contact with the soil.

The Contractor shall install coir rolls level along a horizontal contour. Contractor shall place coir rolls approximately parallel to shoreline. The Contractor shall install coir roll such that approximately one-third (1/3) of its height extends above the mean water elevation.

The Contractor shall select and use wooden stakes made from strong, durable wood species that do not have knots or flaws. The stakes shall be pointed at one end, not wedge shaped. Stakes for coir rolls shall be approximately one and one half by one and one-half inches (1.5” x 1.5”), unless otherwise specified. Stake length shall be specified on the Drawings.

Place live willow stakes in the coir fiber roll by pulling the roll’s fibers apart. If necessary, wedge a pilot bar back and forth to create a hole for the willow stake. Drive or place the willow stake to the base of the coir roll. If driven, Contractor shall ensure that willow stake is not damaged.

Article 20.6 Maintenance and Repair

The Contractor shall maintain the areas covered by matting until all Work on the Project has been completed and accepted. Prior to acceptance of the Work, the damaged areas shall be reshaped, reseeded, and the matting satisfactorily repaired or replaced as herein specified with no additional compensation.

Article 20.7 Measurement

The quantity of Soil Stabilization Matting to be paid for shall be the number of units of one thousand (1,000) square feet, measured to the nearest one-tenth (0.1) unit on the slope of the ground surface.

Method of measurement shall be by length per linear foot of Coil Fiber Roll of the specified diameter complete and accepted in its final position. The bid item shall include all furnishing and placing coir fiber roll, willow stakes, wood stakes, fiber netting, and all other materials necessary to complete the Work described in this Section.
Article 20.8 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Unit cost payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Soil Stabilization Matting (Type)</td>
<td>1,000 Square Feet</td>
</tr>
<tr>
<td>Coir Fiber Roll (Diameter) with Willow Stake</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
MUNICIPALITY OF ANCHORAGE
STANDARD SPECIFICATIONS

DIVISION 70
MISCELLANEOUS
STANDARD DETAILS
70-1 Standard Location for New Utilities
70-2 Locations for Existing Utilities (Approval Required for New Utilities)
70-3 Typical Water and Sewer Locations
70-4 Adjustment for Gas Valve Key Box (1/4" thru 4")
70-5 Adjustment for Gas Valve Manhole
70-6 Standard Method for Shoring Phone/Conduit ACS Approved Method and Procedure #86-1
70-7 Striping Notes
70-8 Intersection Approach Striping
70-9 Left-Turn Pocket Approach Striping
70-10 Raised Median Approach and Two Lanes Drive to Right Striping
70-11 Left-Turn Pocket Approach from Two Way Center Left-Turn Lane Striping
70-12 Two Way Center Left Turn Lane Striping
70-13 Passing on Both Sides of an Obstruction
70-14 Layout Templates for Stencils
70-15 Approach to Railroad Crossing on Two Lane Two Way Highway
70-16 Typical Uncurbed Return with Pathway
70-17 Typical Uncurbed Return without Pathway
70-18 Typical Curb Return with Sidewalk
70-19 Typical Curb Return without Sidewalk
70-20 Post Mounted Sign with Shoulder
70-21 Post Mounted Sign with No Shoulder
70-22 Post Mounted Sign with Guardrail
70-23 Post Mounted Sign Curb without Sidewalk
70-24 Post Mounted Sign Curb with Parkway and Sidewalk
70-25 Post Mounted Sign Curb with Sidewalk without Parkway
70-26 Post Mounted Sign Raised Medians
70-27 Post Mounted Sign Secondary Panel Height and Sign Positioning
70-28 Street Name Signs
70-29 Sign on Round Post
70-30 Street Name on Round Post
70-31 Concrete Foundation for Sign Post
70-32  12" Plate Installation Detail with Supplemental 8" D3-D1 Signs
70-33  Signal Pole Mast Arm Sign Mounting
70-34  Wood Bollard
70-35  Removable Wood Bollard
70-36  Removable Steel Rectangular Bollard
70-37  Steel Bollard
70-38.1 Removable Circular Bollard (Sheet 1 of 2)
70-38.2 Removable Circular Bollard (Sheet 2 of 2)
70-39  Guard Rail Detail
70-40  Cluster Mailbox Location
70-41  Fence Details
70-42  Fence Details
NOTES:
1. OFFSETS ARE TO CENTER OF UTILITY.
2. ADDITIONAL RIGHT-OF-WAY/EASEMENT MAY BE REQUIRED TO MEET MINIMUM SET-BACKS FOR HYDRANTS.
3. SET HYDRANT A MINIMUM OF 5' FROM BACK OF CURB OR 1' FROM EDGE OF SIDEWALK.
4. MAINTAIN SEPARATION DISTANCES AS IDENTIFIED IN THE AWWA DESIGN AND CONSTRUCTION PRACTICES MANUAL, ADEC 18 AAC 72, AND 18 AAC 80.
5. LOCATE SANITARY SEWER BETWEEN 5' AND 6' FROM CENTERLINE. MANHOLE LIDS SHALL BE LOCATED IN THE CENTER OF A TRAVEL LANE. IF THERE IS A TWO-WAY CENTER TURN LANE, THE MANHOLE LIDS SHALL BE PLACED APPROXIMATELY ON THE LINE BETWEEN LANES.
6. ADA REQUIREMENTS SHALL GOVERN WHEN PLACING STORM DRAIN MANHOLE LIDS AND CATCH BASINS IN PEDESTRIAN FACILITIES.

LEGEND:
- G - GAS LINE
- S - SANITARY SEWER LINE
- SD - STORM DRAIN LINE
- W - WATER LINE
- C - R.O.W. CENTERLINE

MUNICIPALITY OF ANCHORAGE
SCALE: NTS
APPROVED:
REVISED: 8/08
SECTION # DIV 50, 55, 60 & 70
DETAIL # 70-1

STANDARD LOCATION FOR NEW UTILITIES
NOTES:
1. OFFSETS ARE TO CENTER OF UTILITY.
2. ADDITIONAL RIGHT-OF-WAY MAY BE REQUIRED TO MEET MINIMUM SET-BACKS FOR HYDRANTS.
3. SET HYDRANT A MINIMUM OF 5' FROM BACK OF CURB OR 1' FROM EDGE OF SIDEWALK.
4. LOCATE UTILITIES TO MAINTAIN MINIMUM SEPARATIONS AS IDENTIFIED IN THE AWWU DESIGN AND CONSTRUCTION PRACTICES MANUAL, ADEC 18 AAC 72, AND 18 AAC 80.
NOTES:

1. SANITARY SEWER SERVICE CONNECTIONS SHALL BE FIVE TO FIFTEEN FEET (5' TO 15') FROM PROPERTY CORNER, IN LOWER ONE–THIRD OF THE LOT TO BE SERVED.

2. WATER SERVICE CONNECTIONS SHALL BE A MINIMUM OF FIVE FEET (5') FROM PROPERTY CORNER OF THE LOT TO BE SERVED.

3. WATER AND SEWER SERVICES SHALL MAINTAIN A MINIMUM TEN FOOT (10') SEPARATION.

4. WATER AND SEWER SERVICES SHALL MAINTAIN A MINIMUM FIFTEEN FOOT (15') SEPARATION FROM FIRE HYDRANTS.

5. LOCATE WATER AND SANITARY SEWER SERVICE TO MAINTAIN A MINIMUM TEN FOOT (10') SEPARATION BETWEEN OUTSIDE OF PIPE AND STORM SEWER, CATCH BASINS, MANHOLES, STREET LIGHTING, UTILITY POLES, UTILITY PEDESTALS, METER BASES AND TRANSFORMER PADS.

6. FOR STANDARD LOCATION AND SEPARATION FOR SANITARY SEWER, SEE NOTE 5 ON STANDARD DETAIL 70–1.
INSTALL AND ADJUST ROADWAY BOX SLIDING SLEEVE TO GRADE BY PAVING CONTRACTOR. (FURNISHED BY GAS COMPANY)

INSTALL CUT SECTION OF 4" C.I. SOIL PIPE, FURNISHED BY GAS COMPANY BY PAVING CONTRACTOR.

C.I. VALVE BOX BOOT TO BE INSTALLED BY GAS COMPANY

FEATHER PAVEMENT
FINISH SURFACE

1/2"

GAS LINE

ADJUSTMENT FOR GAS VALVE KEY BOX (1/4" thru 4")
24" Ø C.I. MANHOLE FURNISHED BY GAS COMPANY; PAVING CONTRACTOR SHALL FURNISH GROUT & SET MANHOLE RING

FEATHER PAVEMENT

FINISH SURFACE

1/2"

ONE OR MORE COURSES OF BRICK

APPROX. 2" GROUT

GROUT

30" I.D. CONC. SEWER PIPE INSTALLED BY GAS CO.
NOTES:
1. SUPPORT DUCTS WITH 2”x4” AND STRAPS AT JOINTS BEFORE EXCAVATING UNDER DUCTS.
2. PLACE AND COMPACT CLASSIFIED MATERIAL UNDER DUCT BANK UP TO WITHIN 18" OF DUCT. THE LAST 18" TO BE CONCRETE OR CONCRETE SLURRY.
3. DUCTS TO BE ENCASED IN 3" OF SAND (ON ALL SIDES).
GENERAL NOTES:

1. ALL ONLYS, ARROWS, CROSSWALKS, STOP BARS AND OTHER MARKINGS SHALL BE AS INDICATED IN THE DRAWINGS AND SPECIFICATIONS.
2. DUAL–TURN LANE, TURN POCKET REVERSAL AND CENTERLINE DOUBLE LINES SHALL BE TWO 4" YELLOW LINES SEPARATED BY A 4" SPACE.
3. REGULAR LANE LINES SHALL BE A WHITE 4" WIDE LINE. SKIP LINE SPACING SHALL BE A 10’ LINE AND A 30’ SPACE.
4. MEDIAN NOSE AND THE TOP AND FACE OF CURB OF ALL MEDIAN ISLAND NOSING SHALL BE YELLOW A MINIMUM OF 5’ BACK FROM THE FLOWLINE.
5. "W" IS THE WIDTH OF THE LANE MEASURED FROM CENTER LANE LINE TO CENTER LANE LINE OR FROM CENTER LANE LINE TO EDGE OF PAVEMENT.
6. "L" IS TAPER LENGTH. "L" IS DETERMINED BY TAKING THE SPEED LIMIT (M.P.H.) TIMES THE OFFSET WIDTH IN FEET OR AS INDICATED ON DRAWINGS. MINIMUM "L" IS 100’.
7. "D" IS THE LANE REVERSAL DISTANCE. A RATIO OF 8:1 FOR THE ENTRANCE TAPER IS DESIRABLE WITH A MINIMUM RATIO OF 3:1.
8. "S" IS THE SHY DISTANCE MEASURED FROM THE FACE OF CURB TO THE CENTER OF THE OUTSIDE YELLOW LINE. "S" IS 18” OR AS SHOWN ON THE DRAWINGS.
9. THESE NOTES APPLY TO STANDARD DETAILS 70–8 THROUGH 70–15.
1. ARROW MARKINGS SHALL BE PLACED AT THE START OF AND AT THE INTERSECTION OF AUXILIARY TURN Lanes. ADDITIONAL ARROW MARKINGS SHALL BE PLACED AND SPACED PROPORTIONATELY WHEN AUXILIARY TURN Lanes EXCEED 150'.

2. REGULAR LANE LINES SHALL BE A SOLID WHITE 4” LINE.

3. LEFT AND RIGHT TURN DELINEATOR LINES SHALL BE A SOLID WHITE 8” LINE.

4. 4” SOLID WHITE LINES APPROACHING INTERSECTIONS ARE 100’ IN LENGTH MINIMUM.

5. 4” SOLID WHITE LINES DEPARTING INTERSECTIONS ARE 50’ IN LENGTH PLUS THE PARTIAL SKIP CYCLE LENGTH.

6. DOTTED GUIDELINES USED FOR DUAL TURN Lanes SHALL BE 8” WHITE SKIP WITH A 1’ LINE AND A 3’ SPACE.

7. STRAIGHT AHEAD AND COMBINATION ARROWS SHALL BE USED ONLY WHEN INDICATED ON DRAWINGS.

8. "ONLY" WORD MARKINGS ARE USED ONLY WHEN A THROUGH LANE TERMINATES AS A TURN.
LEFT-TURN POCKET APPROACH

SEE STANDARD DETAIL 70–7 FOR GENERAL STRIPING NOTES
RAISED MEDIAN APPROACH

SEE STANDARD DETAIL 70–7 FOR GENERAL STRIPING NOTES
LEFT-TURN POCKET APPROACH FROM TWO WAY CENTER LEFT-TURN LANE

SEE STANDARD DETAIL 70–7 FOR GENERAL STRIPING NOTES
TWO WAY CENTER LEFT TURN LANE

1. INSTALL DUAL TURN LANE ARROWS EVERY 200', UNLESS OTHERWISE SPECIFIED.
PASSING ON BOTH SIDES OF AN OBSTRUCTION
LAYOUT TEMPLATES FOR STENCILS

1. ALL SYMBOLS SHOWN SHALL BE WHITE AND REFLECTORIZED.
2. RIGHT AND LEFT TURN AUXILIARY LANE MARKINGS ARE IDENTICAL BUT REVERSED.
NOTES:
1. ALL MARKINGS SOLID WHITE UNLESS INDICATED OTHERWISE.
2. ON 4–LANE ROADWAYS THE RAILROAD CROSSING APPROACH MARKING SHALL BE PLACED IN EACH LANE OF THE APPROACH.
3. LOCATE STOP BAR 15' FROM RAILROAD TRACK OR 8' FROM GATE, IF PRESENT.
4. EDGE LINES AND LANE LINES ON A UNI–DIRECTIONAL APPROACH SHALL BE PLACED IN A NORMAL MANNER EXCEPT THAT THE LANE LINE(S) SHALL BE SOLID 4" WHITE IN LIEU OF BROKEN FOR A DISTANCE = (D+60') IN ADVANCE OF THE STOP BANDS.

<table>
<thead>
<tr>
<th>POSTED LIMIT</th>
<th>D</th>
</tr>
</thead>
<tbody>
<tr>
<td>30 M.P.H.</td>
<td>225'</td>
</tr>
<tr>
<td>40</td>
<td>350'</td>
</tr>
<tr>
<td>50</td>
<td>475'</td>
</tr>
<tr>
<td>60</td>
<td>625'</td>
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</tbody>
</table>

RAILROAD SYMBOL
TYPICAL UNCURBED RETURN WITH PATHWAY

1. WHEN THE SIDE STREET LACKS EDGE LINES, BEGIN AND END THE MAIN STREET EDGE LINES AT THE MAIN STREET PAVEMENT RETURNS AND EXTEND THE STOP BAR TO THE EDGE OF PAVEMENT AS SHOWN IN THE UNCURBED RETURN WITHOUT PATHWAY DETAIL.

2. INSTALL DETECTABLE WARNING TILES IN PATHWAYS AT THE LOCATIONS SHOWN. INSTALL ACCORDING TO THE MANUFACTURER’S WRITTEN INSTALLATION INSTRUCTIONS.

3. BREAK THE MAIN STREET CENTERLINE MARKINGS AT SIDE STREET INTERSECTIONS ONLY WHEN LEFT TURN LANE ARE PROVIDED. OTHERWISE, INSTALL THEM CONTINUOUSLY THROUGH THE INTERSECTION.

4. LOCATE STOP SIGNS SO THEY ARE:
   A. VISIBLE TO APPROACHING TRAFFIC AND
   B. AS NEAR TO THE STOP BAR AS PRACTICABLE.

5. WHEN THE SIDE STREET LACKS THE RIGHT OF WAY TO INSTALL THE STOP SIGN AT THE 13.5' OFFSET, REDUCE THE OFFSET TO FIT THE SIGN WITHIN THE RIGHT OF WAY. IF SIGHT OBSTRUCTIONS OBSCURE THE SIGN, RELOCATE THE SIGN SO IT IS VISIBLE.
TYPICAL UNCURBED RETURN WITHOUT PATHWAY

NOTES:
1. BREAK THE MAIN STREET CENTERLINE MARKINGS AT SIDE STREET INTERSECTIONS ONLY WHEN LEFT TURN LANES ARE PROVIDED. OTHERWISE, INSTALL THEM CONTINUOUSLY THROUGH THE INTERSECTION.
2. LOCATE STOP SIGNS SO THEY ARE:
   A. VISIBLE TO APPROACHING TRAFFIC AND
   B. AS NEAR TO THE STOP BAR AS PRACTICABLE.
3. WHEN THE SIDE STREET LACKS THE RIGHT OF WAY TO INSTALL THE STOP SIGN AT THE 13.5’ OFFSET, REDUCE THE OFFSET TO FIT THE SIGN WITHIN THE RIGHT OF WAY. IF SIGHT OBSTRUCTIONS OBSCURE THE SIGN, RELOCATE THE SIGN SO IT IS VISIBLE.
TYPICAL CURBED RETURN WITH SIDEWALK

NOTES:

1. WHEN SIDEWALKS ARE SEPARATED FROM THE BACK OF CURB, JOIN THE TWO AS SHOWN USING A REVERSE CURVE WITH A MINIMUM RADIUS OF 30'.

2. BREAK THE MAIN STREET CENTERLINE MARKINGS AT SIDE STREET INTERSECTIONS ONLY WHEN LEFT TURN LANES ARE PROVIDED. OTHERWISE, INSTALL THEM CONTINUOUSLY THROUGH THE INTERSECTION.

3. LOCATE STOP SIGNS SO THEY ARE
   A. VISIBLE TO APPROACHING TRAFFIC AND
   B. AS NEAR TO THE STOP BAR AS PRACTICABLE.
TYPICAL CURBED RETURN WITHOUT SIDEWALK

NOTES:

1. BREAK THE MAIN STREET CENTERLINE MARKINGS AT SIDE STREET INTERSECTIONS ONLY WHEN LEFT TURN LANES ARE PROVIDED. OTHERWISE, INSTALL THEM CONTINUOUSLY THROUGH THE INTERSECTION.

2. LOCATE STOP SIGNS SO THEY ARE:
   A. VISIBLE TO APPROACHING TRAFFIC
   B. AS NEAR TO THE STOP BAR AS PRACTICABLE.

3. WHEN THE SIDE STREET LACKS THE RIGHT OF WAY TO INSTALL THE STOP SIGN AT THE 13.5' OFFSET, REDUCE THE OFFSET TO FIT THE SIGN WITHIN THE RIGHT OF WAY. IF SIGHT OBSTRUCTIONS OBSCURE THE STOP SIGN, RELOCATE THE SIGN SO IT IS VISIBLE.
NOTES:

1. UNLESS SHOWN OTHERWISE ON THE DRAWINGS, THE STANDARD SIGN OFFSET IS 12'.  THE MINIMUM IS 6'.

2. IF SIGNS EXTEND OVER SIDEWALKS, THE MINIMUM VERTICAL CLEARANCE IS 7'–0".

3. ADD 6" TO MOUNTING HEIGHT ON UNPAVED ROADS.

4. IF SIGNS EXTEND OVER BIKE PATHS, THE MINIMUM VERTICAL CLEARANCE IS 8'–0".

5. PAINT ALL SIGN MOUNTING FASTENERS ON SIGN FACE A COLOR MATCHING THE SIGN FACE.

6. ATTACH ALL SIGNS ZEES AND BRACES MOUNTED TO THE POSTS WITH 5/16" BOLTS WITH SELF-LOCKING NUTS.
NO SHOULDER

NOTES:


2. IF SIGNS EXTEND OVER SIDEWALKS, THE MINIMUM VERTICAL CLEARANCE IS 7’-0”.

3. ADD 6” TO MOUNTING HEIGHT ON UNPAVED ROADS.

4. IF SIGNS EXTEND OVER BIKE PATHS, THE MINIMUM VERTICAL CLEARANCE IS 8’-0”.

5. PAINT ALL SIGN MOUNTING FASTENERS ON SIGN FACE A COLOR MATCHING THE SIGN FACE.

6. ATTACH ALL SIGNS ZEES AND BRACES MOUNTED TO THE POSTS WITH 5/16” BOLTS WITH SELF-LOCKING NUTS.
NOTES:
1. IF SIGNS EXTEND OVER SIDEWALKS, THE MINIMUM VERTICAL CLEARANCE IS 7’-0”.
2. ADD 6” TO MOUNTING HEIGHT ON UNPAVED ROADS.
3. IF SIGNS EXTEND OVER BIKE PATHS, THE MINIMUM VERTICAL CLEARANCE IS 8’-0”.
4. PAINT ALL SIGN MOUNTING FASTENERS ON SIGN FACE A COLOR MATCHING THE SIGN FACE.
5. ATTACH ALL SIGNS ZEES AND BRACES MOUNTED TO THE POSTS WITH 5/16” BOLTS WITH SELF-LOCKING NUTS.
CURB WITHOUT SIDEWALK

NOTES:
1. PAINT ALL SIGN MOUNTING FASTENERS ON SIGN FACE A COLOR MATCHING THE SIGN FACE.
2. ATTACH ALL SIGNS ZEES AND BRACES MOUNTED TO THE POSTS WITH 5/16" BOLTS WITH SELF-LOCKING NUTS.
CURB WITH PARKWAY AND SIDEWALK

NOTES:

1. IF PARKWAY IS LESS THAN 5', SIGNS SHOULD BE PLACED BEHIND SIDEWALK.
2. IF SIGNS EXTEND OVER SIDEWALKS, THE MINIMUM VERTICAL CLEARANCE IS 7'-0".
3. IF SIGNS EXTEND OVER BIKE PATHS, THE MINIMUM VERTICAL CLEARANCE IS 8'-0".
4. PAINT ALL SIGN MOUNTING FASTENERS ON SIGN FACE A COLOR MATCHING THE SIGN FACE.
5. ATTACH ALL SIGNS ZEES AND BRACES MOUNTED TO THE POSTS WITH 5/16" BOLTS WITH SELF-LOCKING NUTS.
Curb with Attached Sidewalk

NOTE:
1. Sign face shall be placed outside sidewalk except when row limitations exist; for example in the central business district or other similar districts.
RAISED MEDIAN

PRIMARY PANEL

7' MIN.

4' MIN.

1' MIN

5' MIN

MEDIAN NOSE

OBJECT MARKERS

Ø OF SIGN AND MEDIAN.
SECONDARY PANEL HEIGHT
ALL TWO PANEL MOUNTING

SIGN POSITIONING

POST MOUNTED SIGN
SECONDARY PANEL HEIGHT
AND SIGN POSITIONING
CAST SIGN BRACKETS AND BASE ALUMINUM ALLOY 356-T6

(S3-1D)
STREET NAME SIGN

SQUARE TUBE CAP MAY VARY IN SIZE

TELSPAR

STREET NAME SIGNS

MUNICIPALITY OF ANCHORAGE

SCALE: NTS
APPROVED:
REVISED: 10/08

SECTION # 70.11
DETAIL # 70-28
POST

STEEL SADDLE MOUNTING BRACKET

STAINLESS STEEL BAND

SIGN ON ROUND POST

SECTION #
70.11

DETAIL #
70-29
CAST SIGN BRACKET ALUMINUM ALLOY 356-T6 CAN BE ATTACHED TO POST WITH STAINLESS STEEL STRAPS.
CONCRETE FOUNDATION
FOR SIGN POST CURB INLET FRAME

PERFORATED STEEL TUBES (P.S.T.)
(12ga. – .105” Wall Thickness)

<table>
<thead>
<tr>
<th>SIGN SURFACE AREA</th>
<th>POST SIZE</th>
<th>EMBEDMENT DEPTH</th>
<th>CONCRETE DEPTH</th>
</tr>
</thead>
<tbody>
<tr>
<td>SQ. FT.</td>
<td>2” X 2”</td>
<td>27”</td>
<td>24”</td>
</tr>
<tr>
<td>7’ OR LESS</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>GREATER THAN 7’</td>
<td>2½” X 2½”</td>
<td>33”</td>
<td>30”</td>
</tr>
</tbody>
</table>
RIVET DETAIL
ELEVATION VIEW

VIEW A-A

PLAN VIEW

12" SIGNS/PLATES 36" OR GREATER IN LENGTH
BRACING DETAIL

CAST POST TOP SIGN BRACKET
ALUMINUM ALLOY 356-T6

INSTALL STREET NAME SIGN
(OR 2 WITH SECOND BRACKET)

INSTALL TWO 12" D3-1D OR 12" PLATES BACK TO BACK ON THE POST

FOR POST SIZE REFER TO STANDARD DETAIL 70-31

12' PLATE INSTALLATION DETAIL WITH SUPPLEMENTAL 8' D3-1D SIGNS

MUNICIPALITY OF ANCHORAGE

SECTION # 70.11
DETAIL # 70-32
1/4" STEEL PLATE 2"X 5"

3/16"
ALUMINUM POLE PLATE
BAND BUCKLE

3/4" STAINLESS BAND, .030" THICK DOUBLE WRAPPED AROUND MAST ARM (INSTALL 2 BANDS AROUND EACH POLE PLATE)

1 1/2" STEEL PIPE, 1 1/2" LONG

3/8" X 2 1/2" BOLTS WITH SELF LOCKING NUTS

1 1/2" PERFORATED TUBE, 12 GAUGE
WOOD BOLLARD

45° CHAMFER

1 1/2"

8" x 8" ROUGH SAWN, PRESSURE TREATED WOOD POST

TYPE II CLASSIFIED FILL/BACKFILL COMPACTED TO MINIMUM 95% MAX DENSITY

2' 6"

3'

6'

1' 6"
3 DOT RED REFLECTOR, OM-2R 6"x15", LAG BOLT AT 3 POINTS

8"x8" ROUGH SAWN, PRESSURE TREATED WOOD POST

1/2"x6" GALVANIZED, DROPFORGED EYEBOLT

1/2" THICK FLAT IRON, WELDED ON BOTH SIDES TO METAL SLEEVE. 1" DIA. HOLE TO RECEIVE PADLOCK OF 4" OVERALL SIZE

FINISH SURFACE

8-7/8"x 8-7/8" GALVANIZED SQUARE SLEEVE OR EQUAL. SET 1/2" ABOVE TRAIL SURFACE AND ALLOW 1/4" GAP ON ALL SIDES BETWEEN SLEEVE AND POST

(2) 1"X12" GALVANIZED STEEL BOLTS TO PROVIDE SOLID BASE FOR POST REST

TYPE II CLASSIFIED FILL.BACKFILL COMPACTED TO MIN. 95% MAX. DENSITY

NOTES
1. LOCATE PADLOCK AWAY FROM TRAFFIC FLOW.
NOTES:

1. POSTS SHALL BE PRIMED AND RECEIVE TWO COATS MINIMUM OF DARK GREEN POWDER COAT PAINT I.A.W. MANUFACTURER’S RECOMMENDATIONS. TOPCOAT WITH TWO COATS YELLOW CARBOLINE 139 UNLESS GUARD POST COVER OR SLEEVE IS INSTALLED.
NOTES:
1. ALL WELDS, UNLESS OTHERWISE SHOWN, SHALL BE THREE-SIXTEENTH INCH (3/16") FILLET – ALL AROUND.
2. CAST IN PLACE WITH CLASS 'A' CONCRETE IN A TWELVE INCH DIAMETER BY THIRTY-SIX INCH (12" X 36") FOUNDATION TUBE.
3. REMOVABLE BOLLARDS ARE TO HAVE ADHESIVE REFLECTORS ON BOTH FRONT AND BACK OF POST.
4. ALL EXTERIOR CORNERS AND EDGES SHALL BE ROUNDED TO PROVIDE A PROJECTION FREE SURFACE.
NOTE: ALL FINAL FABRICATIONS TO BE GALVANIZED PRIOR TO ASSEMBLY

REMOVABLE BOLLARD ASSEMBLY

SECTIONS

- 1 3/4"
- 4"
- 1 1/2" Ø SCHEDULE 40 PIPE
- 1/4" THICK
- 3/8"
- R 3/8"
- 3/8"
- 1 1/2"
- 1 3/4"
- 1 1/2"
- 3 1/2"
- 3 1/4"
- 2 8"
- 5/8"

LATCH ENLARGEMENT

not to scale

LATCH

1/2" HOLE TO ACCEPT PADLOCK

2' 6"

6" TYP.

SEWER ROCK

TYPE IIA CLASSIFIED FILL, COMPACTED

7" Ø (I.D.) BOLLARD COVER ATTACHED WITH 2 SET SCREWS

BOLLARD

1/2" Ø X 7 1/2 " THREAD ROU AND 2 FLUSH BOLTS, ONE SECURED TO BOLLARD WITH TACK WELD

LATCH
NOTE: PROVIDE 1 TEMPORARY CAP PER REMOVABLE BOLLARD TO OWNER.

REMOVABLE BOLLARD TEMPORARY CAP

PLAN VIEW

SECTIONS

1/8" WELD TO TOP OF PIPE, GRIND EDGES SMOOTH
6"Ø SCHEDULE 40 PIPE
HOLE TO RECEIVE 1/2"Ø THREADED ROD
HOLE TO RECEIVE 1/4"Ø SET SCREW

BOLLARD

CROSS-SECTION: TOP
1/8" Ø TABS WITH HOLE THREADED TO ACCEPT 3/8" Ø MACHINE BOLT

CROSS-SECTION: BOTTOM
1/8" Ø WELD TO BASE OF PIPE

SECTIONS

1/2"Ø X 7.625" STEEL ROD, WELD TO TUBING WALL
7"Ø SCHEDULE 40 PIPE

NOTE: SET SLEEVE 1/8" ABOVE ADJACENT SURFACE

BOLLARD SLEEVE

MUNICIPALITY OF ANCHORAGE

SCALE: NTS
APPROVED:
REVISED: 10/08

REMOVABLE BOLLARD (ROUND)
SHEET 2 OF 2

SECTION # 70.10
DETAIL # 70–38.2
WOOD POST

STEEL POST

NOTES:
1. PROVIDE GUARD RAIL OF TYPE AND GAUGE SPECIFIED IN THE CONTRACT DOCUMENTS.
2. SEE ADOT&PF STANDARD DRAWING G–10 FOR GUARDRAIL POST LENGTHS CORRESPONDING TO COMBINATIONS OF SLOPE AND BEHIND–POST EMBANKMENT WIDTHS.
3. SEE ADOT&PF STANDARD DRAWING G–00 FOR STANDARD W–BEAM END SECTION OR TERMINAL CONNECTOR.
NOTE:
1. COORDINATE WITH LOCAL POSTAL AUTHORITIES TO DETERMINE LOCATIONS OF CLUSTER MAILBOXES. IF CLUSTERING OR SPECIAL LOCATIONS ARE SPECIFIED, PROVISIONS SHALL BE INCLUDED IN THE CONTRACT DOCUMENTS.
NOTES:
1. GAUGE OF FABRIC AS SPECIFIED ON DRAWINGS.
2. SIZE OF TUBULAR STEEL FOR GATE FRAMES IS SPECIFIED IN SECTION 70.18 – CHAIN LINK FENCES.
NOTE:
1. SIZE OF TUBULAR STEEL FOR GATES IS SPECIFIED IN SECTION 70.18 – CHAIN LINK FENCES.
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SECTION 75.01 GENERAL

Article 1.1 Scope of Work

The work covered by these Specifications consists of providing all plants, labor, equipment, supplies, material, transportation, handling and storage, and performing all operations in connection with the construction of the landscaping improvements as provided for in this Division.

Article 1.2 Glossary of Terms

Reference the following for terminology


Article 1.3 Payment - General

Payment for all Work (including the plant establishment period) included in this Division shall be paid for in accordance with the Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described.
SECTION 75.02    LANDSCAPING

Article 2.1    General

A. Scope of Work

The work under this Section consists of providing all operations pertaining to the planting and maintenance of trees, shrubs, ground cover, perennials and annuals. The Work shall also include all operations pertaining to removing and replacing trees and protecting existing trees in place.

B. Agency Standards: Nomenclature

All operations shall meet industry consensus on the planting and care operations and all plant materials used shall be true to name and size in conformity with the following standards:


C. Definition

The term "planting areas" as used in this Specification, shall mean all areas to be planted with trees, shrubs, ground cover, annuals, perennials, bulbs, native plant materials and/or seeded areas.

Article 2.2 Materials

A. Plant Materials

1. Plant Schedule

A complete schedule of plants, including quantities, sizes, and other requirements, is shown on the Landscape Drawings. The Contractor shall furnish a report listing the proposed sources of the plants and location grown. In the event of plant count discrepancy between the Plant Schedule and the plants counted on the Drawings, the Drawings shall prevail. Discrepancies between the quantity shown on the Plant Schedule and those required by the Drawing shall not entitle the Contractor to claim any additional compensation, nor relieve him of the obligation to complete the Work shown on the Drawings.

No substitutes shall be accepted, except with the written permission of the Engineer. The Contractor shall submit all substitution requests, noting the source of plants, location, size, and condition, within thirty (30) days of receiving the Notice to Proceed.

Each plant shall have a durable legible label with plant size and name (genus, species, variety, cultivar) securely attached when delivered and in place until after acceptance. Labels shall not girdle or damage plants.

2. Plant Quality

a. Species and Culture

All plants shall be true to species and variety specified and nursery grown in accordance with good horticultural practices.

Plants shall be so trained in development and appearance as to be compact and symmetrical. They shall be sound, healthy, vigorous, well-branched, and densely foliated when in leaf. Plants shall be free of disease and insect adults, eggs, pupate, or larvae. They shall have healthy, well developed root systems and shall be free from physical damage or other conditions that would prevent thriving growth.

All plant material shall be free of contamination by any plant not specified, including non-native invasive plants, seeds, and plant parts.
b. Tree Trunk Specifications

Tree trunks shall be straight and well-tapered. Trees with multiple leaders, unless specified, will be rejected. Damaged, cut, or crooked leaders; included bark, bark abrasions, sunscald, disfiguring knots, mold, and prematurely opened buds, or cut limbs over three-quarter inch (3/4") diameter that are not completely callused are cause for rejection.

Trunk diameter and taper shall be sufficient so that tree remains vertical without support.

c. Crown Configuration Specification

Crown ratio (distance from bottom of canopy to tree top/tree height) should be at least sixty-six percent (66%) of total tree height.

One-half (1/2) or more of the foliage should be on branches originating on the lower two-thirds (2/3) of the trunk, and one-half (1/2) or less should originate on the upper one third (1/3).

d. Branch Specifications

All branches shall be less than two thirds (2/3) the diameter of the trunk measured one inch (1") above the branch.

The attachment of branches shall contain no bark inclusions.

There shall be one dominant leader straight to the top of the tree. Larger branches shall be spaced at least six inches (6") apart and arranged radially and vertically along the stem.

There shall be live buds or foliage to the ends of all twigs.

All pruning cuts shall comply with ANSI A300 Standards and shall be made outside the branch collar and branch bark ridge. Branches shall not be pruned immediately prior to delivery. Needled evergreens shall not have been sheared.

e. Root Flare and Root Ball Specifications

No root in the root ball larger than one-quarter inch (1/4") diameter shall circle more than one-third around the root ball. There shall be no roots in the root ball that make an abrupt turn of ninety degrees (90°) or more. There shall be no roots in the root ball that are ‘J’ roots, kinked, stem girdling, or potentially stem girdling. For all plants,
including bare root, roots shall exhibit healthy, well-branched root systems characteristic of the species with adequate spread.

Roots in ball-in-burlap root balls or in containers shall be well-rooted and uniform throughout soil mix or growth media. Roots of trees produced in containers shall have been held in the container for a period of one growing season and no longer. Roots shall not be pot bound.

Major roots shall be less than two inches (2") from the top of root flare. The root flare (trunk flare, root collar) shall be at the surface of the root ball (container media). As defined by ANSI Z60.1, root ball depth measurement is from the top of the ball, which in all cases shall begin at the root flare. Soil above the root flare shall not be included in ball depth measurement and should be removed.

Root ball size shall meet ANSI Z60.1.

Nursery Stock rot balls shall be of sufficient depth to include fibrous and feeding roots. Balls shall be securely wrapped in burlap and tightly bound with rope or twine. Trees may be wire baskets lined with burlap and tightly bound with rope or twine. Balled-in-burlap plants with manufactured balls or container plants that are dry, cracked, or broken before the plant operation will be rejected.

The Engineer reserves the right to inspect root system of trees and shrubs and to reject any material found to be non-compliant.

3. Size and Grading Standards

Plants shall have a standard balance between height, crown spread, diameter and root ball size according to the ANSI Z60.1. All plants shall be typical of their species or variety.

Plants shall conform to the measurements specified except that plants larger than those specified may be used, if approved by the Engineer. If larger plants are approved, the root ball, root spread, or container shall be increased in proportion of the size of the plant.

Caliper measurements shall be taken six inches (6") above the trunk flare for trees up to four inches (4") in caliper, and 12 inches (12") above the trunk flare for trees over four inches (4") in caliper. Plants shall be measured when branches are in their normal position. If a range of size is given, no plant shall be less than the minimum size, and no less than fifty percent (50%) of the plants shall be as large as the maximum size specified. Plants that meet measurements but do not possess a normal balance between height and spread shall be rejected.
Plants shall not be altered by pruning or other means to meet specifications.

4. Plant Certification

All plants shall comply with State and Federal laws governing the shipping, selling and handling of plant stock and inspection for plant diseases and pest infestations. Plants shall be certified free from disease and infestation and invasive weeds. Any inspection certificates required by law shall accompany each shipment invoiced or order of stock, and on arrival, the certificate shall be filed with the Engineer before acceptance.

5. Delivery, Storage, and Handling

All plants shall be packed, transported, and handled with utmost care to ensure adequate protection against injury or damage to the root ball, and desiccation. Plants must be protected from excessive vibrations. Plants shall not be thrown or bounced off a truck or loader to the ground. Plants shall not be dragged, lifted, or pulled by the trunk or braces in a manner that will damage the branches or loosen the roots in the ball.

Plants material transported in vehicles shall be protected from wind whipping either by use of covered vehicle or secure tarps. Failure to protect plant material during transport to the site will result in rejection of plant material.

6. Inspection

The Engineer shall make periodic inspections prior to and during the installation and maintenance periods of the Work. All plants shall be inspected upon delivery to the job site whereupon the Engineer has the right to reject unacceptable plant material. Should plant materials, installation procedures, or other conditions be observed not in keeping with the Drawings, details, and these Specifications, the Engineer will direct the Contractor to correct by repair, and/or replacement as appropriate. The Engineer shall be the sole judge of the conditions of quality and acceptability and will direct all corrections in writing to the Contractor. All rejected materials shall be immediately removed from the site and replaced with specified materials at no additional cost to the Owner.
B. Mulch

Mulch shall consist of shredded bark mulch, wood chips or rock mulch as specified on the Drawings. Material shall be uniform in size, color, quality and overall appearance. Mulch shall be free of material injurious to plant growth. Sources of mulch should be free of weeds and invasive plant parts or seeds. Sawdust, dirt, garbage, or other debris mixed in the mulch is not acceptable. Contractor shall submit two pounds of proposed mulch for inspection by Engineer.

1. Wood Chips

Wood chips shall consist of wood products having a size of two and one-half inch (2-1/2") minus with a thickness not greater than three-eighths inch (3/8"). Wood chips shall be uniform in overall appearance, color, quality, and size and are subject to approval by the Engineer. Wood chips are to be free of sawdust, dirt, twigs, excessive bark, or any other debris.

2. Shredded Bark Mulch

Shredded bark mulch shall consist of shredded bark and wood. Maximum length of any individual component shall be two inches (2") and a minimum of seventy-five percent (75%) of the mulch shall pass through a one inch (1") screen. Mulch shall be free of germination-inhibiting ingredients. The bark mulch shall have the characteristics of retaining moisture, forming a mat not susceptible to spreading by wind or rain, and providing a good growth medium for plants. Shredded bark much may contain up to fifty percent (50%) shredded wood material. Wood chips are not acceptable. Bark mulch containing shredded wood shall be aged a minimum of one year prior to installation. Bark mulch shall be free of soil, rocks, and weeds.

3. Rock Mulch

Rock mulch shall be three to four inch (3" to 4") washed river rock, uniform in size. All fines shall be screened from the aggregate within a one-quarter inch (1/4") tolerance. Rock mulch shall be composed of round rocks that may be varied in color. The material shall be free of organic and inorganic debris and trash.

C. Anti-desiccants

Anti-desiccants shall consist of "Wilt-Pruf" or approved equal.

D. Topsoil

Refer to Section 75.03, Article 3.2 - Materials.
E. Fertilizer/Lime

Refer to Section 75.05, Article 5.2, SubArticles B - Fertilizer and C - Limestone.

F. Water

Water used for the planting, establishment, and all operations of maintenance shall be potable. The water shall be suitable for irrigation, free from substances harmful to plants. The Contractor shall supply a water source unless waived by written authorization from the Engineer.

G. Wood Stakes and Ties

Deciduous Trees: Contractor shall provide two (2) stakes that are six feet (6’) in length and are two by two inches (2” x 2”) around. Stakes shall be pointed at one end.

Evergreen Trees: Contractor shall provide three (3) stakes that are twenty-four inches (24”) in length and are two by two inches (2” x 2”) around. Stakes shall be pointed at one end.

Ties: Ties shall be of three-quarter inch (3/4”) or wider bands of polypropylene, elasticized or webbed strapping, or horticultural tape. Ties shall have a smooth surface and be flexible to allow some movement of the trunk without damaging the bark.

Article 2.3 Construction

A. Notification

Contractor shall notify Engineer at least 5 (five) working days prior to delivery of plant material to the site. Engineer shall inspect plant material prior to off-loading.

B. Planting Season

Planting shall be done when temperatures are above freezing, the ground is frost free, and the soil is in a workable condition. Unless otherwise specified in writing by the Engineer, planting of trees shall be done between May 1 and October 1.

C. Layout and Coordination

The Contractor shall mark all planting areas with stakes or paint. The Engineer shall approve the layout before planting begins. Contractor shall not stage planting operations on trails or sidewalks.

The Contractor shall obtain a utility locate and ensure that utility marking is complete before any excavation begins. The Contractor shall locate any subsurface improvements such as sprinkling system or conduits. The Contractor shall be
responsible for all damage resulting from neglect or failure to comply with this requirement.

Shrubs and small trees with mature height less than fifteen feet (15’) shall be planted at least ten feet (10’) from overhead power lines. Trees with a mature height greater than fifteen feet (15’) but less than twenty-five (25’) shall be planted at least fifteen feet (15’) from overhead power lines. Trees with a mature height greater than twenty-five feet (25’) shall be planted at least twenty feet (20’) from overhead power lines.

The Contractor shall be responsible for moving trees if planted closer than the specified distance. The Engineer reserves the right to relocate plant materials based on utility locations.

D. Protection of Existing Trees

Contractor shall protect existing trees that are not designated for removal on the Drawings. Contractor shall disturb not more than one-third (1/3) of the root system. Contractor shall temporarily brace the tree trunk in place until the root system is backfilled. Contractor shall thoroughly water exposed root systems and cover them with a waterproof sheet until backfilled. Contractor shall ensure bark, branches, roots, and balls of plants are adequately protected at all times from damage including sun, drying winds and frost.

Contractor should identify a Tree Protection Zone (TPZ) in which activities are restricted around each existing tree that is not designated for removal on the Drawings. The TPZ shall be submitted and approved by the Engineer. The TPZ boundary shall be at least thirteen feet (13’) from the trunk and a minimum depth of four feet (4’). The canopy of the tree shall be protected to the drip line. The Contractor shall not stage or store materials within the TPZ. Contractor shall ensure that all land disturbance, excavation, trenching and grade changes, directional boring, routing of utilities and irrigation systems, sidewalks, driveways, construction access roads, changes to street lighting, fire hydrants and utility boxes takes place outside of the TPZ.

Contractor shall ensure tree branches, trunk, and roots of existing trees are protected during construction. Measures for protection may include effective barrier fencing, branch and/or root pruning, protective mulch, supplementary water, soil aeration and information signage.

Contractor shall replace tree(s) indicated on the plan as “existing” which Contractor damages beyond repair with a four inch (4”) caliper tree (deciduous) or fourteen foot (14’) height (evergreen) of the same species, at no additional cost to the Owner. Contractor shall remove and dispose of damaged trees, and install replacement trees in accordance with this Section. Payment for this Work is incidental to the Contract.
E. Existing Plant Relocation:

Existing trees and shrubs designed for relocation as shown on the Drawings shall be dug according to the applicable standards for the plant type. Plants shall be pruned, dug, balled and burlapped, containerized, or dug bare root, moved and planted in accordance with specified tree planting requirements.

Trees designated for relocation may be stored in a designated plant storage area for heeling-in of materials until final planting areas are prepared. Contractor shall maintain plants in storage areas by bracing plants in vertical position and setting balls in an enclosed berm of topsoil or bark. Contractor shall water as required to maintain adequate root moisture. Contractor shall re-burlap plants balls if required before final transplanting operations.

F. Setting and Planting

Contractor shall set balled and burlapped plants, which are not planted immediately upon delivery, on the ground and protect them with soil, moist shredded bark, mulch or other acceptable material. Contractor shall protect plants, if possible, from direct sun until they are planted. Contractor shall keep the soil in the containers and root balls in a moist condition.

Planting pit will be a minimum radius of three (3) times the root ball diameter. The sides of the hole will be sloped at forty-five degrees (45°) and scarified. The depth of the planting pit will vary but shall match root ball depth.

Plants shall be gently removed from containers before planting. Plants shall not be pulled from the container by the trunk.

Each plant shall be planted straight and plumb per standard planting details. All plants shall be set to ultimate finished grade, or slightly higher, so that they will be left in the same relation to the surrounding grade as they have stood before being moved. Plants planted lower than the surrounding grade will be replanted to specified grade before final inspection/approval is granted.

Plants shall be set on firm soil (undisturbed or compacted) so that plant will be at the same depth one year after planting. Any repositioning of trees shall be done by supporting and moving the root ball, not lifting by the trunk. Plants shall be set with the top of the root flare at or slightly above finished grade. Any soil above the root flare must be carefully removed. The root flare shall be within one inch (1") (above or below) of the soil surface. At least two roots must emerge from the trunk within one to three inches (1" to 3") of the soil surface, measured four inches from the trunk.

Roots that are circling the bottom, sides or surface of the root ball shall be gently separated and directed away from the trunk. Roots of bare root plants shall be spread into a natural position, over a pedestal of firm soil if necessary, free of
bunching, kinking or circling. Soil shall be worked firmly into and around the roots so that there are no air pockets. All broken or damaged roots shall be cut back to the point where they are clean and free of rot. No other root pruning shall be done.

After the plant has been set, all ropes, wire, stakes, burlap, plant labels and wrapping around the trunk or branches shall be removed.

Contractor shall remove wire baskets and burlap from the planting area. If a pulp nursery pot is used, it shall be removed from the planting area.

Staking is not recommended as a routine practice. Exceptions include very windy sites, areas where vandalism is expected, or when planting large bare root trees in light soil. Roots must remain stationary but the trunk shall be able to sway in the wind to develop trunk taper and strength. The Contractor shall remove staking and guying material no later than one (1) year after installation.

G. Backfilling Planting Pits and Planting Beds

Backfill shall be approved topsoil per Section 75.04 - Topsoil. When holes are approximately two-thirds (2/3) full, soil shall be thoroughly watered to eliminate air pockets. Planting pits and beds shall be backfilled carefully as specified to fill all voids and to avoid breaking root ball or bruising roots. If settling of the backfill occurs after watering, add more backfill to bring to finish grade.

H. Trees

A continuous six inch (6") soil saucer shall be formed around each tree planted on lower side of the slope to act as a watering basin, except where noted on the Drawings and/or details. This saucer shall be repaired as necessary to continue functioning throughout the maintenance period.

I. Shrubs

A continuous four inch (4") soil saucer shall be formed around each shrub on the downhill side of a slope to form a watering basin; except where noted on the Drawings and/or details. This saucer shall be repaired as necessary to continue functioning throughout the maintenance period.

J. Ground Cover

Excavate a hole sufficient to receive the root spread and backfill around plants with topsoil and tamp soil to hold plant in place. The ground cover shall be planted in alternate rows, unless otherwise specified.
K. Watering

Thoroughly water each plant immediately following planting. Under no condition shall plants not be watered in the same day as planting. The Contractor shall water per maintenance specifications, Article 2.4, SubArticle B – Watering. The Contractor shall assume full responsibility for plant failure as a direct result of insufficient watering. Upon directive from the Engineer, the Contractor shall remove the affected plants and replace them immediately. Replacement of plants is considered incidental to the Contract and no separate payment shall be made.

L. Mulching

After planting has been approved by the Engineer, mulch materials as specified shall be placed and spread where and to the depth indicated on the Drawings. The boundaries of this mulch shall include planting-saucers around the trees and shrubs unless otherwise indicated. Mulch shall be pulled back four to six inches (4” to 6”) from the tree trunk.

M. Pruning and Repair

The only pruning allowed at planting shall be removal of dead, damaged, or broken branches and roots. Pruning shall conform to the American National Standard for Tree Care Operation, ANSI A300. No pruning paint or other wound dressing shall be used.

N. Staking

The Contractor shall stake only if necessary. Ties made of approved material shall be attached directly to the stakes or attached to the stakes by wire. In no case shall the wire extend around the tree trunk, even if covered by a hose.

Ties shall be attached loosely enough to allow the trunk to sway in the wind without allowing movement to the roots. Ties shall be secured at the lowest point on the trunk at which the root ball remains stationary, approximately one-third up the tree. Stakes shall be driven outside the root ball and eighteen inches (18”) into solid ground. Tops of stakes shall be cut off above ties so they do not damage the branches.

For trees larger than three inches (3”) in caliper, ties shall be attached to three guy wires and ground anchors or stakes as detailed in the Drawings. Anchors or stakes shall be driven at a forty-five degree (45°) angle to the ground and placed at one hundred and twenty degree (120°) intervals around the trunk. Staking and guying shall conform to the Drawings.

The Contractor shall remove all stakes, ties, and guy wires one year after installation. The stakes, ties and wires are the property of the Contractor.
O. Clean-up

The Contractor shall keep the project site and all roads and trails utilized during the project, clean and free of trash, excess equipment, materials, and rubbish, including sweeping and washing of trails and pedestrian facilities. Cleanup will be one of the conditions to be met prior to all phases of planting acceptance.

P. Winter Shut-down

Winter shut-down of all installation work shall occur between October 1 and May 1 of the following spring. During winter shutdown periods or work suspensions, the Contractor shall comply with Division 10 – Standard General Provisions regarding responsibilities and protection of all Work under the Contract.

Q. Plant Replacement

A tree is considered dead when the main leader has died back or when twenty-five (25%) of the crown is dead. Contractor shall provide plant material replacements of the same size and type as specified on the plant schedule. The Contractor shall repair to the satisfaction of the Engineer, or replace dead or damaged plant materials at no additional cost to the Owner within five (5) working days after receiving written notice to do so by the Engineer. If Work is not completed to the Engineer’s approval within the time limitation, the Engineer may exercise the option to have a contractor of the Owner’s choosing to perform the Work. If this option is exercised, the cost of that Work will be the responsibility of the Contractor and the Owner shall withhold payment to the Contractor in sufficient amount to recover those costs.

R. Acceptance of Initial Planting Operations

Upon completion of all initial planting operations (planting and seeding), the Contractor shall, per Division 10, Section 10.05, Article 5.26 – Final Inspection, submit a written request for an inspection of plant material.

Initial planting operation ends when:

1. All plants are installed, mulched and watered as specified;
2. Stakes and guys are in place;
3. All construction material and excess excavated material is removed and clean-up is completed.
4. Planting area is free of weeds or any unspecified plants.

Upon written acceptance of all Work by the Engineer, the one year plant establishment period shall begin.
Article 2.4 Maintenance

A. General

The Contractor shall furnish all labor, materials, supplies and equipment required to establish, maintain, and protect the planted and seeded areas, for a one year plant establishment period from date of acceptance of the initial planting operations. However, maintenance activities shall commence immediately after each item is planted or when areas have been seeded.

The Contractor shall supply a maintenance schedule to the Engineer, thirty (30) days prior to the landscape inspection. The Contractor shall also be responsible for protection of his work during the maintenance period, and shall repair and replace all materials and seeded areas damaged or destroyed within the scope of the Work, regardless of cause.

The Contractor’s staff shall include supervisory personnel experienced in landscape maintenance. The Work Force is to be experienced and familiar with maintaining plant material in subarctic conditions.

Contractor shall replace any tree or shrub damaged by a lawnmower, weed whip or other equipment at no additional cost to the Owner.

B. Watering

A proposed watering schedule shall be submitted to the Engineer thirty (30) days prior to installation of plant materials. The Contractor shall deep water all trees and shrubs, providing water penetration throughout the root zone to the full depth of the planting pits.

The Contractor shall deep water all trees and shrubs twice each week during the maintenance period. Watering shall cease at first hard frost in the fall and shall resume upon ground thaw in the spring.

If at any time during the maintenance period weather conditions (such as extended period with no rain or continuous drying winds) cause the plant root zone to dry out, the Engineer may direct the Contractor to deep water all trees and shrubs. Contractor shall provide supplemental watering immediately and at no additional cost to the Municipality.

Water application shall be applied at a rate that will provide moisture penetration throughout the entire root zone with a minimum of water run-off. Should soil conditions be encountered that are not conducive to water absorption, the Contractor shall take whatever corrective actions that may be required to correct this condition, without additional cost to the Owner.
Turf, seeded, bulb areas, and annual flower beds shall be watered at such frequency as weather conditions require to maintain soil moisture within the root zone. When establishing turf and seeded areas, the soil shall be watered often enough to maintain a moist seedbed to promote healthy seed germination resulting in an even and uniform coverage. If the Contractor does not provide adequate watering as required by the Engineer, the Engineer will hire others to perform this task and deduct costs from final payment to the Contractor.

C. Pruning

Pruning shall only be conducted for repair or as specified by the Engineer. Pruning shall conform to ANSI 300 Standards and shall be done by a certified arborist. Dead, broken, or damaged branches may be pruned at any time. Pruning for form shall begin the year after installation. No tree shall be topped. Any tree damaged by improper pruning shall be replaced by the Contractor.

D. Staking and Guying

Stakes and guys, where used, are to be inspected and adjusted as necessary throughout the maintenance period to prevent girdling of trunks or branches, and to prevent rubbing that causes bark wounds. Damaged or missing tree stakes shall be immediately replaced by the Contractor at no additional cost to the Owner. Contractor shall remove staking after the first year, unless the Engineer requests them to be left on longer. Unless otherwise specified, stakes shall be removed at the end of warranty.

E. Plant Repair and Replacement

The Contractor shall repair/replace damaged plant materials, regardless of cause, immediately upon notification by the Engineer. Repair shall include pruning, guying, staking, etc., as necessary. Should repair of plant materials reduce their acceptance to less than minimum specified conditions, the Contractor shall replace plants with specified plant replacements at no additional cost to the Owner.

F. Fertilization

If the construction or maintenance period extends into a second growing season, representative soil tests from the project site shall be taken by the Contractor and submitted to an approved testing lab no later than May 5 for fertility testing. The results of these tests and recommendations for fertilization and limestone application shall be provided to the Engineer and will be the basis for establishing required application rates. All necessary applications shall be completed prior to June 15 or before the end of the maintenance period, whichever occurs first.

Formulations will vary according to soil tests.
G. Diseases and Pests

The Contractor shall coordinate with the Engineer in the event that disease, invasive plant infestation, or pest problems are observed on plants within a Project area. The Contractor and the Engineer shall formulate an Integrated Pest Management program to control the disease, invasive plants, or pests. The IPM program can use biological, physical, cultural, mechanical, behavioral, and chemical methods to resolve the issue. Chemical pesticides are to be used only when other options are not feasible or effective. If pesticides are used, the least toxic pesticide to accomplish the task shall be used.

The Contractor shall apply all materials in complete compliance with all State, Federal, and local regulations, and shall supply the Engineer written proof of their safety and acceptability by State, Federal and Local jurisdictions.

In the event a "restricted use" pesticide is to be applied, the Contractor shall obtain appropriate permits and certifications (according to current 18 AAC 90 Pesticide Control) from the State of Alaska, Department of Environmental Conservation. Proof of certification shall be transmitted to the Engineer prior to application of the chemicals.

H. Weeding

The Contractor shall maintain all areas in a weed-free condition. Weed removal shall be a routine maintenance activity.

I. Cleanup

The Contractor shall keep the project site clean and free of all trash and excess equipment, materials, rubbish, including plant tags, wire, burlap, ribbon, and all debris found within the Project limits, including all roads and trails utilized during Construction. Cleanup will be one of the conditions to be met prior to acceptance of landscape installation and Final Acceptance.

J. Other Tree and Shrub Maintenance

To protect coniferous trees during the winter from excessive desiccation, apply an anti-desiccant such as "Wilt-Pruf" (or approved equal) prior to the winter shut-down period. Complete coverage of all foliage is required.

K. Inspection

The Engineer shall make periodic maintenance inspections of the work. All deficiencies noted shall be corrected within five (5) calendar days from written notice, at no additional cost to the Owner. All delays beyond the five-day period shall result in an equal number of days added to the one year plant establishment period.
**Article 2.5 Final Acceptance**

A Final Acceptance Inspection of the project will occur after completion of the one year plant establishment period. Conditions governing final acceptance of the planted and seeded areas are that, in the opinion of the Engineer, all plants are in a live, uniform, and sound and healthy and flourishing condition; free of disease, insect infestation and physical damage, and free of weeds, rubbish and construction debris. If the Engineer does not accept the improvements, the Contractor shall correct all deficiencies until acceptance is received from the Engineer. All costs associated with correcting the deficiencies shall be paid by the Contractor without additional cost to the Owner.

Should required corrections not be made within thirty (30) days after the initial Final Maintenance Acceptance Inspection, the Contractor shall be assessed liquidated damages per Division 10, Section 10.05, Article 5.27 – Liquidated Damages, until all Work is complete and accepted by the Engineer.

**Article 2.6 Measurement**

The quantity of plants to be paid for shall be by individual plant count, or by area as specified.

**Article 2.7 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section. Payment for all trees, shrubs, and perennials includes payment for the one year establishment period. Owner will pay the accepted quantity of trees, shrubs and perennials after the Acceptance of Initial Planting Operations, based on seventy percent (70%) of the Contract unit price for the respective pay items.

Owner will make two payments for the remaining thirty percent (30%) of the Contract price. One-half of the remaining thirty percent (30%), fifteen percent (15%), shall be paid halfway through the one-year plant establishment period. The second half shall be paid upon Final Acceptance except as noted below.

If damaged trees, shrubs, and perennials are not repaired or replaced as required within fourteen (14) days of written notice, the Engineer may replace or have replaced the damaged items and deduct the cost of said repair or replacement work from the remaining payments. The cost of said repair or replacement work is based on receipts for replacement plants, shrubs, or trees plus ten percent (10%) administrative markup.
Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Trees (by species and size as indicated on Drawings)</td>
<td>Each</td>
</tr>
<tr>
<td>Shrubs (by species and size as indicated on Drawings)</td>
<td>Each</td>
</tr>
<tr>
<td>Ground Cover (by species and size)</td>
<td>Each</td>
</tr>
<tr>
<td>Perennials (by species and size)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 75.03  TOPSOIL

Article 3.1  General

The Work under this Section consists of providing all operations pertaining to furnishing, transporting, and spreading of topsoil.

Article 3.2  Materials

Topsoil furnished by the Contractor shall consist of a natural friable surface soil without admixtures of undesirable subsoil, refuse, or foreign materials. It shall be shredded and free from roots, hard clay, rocks larger than one inch (1") in any dimension, noxious weeds, tall grass, brush, sticks, stubble, or other litter, and shall have indicated by a healthy growth of crops, grasses, trees, or other vegetation that it is free-draining and non-toxic. Topsoil shall contain not more than ten percent (10%) gravel by dry weight of total sample. For the purposes of this specification gravel is defined per ASTM D422 modified to include only material passing one inch (1") and retained on the No. 4 sieve.

Topsoil shall conform to the following requirements, as tested using the procedures included in ASTM D422, ASTM D2974 and AASHTO T267. The topsoil shall be inspected and tested by the Engineer before approval will be granted for its use.

<table>
<thead>
<tr>
<th>Organic Material *</th>
<th>Topsoil Mix</th>
<th>Athletic / Sports Field</th>
<th>Sports Field / Infield Loam</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>15-25% by dry weight of total sample</td>
<td>Not less than 15% nor more than 25% by volume</td>
<td>0.0%</td>
</tr>
<tr>
<td>Silt</td>
<td>25% to 45% by dry weight:</td>
<td>Not less than 20% nor more than 35% by volume</td>
<td>80% by volume</td>
</tr>
<tr>
<td>Sand</td>
<td>35% to 55% by dry weight:</td>
<td>Not less than 50% nor more than 55% by volume</td>
<td>20% by volume</td>
</tr>
</tbody>
</table>

Required depth of the field is six inches (6") after settling.

* Organic matter is to be determined by loss-on-ignition of oven-dried material in accordance with ASTM D2974.)

Limestone and Fertilizer:  Fertilizer shall be of standard commercial types supplied separately or in mixtures, and furnished in moisture-proof containers. Each container shall be marked with the weight and the manufacturer's guaranteed analysis of the contents showing the percentage for each ingredient contained therein.
The proportion of chemical ingredients furnished shall be a mixture that provides the total available nitrogen, phosphoric, and potassium as required by the soil analysis or as specified in the Special Provisions.

Tolerances of the chemical ingredient shall be plus or minus two percent (±2%).

No cyanamid compounds or hydrated lime will be permitted in mixed fertilizers.

Limestone shall contain not less than eighty-five percent (85%) of calcium and magnesium carbonates. Agricultural ground limestone suitable for application by a fertilizer spreader shall conform to the following gradation:

<table>
<thead>
<tr>
<th>Sieve Designation</th>
<th>Minimum Percent Passing, by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td># 10</td>
<td>100</td>
</tr>
<tr>
<td># 20</td>
<td>90</td>
</tr>
<tr>
<td># 100</td>
<td>50</td>
</tr>
</tbody>
</table>

Pelletized limestone may be used subject to approval by the Engineer.

Sufficient fertilizer and limestone shall be applied to the topsoil such that the total natural and applied chemical constituents are within the following ranges:

- Nitrogen: 30-50 PPM
- Phosphoric Acid: 60-110 PPM
- Potassium: 76-150 PPM
- Limestone: Sufficient to attain a pH of 6.0 to 7.0

The Contractor shall furnish soil analysis test reports. Fertilizer and limestone shall be applied at the rates indicated by the soil tests and worked into the topsoil to a uniform depth of two inches (2”).

Organic material for incorporation into topsoil, shall be partially decomposed peat moss. Organic material shall be from a source above the water table. Peat moss may require chopping or shredding to insure thorough mixing with the topsoil.

Article 3.3 Placing

The topsoil shall be evenly spread on the designated areas to a depth of four inches (4”) after settlement unless otherwise specified on the Drawings. Contractor shall not spread the topsoil when the ground or topsoil is frozen, excessively wet, or otherwise in a condition
detrimental to the Work. Roadway surfaces shall be kept clean during hauling and spreading operation.

**Article 3.4 Measurement**

Measurement shall be the number of 1,000 square foot units measured to the nearest 0.1 unit on the ground surface. Stockpiling and rehandling of topsoil during stripping operations, or during placement, shall not be measured for payment.

**Article 3.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Unit cost payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Topsoil (Depth)</td>
<td>1,000 Square Feet (MSF)</td>
</tr>
</tbody>
</table>
SECTION 75.04 SEEDING

Article 4.1 General

The Work under this Section shall consist of providing all labor, equipment, and materials for the preparation of ground surfaces for the application and maintenance of seeded areas, fertilization, lime application (if necessary), watering, and mulching at locations shown on the Drawings or established by the Engineer.

All seeding shall be performed between May 1 and September 1. Seeding any other time will only be allowed upon written approval from the Engineer. Seeding shall not be done during windy conditions or when climatic or ground conditions would hinder placement or proper germination of seed mixes.

Article 4.2 Materials

A. Seed

Seed shall conform to one of the following seed mix types and application rates:

<table>
<thead>
<tr>
<th>Schedule A: Mowable Seed Mix</th>
</tr>
</thead>
<tbody>
<tr>
<td>Application Rate: 5 lbs./1,000 s.f.</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Name</th>
<th>Proportion</th>
<th>Purity</th>
<th>Germination</th>
</tr>
</thead>
<tbody>
<tr>
<td>Annual Ryegrass (Lolium multiflorum)</td>
<td>5%</td>
<td>90%</td>
<td>85%</td>
</tr>
<tr>
<td>Kentucky Bluegrass Alpine (Poa alpine)</td>
<td>30%</td>
<td>90%</td>
<td>85%</td>
</tr>
<tr>
<td>Alene (Poa pratensis)</td>
<td>25%</td>
<td>90%</td>
<td>85%</td>
</tr>
<tr>
<td>Boreal Fescue (Festuca rubra ‘Boreal’)</td>
<td>40%</td>
<td>90%</td>
<td>85%</td>
</tr>
</tbody>
</table>
### Schedule B: Wildflower Seed Mix

**Application Rate:** 3 lbs./1,000 s.f.

<table>
<thead>
<tr>
<th>Name</th>
<th>Proportion By Weight</th>
<th>Purity</th>
<th>Germination</th>
</tr>
</thead>
<tbody>
<tr>
<td>Red Fescue (Arctared) (<em>Festuca rubra</em> ‘Arctared’)</td>
<td>25%</td>
<td>80%</td>
<td>98%</td>
</tr>
<tr>
<td>Egan American Sloughgrass (<em>Beckmannia syzigachne</em> ‘Egan’)</td>
<td>20%</td>
<td>85%</td>
<td>90%</td>
</tr>
<tr>
<td>Wideleaf Polargrass (<em>Arctagrostis latifolia</em>)</td>
<td>20%</td>
<td>85%</td>
<td>90%</td>
</tr>
<tr>
<td>Western Columbine (<em>Aquilegia formosa</em>)</td>
<td>5%</td>
<td>85%</td>
<td>85%</td>
</tr>
<tr>
<td>Siberian Aster (<em>Aster sibiricus</em>)</td>
<td>5%</td>
<td>80%</td>
<td>85%</td>
</tr>
<tr>
<td>Tall Larkspur (<em>Delphinium exaltatum</em>)</td>
<td>5%</td>
<td>70%</td>
<td>85%</td>
</tr>
<tr>
<td>Nootka Lupine (<em>Lupinus nootkatensis</em>)</td>
<td>15%</td>
<td>85%</td>
<td>95%</td>
</tr>
<tr>
<td>Jacob’s Ladder (<em>Polemonium pulcherrimum</em>)</td>
<td>5%</td>
<td>85%</td>
<td>85%</td>
</tr>
</tbody>
</table>

### Schedule C: Wetland Seed Mix

**Application Rate:** 3 lbs./1,000 s.f.

<table>
<thead>
<tr>
<th>Name</th>
<th>Proportion By Weight</th>
<th>Purity</th>
<th>Germination</th>
</tr>
</thead>
<tbody>
<tr>
<td>Red Fescue (Arctared) (<em>Festuca rubra</em> ‘Arctared’)</td>
<td>30%</td>
<td>90%</td>
<td>85%</td>
</tr>
<tr>
<td>Bering Hairgrass (<em>Deschampsia beringensis</em>)</td>
<td>30%</td>
<td>90%</td>
<td>85%</td>
</tr>
<tr>
<td>Polargrass (<em>Arctagrostis latifolia</em>)</td>
<td>10%</td>
<td>90%</td>
<td>85%</td>
</tr>
<tr>
<td>Egan American Sloughgrass (<em>Beckmannia syzigachne</em> ‘Egan’)</td>
<td>30%</td>
<td>90%</td>
<td>85%</td>
</tr>
</tbody>
</table>
Schedule D: Unmowable Seed Mix
Application Rate: 5 lbs./1,000 s.f.

<table>
<thead>
<tr>
<th>Name</th>
<th>Proportion By Weight</th>
<th>Purity</th>
<th>Germination</th>
</tr>
</thead>
</table>
| Red Fescue (Arctared) (
*Festuca rubra* ‘Arctared’)         | 30%                  | 90%    | 85%         |
| Kentucky Bluegrass:                 |                      |        |             |
| Alpine (Poa alpine)                | 30%                  | 90%    | 85%         |
| Alene (Poa pratensis)              | 20%                  | 90%    | 85%         |
| Hard Fescue (Durar)                | 20%                  | 90%    | 85%         |

Schedule E: Athletic Field Seed Mix
Application Rate: 4 lbs./1,000 s.f.

<table>
<thead>
<tr>
<th>Name</th>
<th>Proportion By Weight</th>
<th>Purity</th>
<th>Germination</th>
</tr>
</thead>
<tbody>
<tr>
<td>Fescue (Arctared)</td>
<td>60%</td>
<td>90%</td>
<td>85%</td>
</tr>
<tr>
<td>Kentucky Bluegrass (Alpine)</td>
<td>25%</td>
<td>30%</td>
<td>90%</td>
</tr>
<tr>
<td>Rye Grass (Pinstripe Perennial)</td>
<td>15%</td>
<td>90%</td>
<td>85%</td>
</tr>
</tbody>
</table>

B. Fertilizer

Fertilizer shall be of standard commercial types supplied separately or in mixtures, and furnished in moisture-proof containers. Each container shall be marked with the weight and the manufacturer’s guaranteed analysis of the contents showing the percentage for each ingredient contained therein. The proportion of chemical ingredients furnished shall be a mixture such as to provide the total available nitrogen, phosphoric, and potassium as required by the soil analysis or as specified in the Special Provisions.

Tolerances of the chemical ingredients shall be plus or minus two percent (±2%).

No cyanamid compounds or hydrated lime will be permitted in mixed fertilizers.
C. Limestone

Limestone shall contain not less than eighty-five percent (85%) of calcium and magnesium carbonates. Agricultural ground limestone suitable for application by a fertilizer spreader shall conform to the following gradation:

<table>
<thead>
<tr>
<th>Sieve Designation</th>
<th>Minimum Percent Passing, by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td># 10</td>
<td>100</td>
</tr>
<tr>
<td># 20</td>
<td>90</td>
</tr>
<tr>
<td># 100</td>
<td>50</td>
</tr>
</tbody>
</table>

Fertilizer and limestone for use in a hydraulic sprayer shall be soluble or ground to a fineness that will permit complete suspension of insoluble particles in water.

D. Mulch

Shall be dried shredded peat moss; or cellulose wood or paper fiber such as "Astromulch," "Silvafibre," "Conwed," or approved equal.

E. Water

Water used in all operations shall be of potable quality.

Article 4.3 Application

A. Soil Preparation

After grading of areas has been completed in conformity with the lines and grades shown on the Drawings, and before beginning seeding operations, the areas to be seeded shall be cultivated to provide a reasonably firm but friable seedbed. Cultivation shall be carried to a depth of two inches (2"). On slopes steeper than 3:1, depth of cultivation may be reduced as directed by the Engineer. All cultivated areas shall be raked or cleared of stones (one inch [1"] in diameter and larger), weeds, plant growth, sticks, stumps, and other debris or irregularities which might interfere with the seeding operation, germination of seed, or subsequent maintenance of the seed-covered areas. Contractor may be required to track-walk slopes 2:1 or over as directed in the Drawings or by the Engineer.

B. Fertilizer

Fertilizer shall be applied at a rate to provide two (2) pounds actual Nitrogen per thousand (1,000) square feet of area. In the absence of soil tests and direction from the Engineer, the Contractor shall apply 16-16-16 at the rate of twelve and
one-half (12.5) pounds per thousand (1,000) square feet. Fertilizer shall be in accordance with Section 75.04 - Topsoil.

C. Limestone

Limestone, whether in liquid or dry form, shall be applied at a sufficient rate to attain a soil pH between 6.0 and 7.0.

D. Application Methods

Apply seed mixtures as specified under Article 5.2, SubArticle A - Seed at rates as specified and/or as directed by the Engineer. Seed, fertilizer, limestone, mulch, and water may be applied by the following methods:

1. Hydraulic Method

Seeding by hydraulic methods shall consist of furnishing and placing a slurry made of seed, fertilizer, dried peat moss or cellulose wood fiber and water.

The dried peat moss or cellulose wood fiber shall be added to the water slurry in the hydraulic seeder after the proportionate amounts of seed and fertilizer have been added. The slurry mixture shall then be combined and applied in such a manner that the rate of application will result in an even distribution of all materials.

Hydraulic seeding equipment shall be capable of maintaining a continuous agitation so that a homogeneous mixture can be applied through a spray nozzle. The pump shall be capable of producing sufficient pressure to maintain a continuous, non-fluctuating spray capable of reaching the extremities of the seeding area with the pump unit located on the roadbed. Sufficient hose shall be provided to reach areas not practical to seed from the nozzle unit situated on the roadbed.

2. Dry Method

Mechanical spreader, seed drills, landscape seeder, cultipacker seeder, fertilizer spreader, or other approved mechanical spreading equipment may be used when seed and fertilizer are to be applied in dry form.

Fertilizer shall be spread separately at the specified rates, and then incorporated in one operation to a minimum depth of two inches (2”). Seeded areas shall be compacted within twenty-four (24) hours from the time the seeding is completed, weather and soil conditions permitting, by cultipacker, roller or other equipment satisfactory to the Engineer. Compacting equipment shall be operated at right angles to the slope. Compaction shall not be performed when the soil is in such condition that it
will be picked up by the equipment, nor shall heavy soils be compacted unless directed by the Engineer.

3. Hand Method

Hand broadcasting by means of portable, hand operated mechanical spreaders or "by hand" may be substituted for the preceding two (2) methods provided that the application rate is twice that of the dry method, and that the application is applied in a minimum of two (2) passes over the areas to be seeded (at ninety degrees [90°] to one another in order to assure uniform and even coverage to all seeded surfaces).

Article 4.4 Maintenance

All maintenance shall be in accordance with Section 75.02, Article 2.4 – Maintenance.

The Contractor shall protect seeded areas from damage from all traffic, whether people, animals, on or off road vehicles, or any other causes which may damage newly seeded and maintained surfaces. Surfaces damaged shall be repaired by regrading, reseeding (including all specified amendments), as directed by the Engineer, at no additional cost to the Owner. The Contractor shall otherwise maintain seeded areas in a satisfactory condition until Final Acceptance of the Work.

On the fortieth (40th) day of the maintenance period, the Contractor shall apply one application of fertilizer (16-16-16) at the rate of seven (7) pounds per thousand (1,000) square feet.

Article 4.5 Measurement

The quantity of seeding to be paid for shall be the number of thousand (1,000) square foot units, measured to the nearest 0.1 unit on the ground surface. The quantity of seeding specified shall include all cultivating, seed, limestone, if required, fertilizer and mulch material of the type specified, complete and accepted.
**Article 4.6  Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment for all seeding includes payment for the forty-five day establishment period. Owner will pay the accepted quantity of seeding after it is acceptably planted, based on seventy percent (70%) of the Contract unit price for the respective pay items.

The remaining thirty percent (30%) shall be paid upon Final Acceptance except as noted below.

If damaged seeded areas or areas with insufficient coverage are not repaired or replaced as required within fourteen (14) days of written notice, the Engineer may replace or have replaced the damaged items and deduct the cost of said repair or replacement work from the remaining payments. The cost of said repair or replacement work is based on receipts for replacement seeding, excavation or earthwork as required, and ten percent (10%) administrative markup.

Payment shall be made on the following basis:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Seeding (Type)</td>
<td>1,000 Square Feet</td>
</tr>
</tbody>
</table>

When more than one type of seeding is specified for any pay item, letter suffixes shall be included in order to differentiate between different types.
SECTION 75.05 SOD

Article 5.1 General

A. The Work under this Section consists of performing all operations pertaining to furnishing, installing, and maintaining sod.

B. Agency Standards: Nomenclature

All plant materials used shall be true to name and size conforming to the Guideline Specifications to Turfgrass Sodding, American Sod Producers Association.

C. Submittals

Sod: Provide sales slip or certificate describing original seed blend.

Article 5.2 Materials

A. Sod shall be:

<table>
<thead>
<tr>
<th>Name</th>
<th>Proportion by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>Kentucky Bluegrass:</td>
<td></td>
</tr>
<tr>
<td>Nugget</td>
<td>50-100%</td>
</tr>
<tr>
<td>Merion</td>
<td>0-50%</td>
</tr>
</tbody>
</table>

B. Sod shall be deep green in color, free of chlorotic conditions or signs of disease, weeds, or infestation. Sod shall be grown in an area of well-drained, loamy soils.

C. Sod shall be #1 Quality/Premium as described in “Guideline Specifications to Turfgrass Sodding.” It shall be grown on cultivated agricultural lands and grown specifically for sod purposes.

Article 5.3 Construction

A. Sod Bed Preparation

After grading of areas has been completed in conformity with the lines and grades shown on the Drawings or as directed by the Engineer, scarify and till to a depth of two inches (2") all areas designated to receive topsoil. All cultivated areas shall be raked or cleared of stones one inch (1") in diameter and larger; all debris or irregularities that might interfere with the placement of sod, or subsequent maintenance shall be removed from the site. The Work shall be approved by the Engineer prior to placement of topsoil.
B. Placement of Topsoil in Sod Areas

Following the grading and cultivation of all areas to receive sod, place a uniform layer of topsoil. Topsoil shall be evenly spread on all designated areas to a finish depth of four inches, plus or minus one-half inch (4" ±1/2") after being lightly rolled with a water-filled roller. Spreading shall not be done when the ground is frozen, excessively wet, or otherwise in a condition detrimental to the Work. The Work shall be accepted by the Engineer prior to continuing other Work.

C. Soil Amendments for Sod Areas

Topsoil shall incorporate fertilizer and lime as necessary to meet topsoil chemical constituents.

D. Sod Placement

1. General
   a. Sod shall be cut no more than twenty-four (24) hours before placement. It shall be stored in a manner that protects sod from moisture loss and from extremes in temperature. Soil shall be kept moist.
   b. Sod shall be cut with sharp blades by mechanized equipment designed for the cutting of sod.

2. Harvest
   a. Within twenty-four (24) hours of harvest, sod shall be mowed to a height of one to one and one-half inches (1" to 1-1/2").
   b. Sod shall be cut so as to leave a full intact root mass. It shall be machine cut with a uniform soil thickness of five-eighths inch, plus or minus one-quarter inch (5/8" ±1/4" ) at the time of cutting. Sod that is dry or without soil firmly attached to roots shall be removed from the project site.
   c. Sod shall be cut in straight lines. Cuts shall be of a width between twelve and eighteen inches (12" and 18"), with all cuts to be the same width, plus or minus one-half inch (±1/2"), regardless of the width chosen. Each roll of sod shall be no more than eight feet (8') in length nor shorter than four feet (4').

3. Placement
   a. The ground surface shall be wet before placement of sod, to the extent that soil is damp to a six inch (6") depth.
b. Existing sod areas adjacent to areas to be installed shall have a clean, straight edge and shall be cut with clean, sharp tools. Existing sod edge shall be cut exactly perpendicular to soil surface.

c. Sod shall be laid perpendicular to the slope of the ground and anchored as appropriate. Where curves are necessary, the sod shall be cut to provide edges in full contact with adjacent sod. There shall be no gaps between adjacent pieces of sod. No sod pieces shall be less than three feet (3') in length, or as approved by the Engineer.

d. The first row of sod shall be laid in an even line commencing on lowest portion of slopes with subsequent rows placed parallel to and tightly against each other. Lateral joints shall be staggered to promote more uniform growth and strength. Care shall be exercised to ensure that the pieces are not stretched or overlapped and that all joints are butted tightly.

4. Finishing

Water the sod immediately after transplanting to prevent drying. As sodding is completed in any one section, the area shall be lightly rolled. The new sod shall then be thoroughly watered to a depth sufficient that the underside of the new sod pad and soil immediately below the pad are thoroughly wet. Contractor is responsible for having adequate water available at the site prior to and during installation.

Article 5.4 Maintenance

Maintenance of sod shall conform to Section 75.02, Article 2.4 - Maintenance.

Article 5.5 Measurement

Sod shall be measured per 1,000 square feet and shall be placed in all areas disturbed by construction, or as directed by the Engineer. The measurement of sod shall include all cultivating, materials, limestone, if required, and fertilizer.

Article 5.6 Basis of Payment

Payment of this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sod</td>
<td>1,000 S.F.</td>
</tr>
</tbody>
</table>
SECTION 75.06 LANDSCAPE EDGING

Article 6.1 General

The Work under this Section consists of all labor, equipment, and materials necessary to place landscape edging. All Work shall be in accordance with these Specifications and shall be placed at the locations shown on the Drawings.

Article 6.2 Materials

Landscape edging shall be “Permaloc Aluminum Edging,” black, aluminum, one-eighth inch (1/8”) thickness by four inch (4”) depth with twelve inch (12”) standard aluminum stakes. Comparable products by other manufacturers will be considered provided complete supporting data from the manufacturer is submitted to the Engineer. Comparable products must be architecturally similar in size, type, and grading of materials, dimensions, finishes, and textures.

Permaloc Corporation
13505 Barry Street
Holland, Michigan 49424

Article 6.3 Construction

Landscape edging shall be installed per manufacturer’s specifications, in all locations shown on Drawings.

Article 6.4 Measurement

Landscape edging shall be measured per linear foot, delivered and accepted in place.

Article 6.5 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Furnish and Install Landscape Edging</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 75.07  LANDSCAPE FABRIC

Article 7.1  General

The Work under this Section consists of all labor, equipment, and materials necessary to place landscape fabric. All Work shall be in accordance with these Specifications and shall be placed at the locations shown on the Drawings.

Article 7.2  Materials

Landscape fabric shall be black polypropylene polyester blend, 28 mills, 3.33 ounces/sq. yard, Mullen burst strength: 250 lbs. Permeability is 28 gals/s.f./min. per fallen head method. Landscape fabric shall be DeWitt Pro 5 Weed Barrier (1-800-325-0950) or approved equal.

Article 7.3  Construction

Landscape fabric shall be installed per manufacturer’s specifications in all plant bed areas, tree wells, and as called out on Drawings. Landscape fabric shall not be visible under bark mulch and all loose ends shall be cut off, tucked under, or otherwise covered with three inches (3") of wood chips by the Contractor. Landscape fabric shall be in direct contact with soil.

Article 7.4  Measurement

Landscape fabric shall be measured per square yard, delivered and accepted in place.

Article 7.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Landscape Fabric</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 75.08   WILLOW STAKING

Article 8.1    General

The Work under this Section consists of providing all operations pertaining to the gathering, planting and establishing willow vegetation using willow stakes.

For additional reference, see the document “Stream Bank Revegetation and Protection – A Guide for Alaska” published by the Alaska Department of Fish & Game.

Article 8.2    Materials

Feltleaf willow cuttings shall be used and no substitutions shall be accepted without the written permission of the Engineer. Contractor shall be responsible for obtaining dormant cuttings and necessary permits for the collection of willow stakes.

The technique for using dormant cuttings relies on cuttings of stems taken from plants in the wild during late winter and requires proper storage and care of the cuttings between the time of collection and the time of planting.

Dormant cuttings are live stakes taken in the late winter or early spring before buds have opened. Live stakes should be ten to eighteen inches (10” to 18”) long and one-half to one and one-half inches (1/2” to 1.5”) in diameter, with visible buds. Place cuttings in plastic and keep frozen or refrigerated until the site is ready to plant. Stakes must not be allowed to dry out, nor should they be kept too moist. If cold storage facilities are not available, cuttings can be stored in a snowbank along the north side of a building or some other location that will remain cold until planting. Cuttings may be taken directly from storage to the site for planting. Plant in early spring, after the ground has thawed, and no later than July 1.

Article 8.3    Construction

A.    Planting

   To plant the cuttings, stick a shovel or three-quarter inch (3/4”) or less rebar into the ground as far vertically as possible, push the shovel forward and place the cutting right side up in the ground behind the shovel so that at least three-fourths (3/4) of the length will be buried. Leave a maximum of two leaf buds exposed above ground. Remove the planting implement and step lightly around the cutting to firm the soil. Create a small depression around the planting to collect water.

   Willow stakes shall be planted as vertically as possible.
B. Spacing

Dormant cuttings shall be planted one to three feet (1’ to 3’) on center unless otherwise shown on the Drawings.

C. Watering

Thoroughly water each plant immediately following planting to remove air pockets and increase contact between the soil and the stake. Under no condition shall plants not be watered in the same day as planting. The Contractor shall maintain moist soil conditions for four to six (4 to 6) weeks to establish plants. Water shall be supplied by the Contractor unless otherwise specified. The Contractor shall assume full responsibility for plant failure as a direct result of insufficient watering.

D. Inspection

The Engineer shall make periodic inspections during the installation and maintenance periods of the Work. Should plant materials, installation procedures, or other conditions be observed not in keeping with the Drawings, details, and these Specifications, the Engineer will direct the Contractor to correct by repair, and/or replacement, as appropriate. The Engineer shall be the sole judge of the conditions of quality and acceptability and will direct all corrections in writing to the Contractor. All rejected materials shall be immediately removed from the site and replaced with specified materials at no additional cost to the Owner.

E. Time of Planting

Spring planting and maintenance shall not begin before May 1, and planting operations may commence as soon as the ground is frost free. Planting and maintenance shall not occur later than July 1, unless otherwise specified or approved in writing by the Engineer.

F. Winter Shut-down

Winter shut-down of all installation work shall occur between October 1 and May 1 of the following spring. During winter shut-down periods or work suspensions, the Contractor shall comply with Division 10 – Standard General Provisions regarding responsibilities and protection of all Work under the Contract.

G. Landscape Inspection

Upon completion of all planting operations, the Contractor shall, per Division 10, Section 10.05, Article 5.26 – Final Inspection, submit a written request for an inspection of planted areas. Upon written acceptance of all Work by the Project Engineer, the sixty (60) day maintenance period shall begin.
Article 8.4  Maintenance

A. General

The Contractor shall furnish all labor, materials, supplies and equipment required to establish, maintain, and protect the planted areas, for a one year plant establishment period from date of acceptance of the initial planting operations. However, maintenance activities shall commence immediately after each item is planted.

The maintenance period shall consist of the time from initial planting through acceptance after the one year plant establishment period.

The Contractor shall supply a maintenance schedule to the Engineer, thirty (30) days prior to the landscape inspection. The Contractor shall also be responsible for protection of his work during the maintenance period, and shall repair and replace all materials and seeded areas damaged or destroyed within the scope of the Work, regardless of cause.

The Contractor’s staff shall include supervisory personnel experienced in landscape maintenance. The Work Force is to be experienced and familiar with maintaining plant materials in subarctic conditions.

Contractor shall replace any willow stake damaged during maintenance at no additional cost to the Owner.

B. Watering

A proposed watering schedule shall be submitted to the Engineer thirty (30) days prior to installation of plant materials. The Contractor shall deep water all planted areas, providing water penetration throughout the root zone.

The Contractor shall maintain soil around willow stakes in a moist condition during the maintenance period. Watering shall cease at first hard frost in the fall and shall resume upon ground thaw in the spring.

If at any time during the maintenance period, weather conditions (such as extended period with no rain or continuous drying winds) cause the plant root zone to dry out, the Engineer may direct the Contractor to deep water all plantings. Any supplemental watering is to be done immediately and at no additional cost to the Owner.

Water application shall be applied at a rate that will provide moisture penetration throughout the entire root zone with a minimum of water run-off. Should soil conditions be encountered not conducive to water absorption, the Contractor shall take whatever corrective actions that may be required to correct this condition, without additional cost to the Owner. If the Contractor does not provide adequate
watering as required by the Engineer, the Engineer will hire others to perform this
task and deduct costs from final payment to the Contractor.

C. Plant Repair and Replacement

The Contractor shall repair/replace damaged plant materials, regardless of cause,
upon notification by the Engineer. Should repair of plant materials reduce their
acceptance to less than minimum specified conditions, the Contractor shall replace
plants with specified plant replacements at no additional cost to the Owner.

D. Inspection

The Engineer shall make periodic maintenance inspections of the Work. All
deficiencies noted shall be corrected within five (5) calendar days from written notice
to do so, at no additional cost to the Owner. All delays beyond the five day period,
shall result in an equal number of days added to the one year plant establishment
period.

Article 8.5 Final Maintenance Acceptance

A Final Acceptance Inspection of the project will occur after completion of the one year
plant establishment period. Conditions governing final acceptance of the planted areas are
that, in the opinion of the Engineer, all willow stake areas are in a live, uniform, and sound
and healthy and flourishing condition; free of disease, insect infestation and physical
damage, and free of weeds, rubbish and construction debris. If the Engineer does not
accept the improvements, the Contractor shall correct all deficiencies until acceptance is
received from the Engineer. All costs associated with correcting the deficiencies shall be
paid by the Contractor without additional cost to the Owner.

Should required corrections not be made within thirty (30) days after the initial Final
Maintenance Acceptance Inspection, the Contractor shall be assessed liquidated damages
per Division 10, Section 10.05, Article 5.27 – Liquidated Damages, until all Work is
complete and accepted by the Engineer.

Article 8.6 Measurement

The quantity of plants to be paid for shall be by square yard as specified.

Article 8.7 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 -
Measurement and Payment, and shall include full payment for all Work described in this
Section.
Payment for all willow plantings includes payment for the one year establishment period. Owner will pay the accepted quantity of willow stakes after they are acceptably planted, based on seventy percent (70%) of the Contract unit price for the respective pay items.

Owner will make two payments for the remaining thirty percent (30%) of the Contract price. One-half of the remaining thirty percent (30%), fifteen percent (15%), shall be paid halfway through the one-year plant establishment period. The second half shall be paid upon Final Acceptance except as noted below.

If damaged willows are not repaired or replaced as required within fourteen (14) days of written notice, the Engineer may replace or have replaced the damaged items and deduct the cost of said repair or replacement work from the remaining payments. The cost of said repair or replacement work is based on receipts for replacement cuttings plus ten percent (10%) administrative markup.

Payment shall be made on the following basis.

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Willow Staking</td>
<td>Square Yard</td>
</tr>
</tbody>
</table>
SECTION 75.09 SITE FURNISHINGS

Article 9.1 General

Work under this Section consists of providing all operations pertaining to the removal, disposal, relocation, pickup, delivery, assembly, and installation of Owner-supplied Transit or Park site furnishings, including the labor, equipment, and materials. Work may include relocation of bus stop signs, removal and/or installation of bus shelters, trash receptacles, bear-proof trash cans, benches, poop stations, playground equipment, picnic tables, and other transit, park, or urban street furnishings.

Article 9.2 Materials

Contractor shall provide anchor bolts and other mounting hardware required for the complete installation of the site furnishings. Cost for these anchor bolts and mounting hardware is incidental to this pay item.

The Contractor is solely responsible for loading, transporting, unpacking, and assembly of site furnishings.

A. Transit Materials

Transit related materials for benches, trash receptacles, and bus shelters shall be furnished by the Owner and shall be the type shown on the Drawings. The Contractor shall contact Anchorage Transit System to pickup materials. The Contractor shall notify Transit Staff within forty-eight (48) hours in the event parts are missing.

B. Park Furnishings

Owner supplied park furnishings shall be the type shown on the Drawings. The Contractor shall contact Anchorage Parks and Recreation to pickup materials.

Article 9.3 Construction

Contractor shall load and transport the owner-supplied site furnishings to the site. Contractor shall assemble and install materials in accordance with the Drawings and the manufacturer’s specifications.

Contractor shall repair paint finishes that are damaged during transportation to the site.

Article 9.4 Method of Measurement

Method of measurement for the removal and disposal of existing bus shelters and trash receptacles is for each unit.
Method of measurement for the installation of the Owner-supplied furnishings is for each unit complete in place.

Method of measurement for the removal and relocation of the bus stop sign is for unit relocated and complete in place.

**Article 9.5 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Install Owner-supplied Furnishing (Type)</td>
<td>Each</td>
</tr>
<tr>
<td>Remove and Relocate Bus Sign</td>
<td>Each</td>
</tr>
<tr>
<td>Remove Furnishing (Type)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 75.10 MODULAR CONCRETE BLOCK WALL

Article 10.1 Description

Work under this Section consists of furnishing all labor, equipment, and materials to complete the construction of a modular concrete block wall and steps. Accomplish all work as shown on the Drawings and as described herein.

Article 10.2 Materials

Wall units shall be eight inch (8") Allan Block Retaining Wall System or an approved equal. Capstones and corner blocks are required.

- Color: Grey
- Size: 8"H x 12"D x 18L
- Style: AB Classic

All materials required for proper installation of the retaining wall system, including specified backfill, are incidental to this Section.

Article 10.3 Manufacturer and Local Representative

<table>
<thead>
<tr>
<th>Manufacturer: Allan Block Corporation</th>
<th>Local Representative: Anchorage Sand and Gravel Co., Inc.</th>
</tr>
</thead>
<tbody>
<tr>
<td>7400 Metro Boulevard, Suite 185</td>
<td>1040 O'Malley Road</td>
</tr>
<tr>
<td>Edina, MN 55439</td>
<td>Anchorage, AK 99515</td>
</tr>
<tr>
<td>800-899-5309</td>
<td>907-267-5163</td>
</tr>
<tr>
<td>952-835-0013 (fax)</td>
<td>907-344-2844 (fax)</td>
</tr>
</tbody>
</table>

Article 10.4 Construction

Contractor shall install wall in accordance with manufacturer’s specifications and as shown on Drawings. Contractor shall install wall to match the geometric layout shown on Drawings, including corners. Any changes in the layout require Engineer’s approval. Contractor shall ensure the tops of wall elevations are continuous.

Contractor shall secure capstone with Type P1 Premium Water Proof Construction adhesive per the manufacturer’s specifications.
Article 10.5 Method of Measurement

Measurement is the area in square feet based on the above-grade vertical face of complete units in place. Blocks, specified backfill or other items shown on the Drawings, are not measured separately for payment and are incidental to this bid item.

Article 10.6 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 – Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

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<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Modular Concrete Block Wall</td>
<td>Square Foot</td>
</tr>
</tbody>
</table>
SECTION 75.11  BOULDERS

Article 11.1  General

Work under this Section includes furnishing the labor, equipment, and materials necessary for the furnishing and placing of Contractor-supplied boulders.

Article 11.2  Materials

Contractor shall furnish boulders similar in appearance, color, type, and approximate size as shown on Drawings or specified in the Contract Documents. No evidence of drilling, scrapes, large flakes, or cracks shall be visible after the boulder is set in place.

Article 11.3  Construction

The Contractor shall coordinate with the Engineer prior to setting boulders to ensure desired face and orientation is achieved. Boulders shall be placed on site as directed by the Engineer. The Contractor shall form a pocket for boulder installation, ensuring that the boulder is even and true to line, buried one-third to one-half (1/3 to 1/2) of the boulder depth, and in accordance with the Standard Detail.

Article 11.4  Method of Measurement

Boulders will be measured as units of the specified size complete in place.

Article 11.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 – Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following unit:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Boulder (size)</td>
<td>Each</td>
</tr>
<tr>
<td>Number</td>
<td>Description</td>
</tr>
<tr>
<td>--------</td>
<td>-----------------------------------</td>
</tr>
<tr>
<td>75-1</td>
<td>Shrub Planting Detail</td>
</tr>
<tr>
<td>75-2</td>
<td>Conifer Planting Detail</td>
</tr>
<tr>
<td>75-3</td>
<td>Deciduous Tree Planting Detail</td>
</tr>
<tr>
<td>75-4</td>
<td>Boulder</td>
</tr>
</tbody>
</table>
BACKFILL WITH TOPSOIL; THOROUGHLY WATER BACKFILL AS TO PREVENT AIR POCKETS. DO NOT TAMPER OR COMPACT. DO NOT FERTILIZE AT TIME OF PLANTING.

SHREDDED BARK MULCH (3" DEPTH); KEEP 3" TO 6" AWAY FROM TRUNK

SCARIFY SIDES AND BOTTOM OF SAUCER-SHAPED PLANTING PIT

D = DIAMETER OF ROOTBALL

NOTES:
1. DEPTH VARIES DEPENDING ON ROOTBALL THICKNESS.
2. SOIL SHALL BE LOOSENED AND SUITABLE FOR ROOT GROWTH. TOP WIDTH OF PLANT PIT SHALL BE 4–5 TIMES ROOTBALL DIAMETER.
SHREDDED BARK MULCH (3" DEPTH); KEEP 3" TO 6" AWAY FROM TRUNK

FLAGGING ON GUY WIRE (TYP.)

(3) 2"x2"x24" WOOD STAKE (IF REQUIRED)

BACKFILL WITH TOPSOIL; THOROUGHLY WATER BACKFILL AS TO PREVENT AIR POCKETS. DO NOT TAMPER OR COMPACT. DO NOT FERTILIZE AT TIME OF PLANTING.

SCARIFY SIDES AND BOTTOM OF PLANTER PIT

NOTES:
1. CUT BURLAP AND WIRE BASKET, PEEL DOWN, REMOVE OR LAY FLAT IN BOTTOM OF PLANT PIT. REMOVE CONTAINERS.

2. DEPTH OF PLANT PIT VARIES DEPENDING ON ROOTBALL THICKNESS. SOIL SHALL BE LOOSENED AND SUITABLE FOR ROOT GROWTH. TOP WIDTH OF PLANT PIT SHALL BE 4–5 TIMES ROOTBALL DIAMETER. SET ROOTBALL ON SOLID GROUND TO PREVENT SETTLING. PLANT TREE TRUNK WITH TRUNK FLARE AT OR UP TO 1" ABOVE FINISHED GROUND.

3. USE THREE 2"x2"x24" WOOD STAKES WHEN SPECIFIED ON DRAWINGS OR REQUIRED BY THE ENGINEER. EMBED AT ANGLE. DO NOT PENETRATE ROOTBALL. USE SOFT, FLEXIBLE MATERIAL FOR TIES. GUY AT APPROXIMATELY 1/3 TREE HEIGHT. REMOVE GUY AFTER ONE YEAR. DO NOT STAKE TREE RIGID, IT MUST MOVE IN THE WIND.
NOTES:

1. **CUT BURLAP AND WIRE BASKET, PEEL DOWN, REMOVE OR LAY FLAT IN BOTTOM OF PLANT PIT. REMOVE CONTAINERS.**

2. **DEPTH OF PLANT PIT VARIES DEPENDING ON ROOTBALL THICKNESS. SOIL SHALL BE LOOSENED AND SUITABLE FOR ROOT GROWTH. TOP WIDTH OF PLANT PIT SHALL BE 4-5 TIMES ROOTBALL DIAMETER. SET ROOTBALL ON SOLID GROUND TO PREVENT SETTLING. PLANT TREE TRUNK WITH TRUNK FLARE AT OR UP TO 1" ABOVE FINISHED GROUND.**

3. **USE TWO 2"x2"x6' WOOD STAKES WHEN SPECIFIED ON DRAWINGS OR REQUIRED BY THE ENGINEER. EMBED 24" DO NOT PENETRATE ROOTBALL. USE SOFT, FLEXIBLE MATERIAL FOR TIES. GUY AT APPROXIMATELY 1/3 TREE HEIGHT. REMOVE GUY AFTER ONE YEAR. DO NOT STAKE TREE RIGID, IT MUST MOVE IN THE WIND.**
NOTES:
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FOR TRAFFIC SIGNALS AND ILLUMINATION
DIVISION 80
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SECTION 80.01  GENERAL

Article 1.1  Scope of Work

Work shall consist of furnishing and installing, modifying, removing or salvaging one or more traffic signal systems, flashing beacon systems, illumination systems, sign illumination systems, traffic count stations, electrical equipment on structures, falsework lighting, partial installations for future systems, or combinations thereof, all as required by the Drawings, and as specified. All necessary labor and equipment to provide fully functioning traffic signals, intersection lighting, or roadway illumination is included.

Prior to installation of foundations, junction boxes, and conduits; Contractor shall locate and protect all new and existing underground utilities; including, but not limited to, pipelines, signal systems, thaw wires, lighting systems, storm drain, sanitary sewers, water systems, and telephone, cable television, and electrical cables. Not all of the existing utilities may be present or shown on the Drawings. Contractor shall adjust foundation, junction box, or conduit location if conflict exists with either existing utilities or proposed improvements. No additional monies is paid or owed to Contractor for the adjustment.

Materials furnished shall be new, except such used materials as may be specifically provided for on the Drawings or in the Special Provisions. Where an existing system is to be modified, the existing material shall be reused on the project, or disposed of as shown in the Drawings, or specified in the Special Provisions.

All systems shall be complete and in operation with all materials in conformance with Drawings, Specifications and the manufacturer's specifications and recommendations, at the time of final acceptance.

Article 1.2  Regulations and Codes

All material, and workmanship where applicable, shall conform to the standards of the Underwriters Laboratories, Inc., the National Electrical Code, and the National Electrical Safety Code together with local amendments. Within this Division, the term "Code" shall mean the National Electrical Code, and the National Electrical Safety Code together with local amendments.

Where applicable, all electrical equipment shall conform to the standards of the National Electrical Manufacturers Association.

Highway and Transportation Officials (AASHTO) shall be referred to in this Division as the 1994 AASHTO design criteria.

**Article 1.3 Equipment List(s) and Drawings**

A. The Contractor shall submit for review and approval, within thirty (30) days following award of the Contract, eight (8) collated copies of a portfolio of equipment and materials which he proposes to install. The portfolio(s) shall consist of a table of contents which includes each item's intended use(s) and the following:

1. For materials on the Approved Products List: a description that includes product name, manufacturer, model or part number, and the conditions listed for approval.

2. For materials not on the Approved Products List: catalog cuts that include the manufacturer's name, type of product, size, model number, conformance specifications, and supplemented by other data as may be required, including manufacturer's maintenance and operations manuals, or sample articles.

3. A wind stress certificate from the manufacturer of poles, signal mast arms, and luminaire arms. Contractor shall submit to the Engineer for approval the Wind Stress Certificate that includes the signed stamp of a professional engineer registered in the State of Alaska; and a statement that indicates that the poles and mast arms meet the wind and mast arm loading requirements specified in Section 80.05, Article 5.1 - General.

4. Contractor shall submit to the Engineer for approval the Materials Certifications for all lighting poles, signal poles, mast arms, connector bolts and anchor bolts, indicating that the steel and galvanizing conform to the requirements in this Division.

The Municipality will not be liable for any material purchased, labor performed, equipment used, or delay to the Work before all equipment and materials have been reviewed and approved.

B. Two (2) paper copies and one electronic copy of traffic signal controller cabinet schematic wiring diagrams shall be submitted at the time the controllers are delivered for testing, or if ordered by the Engineer, prior to purchase. This diagram shall show the location and phasing of the intersection; list of all equipment installed in each cabinet, and show in detail all circuits, parts, and schematic wiring. Contractor shall also provide at this time, two (2) reproducible and one (1) electronic set of Operation and Maintenance manuals and wiring diagrams of any cabinet equipment utilized. These manuals shall show in detail all circuits and parts. Such parts shown thereon shall be identified by name or number and in such a manner as to be readily interpreted.
C. The Contractor shall prepare five (5) complete sets of red lined as-built plans which shall be kept current with the construction. These as-built plans shall detail all construction changes made to the Drawings and also include the following information on each appropriate drawing:

1. Location and depth of conduit runs.
2. Station and offset of all junction boxes.
3. Heights of signal faces and overhead signs.

Copies of such as-built plans shall be furnished at least twice a month during construction so that they may be reviewed for accuracy and completeness. The Contractor shall furnish any additional information required to clarify the as-built plans and shall correct all discrepancies. Progress payment for the signal and illumination Work completed shall not be made until accurate as-built plans reflecting the construction progress have been reviewed and deficiencies corrected.

D. Prior to final inspection of the Work, Contractor shall submit five (5) complete sets of Record Drawings to the Engineer. The Engineer will deliver one (1) copy each to Project Management & Engineering; Traffic Department, Signals Section; Traffic Signal Electronics Shop; Maintenance & Operations, Street Light Maintenance Supervisor; and attach the appropriate sheets of the fifth set in clear envelopes to the inside of each load center.

Article 1.4 Warranties, Guarantees and Instruction Sheets

Manufacturers’ warranties, guaranties, instruction sheets and parts furnished with materials used in the Work shall be delivered to the Engineer.

For equipment brands and models not currently in use within the Municipality of Anchorage, a manufacturer’s representative shall be present to supervise the turn on and adjustment of the signal system. In addition, the representative shall provide one workday of continuous instruction and familiarization in the operation and maintenance of the signal system.

Article 1.5 Maintaining Existing and Temporary Electrical Systems

The Contractor shall maintain the traffic signal and highway lighting systems from the time of the Notice to Proceed until the time of final acceptance except during any authorized stoppages when the Municipality of Anchorage shall assume maintenance. Temporary replacement equipment furnished by the Contractor shall be compatible with existing equipment used in the M.O.A. and approved by the Engineer. Representatives of the Contractor and the Owner shall inspect the project prior to the winter shutdown and prior to spring start-up to ascertain those items that need repair and determine responsibility for the repairs. If the project includes traffic signal Work, Traffic Signal personnel shall be included in the inspection.
The existing Traffic Signal installation may not be shutdown between 7:00 and 8:30 a.m. or 3:00 and 6:00 p.m. weekdays.

The local traffic enforcement agencies and Traffic Department, Signals Section shall be notified prior to any operational shutdown of a traffic signal system.

The Contractor and the Traffic Signal personnel shall do a walk-through inspection of the existing traffic signal system prior to commencing Work.

The Contractor shall provide temporary signalization. At no time shall a signalized intersection operate in an unsignalized mode, except for shutdown due to change over from the existing system to a temporary system, and from the temporary system to a permanent system. Temporary signal system shutdowns shall be limited to periods during normal working hours as specified in this Section, during which flag control shall be used.

The temporary signal system plan shall be submitted to and approved by the Municipal Traffic Engineer or assigned designee prior to implementation. The temporary signal plan shall equal or exceed the system being replaced or modified. That is, the plan shall not downgrade the number of signal heads, signal phases, pedestrian push buttons, etc. The temporary signal system plan shall also include the layout of the temporary intersection. The complete plan shall include intersection geometrics, lane widths, and auxiliary lane pocket lengths.

No vehicle detection will be required in a temporary system, unless called for in the Drawings and Specifications or specified by the Traffic Department. The Contractor shall coordinate all Signal Work with the Traffic Signal Section or the assigned designee at 343-8355.

The Contractor shall be responsible for maintaining any span wire temporary signal installed. The Traffic Department will not assume maintenance responsibility for span wire systems.

The Contractor shall obtain Traffic Department approval prior to turning any maintenance responsibilities over to the Traffic Department, including any maintenance required during Winter Shutdown.

The Contractor shall furnish and install all materials and miscellaneous hardware required to provide a functional traffic signal system. All materials shall conform to the requirements of the Drawings and Specifications. Temporary equipment shall be compatible with existing equipment used in Anchorage.

The temporary signal system may consist of any combination of the following:

1. The existing systems,
2. Relocation of component parts,
3. Guyed wood poles, or
4. Any portion of the permanent signal system.

Traffic signals may be suspended from messenger cables provided that they are mounted by standard span wire hangers and secured with a second cable (tether wire) to prevent misalignment in the wind. Messenger cables shall be at least three-eighths inch (3/8") O.D. “High Tensile” grade cable. Tether wire shall be one-eighth inch (1/8") O.D. steel cable installed with a minimum ground clearance of nineteen feet (19’). All signal faces shall be equipped with backplates and visors. The signal faces of each phase with two or more faces shall be energized using two (2) circuits, with each circuit wired with IMSA 20-1 signal cable. Splices shall be made only at the terminal blocks in the signal faces. Sufficient signal cable slack shall be left at each pole to provide for drip loops and to allow realignment of each signal head.

Whenever a pole of the permanent signal system is included in a span wire signal system, the Contractor shall guy the pole and provide protective collars to prevent chafe damage. Poles with breakaway bases shall not be included in a span wire supported signal system.

The Contractor shall provide illumination at all locations with preexisting lighting and at all intersections where temporary traffic signalization is specified to be provided.

The temporary facilities shall be provided during the life of the Contract on all roadways open to traffic within project limits. The temporary lighting systems shall be operational by sunset on the same day the replaced system is retired, or the roadway is opened to traffic.

A plan for each temporary lighting system shall be submitted to and approved by the Engineer prior to implementation. The temporary lighting plan shall equal or exceed the system(s) being replaced or modified. At intersections, the temporary system shall include a luminaire located on the far right for each through street approach and installed adjacent to the through street radius returns. The through street is the street with the vehicular right of way; both streets shall be considered through at signalized intersections and four-way stops.

The Contractor shall furnish and install all materials and miscellaneous hardware required to provide a functional lighting system including electrical load centers. All materials shall conform to the requirements of the Drawings and Specifications, except that the branch conductors may be triplex aluminum with messenger cable if they are installed overhead. Illumination conductors shall be sized so that the voltage at the most remote luminaire is not less than the minimum required for the ballast as recommended by the manufacturer. The Contractor shall install intermediate conductor and supports to energize luminaires at locations without electrical service.

Luminaires used in the system may be the existing fixtures or new fixtures with a light distribution compatible with the proposed lighting configuration.

The temporary lighting systems may consist of any of the following lighting pole types, or combinations thereof, provided the luminaires have a minimum of thirty feet (30’) mounting height. Mounting height is the difference in elevation between the luminaire retractor and
the edge of traveled way at the same station. The existing poles may be reused if they are not utility owned. Any pole of the permanent lighting and temporary signal systems and any Contractor-supplied poles may be wood and shall meet 1994 AASHTO design criteria for one-hundred-mile-per-hour (100 mph) winds with gusts to one hundred thirty miles per hour (130 mph). All poles, except traffic signal poles, installed within the clear zone shall be provided with FHWA approved slip bases, transformer bases, or frangible couplings.

The load centers to power the temporary lighting and signal systems may be the permanent installations, the existing installations, or temporary installations. The existing load centers may be used only if they are scheduled to remain intact until completion of the project, and reused only if they are approved. The Contractor shall provide approved temporary load centers with photoelectrically-controlled lighting circuits whenever a load center is unavailable for use, or when an existing load center that is not approved is retired due to conflict with the Work. An approved load center is any load center UL labeled as Service Equipment, or UL labeled as Industrial Control Equipment and marked "suitable for use as service equipment." The Contractor shall provide all Work to modify these load centers as required to provide functional temporary lighting and signal systems, and to install them completing all Work in accordance with the NEC.

Once the Contractor commences Work on the project, he shall provide all maintenance for the existing electrical facilities. The Municipality will pay for the electrical power for the above-mentioned electrical systems. The above maintenance does not include any prior damage such as burned out lamps, non-operative detection or other malfunctioning equipment. The Contractor shall present written documentation of all non-functioning and malfunctioning electrical equipment before commencing Work on the project. This malfunctioning equipment shall be inspected jointly by personnel from the Engineer’s staff and the Contractor. In the event the Engineer does not receive notice in writing and the Contractor begins Work on the project, this will suffice as evidence that all equipment is functional and operational.

The Contractor shall furnish the Engineer with the name and phone number of the person responsible for maintaining existing and temporary electrical facilities. Repair work shall commence within one hour of notification for traffic signal systems.

The exact location of existing conduit runs, direct burial cable, pull boxes, and all underground utilities shall be ascertained by the Contractor before using equipment that may damage such facilities or interfere with any system.

Where roadways are to remain open to traffic and existing lighting systems are to be modified, the lighting systems shall remain in operation and the final connection to the modified circuit shall be made so that the modified circuit will be in operation by nightfall of the same day the final connection is made.

Temporary electrical installations shall be kept in effective operation until no longer required. Removal of temporary installations shall conform to the provisions in Section 80.28 – Salvaging Electrical Equipment.
These provisions will not relieve the Contractor in any manner of his responsibilities as provided in Division 10, Section 10.06 - Legal Relations and Responsibilities.

**Article 1.6  Scheduling of Work**

Work shall be so scheduled that each new traffic signal system, highway lighting system, and sign illumination system shall be completed and ready for operation prior to opening to traffic of the corresponding section of new alignment.

Traffic signal systems shall not be placed in operation without energizing the street lighting at the intersection to be controlled if street lighting exists or is being installed with the traffic signals.

Contractor shall not place traffic signal systems into operation. Traffic Signal personnel are the only persons authorized to turn on a traffic signal.

Conductors shall not be pulled into conduit until pull boxes are set to grade, crushed rock sumps installed, grout placed around the conduit, and metallic conduit bonded.

In vehicular undercrossings, soffit lights shall be placed in operation as soon as practicable after falsework has been removed from the structure. Lighting for pedestrian structures shall be placed in operation prior to opening the structure to pedestrian traffic.

If the Engineer orders soffit lights or lighting for pedestrian structures placed in operation before permanent power service is available, the cost of installing and removing temporary power service will be paid for as extra Work as provided in Division 10, Sections 10.05 – Control of Work and 10.07 – Measurement and Payment.

**Article 1.7  Safety Precautions**

Before starting Work on existing series street lighting circuits, the Contractor shall obtain daily, a safety circuit clearance from the serving utility. By-pass switch plugs must be pulled and suitable signs posted at switch boxes before electrical Work begins.

Suitable signs shall be posted at Load Centers when a contractor is working on any of the circuits from that Load Center.

**Article 1.8  Definitions**

The Definitions in NEMA TS-1, Traffic Control Systems, Part I, shall be used along with the following:

1. **Electrolier:** The complete assembly of pole, mast arm, luminaire, ballast, and lamp.

2. **Luminaire:** The assembly which houses the light source and controls the light emitted from the light source. Luminaires consist of hood (including socket), reflector, and glass globe or refractor.
3. Lighting Standard: The pole and mast arm which must support the luminaire.


5. Controller Unit: The solid-state device as described in Section 80.17, Article 17.2 – Controller Unit.

6. Controller Cabinet: A cabinet constructed, wired and equipped as described in Section 80.17, Article 17.5 - Controller Cabinet.

7. Controller Assembly. The controller cabinet, controller unit and the equipment described in Section 80.17. The controller assembly shall also be functioning in accordance with Section 80.17, Articles 17.1- General and 17.6 - Operation.

8. Anchor bolts apply to Luminaire poles and anchor rods apply to Signal poles. They are used interchangeably in this Division.

**Article 1.9 Signs**

Reference Division 80, Section 70.11 – Standard Signs.

**Article 1.10 Measurement**

All Work in this Section shall be measured by lump sum and shall consist of all labor, materials, and equipment necessary to provide temporary signalization and temporary illumination.

**Article 1.11 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Temporary Signalization</td>
<td>Lump Sum</td>
</tr>
<tr>
<td>Temporary Illumination</td>
<td>Lump Sum</td>
</tr>
</tbody>
</table>
SECTION 80.02  EXCAVATING AND BACKFILLING

Article 2.1  General

The excavations required for the installation of conductors, conduits, foundations and other appurtenances shall be performed in such a manner as to avoid any unnecessary damage to the streets, sidewalks, landscaping, and other improvements. The trenches shall not be excavated wider than necessary for the proper installation of the electrical appurtenances and foundations. Excavation shall not be performed until immediately before installation of conduit and other appurtenances. The material from the excavation shall be placed in a position that will not cause damage or obstruction to vehicular and pedestrian traffic nor interfere with surface drainage.

Trench, backfill, and disposal of surplus material shall be performed in accordance with Division 20 – Earthwork.

Excavations after backfilling shall be kept well-filled and maintained in a smooth and well-drained condition until permanent repairs are made.

All excavations shall be filled, and sidewalks, pavement, and landscaping restored at each intersection prior to excavating at any other intersection. Excavations in the street or highway shall be performed in such a manner that not more than one traffic lane is restricted in either direction at any time, unless otherwise provided in the Special Provisions.

Article 2.2  Construction

The Contractor shall excavate the trench to the proper depth as described herein and as shown on the Drawings.

The excavations shall be backfilled with material suitable to the Engineer. All backfill placed in the roadway area shall be Type II-A classified backfill as specified in Division 20., Section 20.21, Article 21.2 - Material. All backfill material shall be placed in uniform layers of not more than six inches (6”) in depth and compacted to a density of not less than ninety-five percent (95%) of the maximum density as directed by the Engineer.

The Contractor shall be responsible for the restoration of all surfacing, turf, and native material to the original condition and appearance.

Article 2.3  Sawcut Trench

Where shown on the Drawings, or as directed by the Engineer, the Contractor shall construct a sawcut trench as detailed in the Drawings. A sawcut trench will be used to cross existing traveled lanes, existing curb and gutter, in median islands, along edges of paved roadways, and in sidewalk areas where a neat cut of the surfacing is required.
The Contractor shall cut the surfacing material full-depth and remove the surfacing material to expose the subgrade materials. The Contractor shall then excavate a trench, dispose of excess and waste materials, and install conduit as described herein.

In sawcuts of asphalt pavement located within the roadway pavement, Contractor shall remove a minimum distance of one foot (1’) back from the edge of the trench, on each side of the trench. Contractor shall remove pavement such that cuts parallel to the direction of travel are not located within the wheel paths.

The entire trench shall be backfilled as specified herein, except non-frost-susceptible sand bedding material shall be used.

The existing surface shall then be restored with like pavement in accordance with Section 40.07 - Remove and Replace Existing Asphalt Surfacing; Section 40.06 - Tack Coat; Section 30.03 - Portland Cement Concrete Sidewalks; or Section 30.02 Portland Cement Concrete Curb and Gutter, and Valley Gutter, as applicable.

Where applicable, asphalt tack coat shall be applied to all edges of the existing pavement prior to placing new asphalt. Asphalt pavement less than three inches (3") in thickness shall be placed in one lift, and asphalt pavement three inches (3") and greater in thickness shall be placed in a minimum of two equal lifts.

In median islands, the Contractor may elect to remove and replace the entire surface of the island along the length of the conduit run. If the Contractor elects to remove the entire surface of the island, the Work shall still be considered as sawcut trenching. The layer of pavement under the median islands, if encountered (normally at the street pavement grade), may be broken out.

The Contractor shall be responsible for the restoration of all surfacing, turf, and native material to original condition and appearance.

**Article 2.4 Measurement**

Measurement for trench and backfill and for sawcut trench shall be per linear foot of horizontal distance of the various widths and depths as set forth in the Bid Schedule. Measurement will be from station to station or from center of device to center of device as staked in the field and as shown on the Drawings.
Article 2.5  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Work not specifically identified for payment under a separate pay item, but required for normal completion of trench and backfill, will be considered incidental and shall be included in the linear foot cost of the trench. Sawcut trench includes removing existing pavement, trench and backfill, and replacing pavement.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Trench and Backfill (Width) (Depth)</td>
<td>Linear Foot</td>
</tr>
<tr>
<td>Sawcut Trench (Width) (Depth)</td>
<td>Linear Foot</td>
</tr>
</tbody>
</table>
SECTION 80.03  REMOVING AND REPLACING IMPROVEMENTS

Article 3.1   General

Improvements such as sidewalks, curbs, gutters, Portland cement concrete and asphalt concrete pavement, base material, lawns and plants and other improvements removed, broken or damaged by the Contractor's operations, shall be replaced or reconstructed with the same kind of material as found on the Work or with materials of equal or better quality. The new Work shall be left in a satisfactory serviceable condition.

Whenever a part of a square or slab of existing sidewalk, curb and gutter, or driveway is broken or damaged, the entire square, section or slab shall be removed and the concrete reconstructed as above specified.

The outline of all areas to be removed in concrete sidewalks and driveways and in pavements shall be cut to a minimum depth of one and one-half inches (1 1/2") with an abrasive type saw prior to removing the sidewalk, driveways, and pavement material. The cut for the remainder of the required depth may be made by a method satisfactory to the Engineer. Cuts shall be neat and true with no shatter outside the removal area.

When a foundation is to be abandoned in place, the top of foundation, anchor bolts, and conduit shall be removed to a depth of not less than one foot (1') below surface of sidewalk or unimproved ground. The resulting hole shall be backfilled with material equivalent to and compacted to the density of the surrounding material.

Article 3.2   Measurement

All Work under this section is incidental to other Work and will not be measured or paid for directly.

Article 3.3   Basis of Payment

No separate payment will be made for this item.
SECTION 80.04 FOUNDATIONS

Article 4.1 General

All foundations for poles, posts and pedestals shall be cast-in-place Portland Cement Concrete or driven pile.

Unless otherwise shown on the Drawings, all items to be relocated shall be provided with new foundations and anchor bolts of the proper type and size.

The Contractor shall be responsible for contour grading around all post, pole, and pedestal foundations. Final or finished grading shall be such that the earth shall be two inches (2") below the top of the base and drain away from the base.

Foundations for poles shall be designed for one-hundred-mile-per-hour (100-mph) winds with gusts to one hundred thirty miles per hour (130 mph) in conformance with the requirements of the 1994 AASHTO design criteria.

Cabinet foundations shall be precast.

The entire controller foundation and the top twelve inches (12") of pole or post foundations shall be formed and the top given a smooth steel trowel finish. Conduits shall be located in the center of the pole-post foundations with clearance allowed for bushings.

The tops of all pole foundations shall be set so that the bottom center of the base plates are between four (4") and six inches (6") above finished grade at the pole's offset. The top of any foundation located on a slope shall be constructed such that the finished slope passes through the top center of the uphill edge of the foundation. The area two feet (2') up and down slope of the edge of the foundation shall be graded so that no portion of the foundation projects above the surrounding slope and so that water will drain away from the foundation.

The Contractor shall field-verify pole foundation stationing and elevations prior to pouring the foundations, to insure that the final locations of the signal heads and mast arms meet the requirements of the Drawings and Specifications. The field-verification includes checking to insure that the heads will be the proper distance above the roadway surface, and mast arms will be of adequate length to place heads and signs in the right locations. Any discrepancies shall be reported to the Traffic Engineer prior to pouring the foundation.

Article 4.2 Cast-In-Place Concrete Foundations

The Contractor shall use a corrugated steel pipe (CSP) form to cast concrete foundations in place. The Contractor shall over excavate the area around the form enough to allow for proper compaction. The backfill operation shall conform to the requirements of Division 20, Section 20.19 – Furnish Foundation Backfill. The use of water for drilling operations or for any other purpose where it may enter the hole is not permitted.
Concrete shall be Class AA-3 Portland Cement conforming to Division 30 – Portland Cement Concrete.

Reinforcing steel and wire fabric shall conform to the requirements of Division 30, Section 30.01, Article 1.3 - Materials and Section 80.05 – Poles, Steel Pedestals and Posts. Reinforcement shall be placed and fastened in conformance with Division 30, Section 30.05, Article 5.2 - Construction, except that bars to be spliced shall be lapped at least fifty (50) bar diameters. Where bar spacing is less than one (1) foot in each direction, the Contractor may tie alternate intersections.

Drilled holes or forms shall be vertical, and true to the locations shown in the Drawings. Upon completion of excavation for a foundation, and prior to the placement of concrete, all loose material shall be removed in order that the foundation rests on firm, undisturbed ground.

Forms, if indicated or required, shall be true to line and grade, with the top of the foundation at the established elevation.

Conduit shall be included in all concrete foundations for wire and cable entry as shown on the Drawings as required to complete the Work. The conduit in pole or post foundations shall extend four inches (4") above the foundation (but not above the slip base adapter) and shall be sloped towards the hand-hole opening. These conduits shall exit the foundations in the top center of the foundation surface.

The reinforcing steel cage, if required, shall be placed and secured symmetrically about the vertical axis and shall be securely blocked to clear the sides of the foundation. Anchor bolt assemblies and conduit ends and reinforcing bar assemblies shall be securely supported by templates. Each anchor bolt shall have two (2) nuts and two (2) washers.

Anchor bolts, nuts and washers shall conform to ASTM F1554 with grade as specified by manufacturer, and shall be hot-dip galvanized after fabrication in accordance with ASTM A153. Anchor bolts for signal mast arm foundations shall conform to ASTM F1554. The grade of steel shall be as specified by the pole manufacturer, for the loading specified in Section 80.05, Article 5.1 - General. The exposed end of all anchor bolts used for signal mast arm poles shall be clearly stamped with the appropriate markings so that the type of bolts used in the foundation can be clearly determined after construction, per ASTM F1554 supplementary requirements S2 and S3. Signal mast arm foundation anchor bolts shall conform to Charpy Impact Requirements at –20°F, per supplementary requirement S5. Anchor bolts may not be field cut or bent. Damage to galvanized surfaces as a result of damage during shipping or construction activities shall be repaired in accordance with Section 80.16, Article 16.3 - Galvanizing.

Material certifications for all anchor bolts shall be submitted to the Traffic Engineer or designated representative prior to acceptance of the foundations for payment.

Reinforcing bars shall be formed into cages and all intersections tied with #14 AWG steel wire. The cages shall be accurately held in position during placing and setting of the
concrete. All reinforcing bars shall be bent cold in as smooth a curve as possible and shall conform to standard practice of the WCRSI. Reinforcing steel shall not be welded except as shown in the construction detail Drawings.

All reinforcing steel shall have a minimum of one inch (1") of concrete cover for controller cabinet and load center foundations, and three inches (3") of cover for signal pole and luminaire foundations.

Surface water shall not be permitted to enter the hole and all water which may have infiltrated in the hole shall be removed before placing concrete. Both forms and ground shall be thoroughly moistened before placing concrete. Each foundation shall be poured in one continuous pour.

Posts, poles and pedestals shall not be erected or placed on the foundation until ten (10) days after placement of the concrete. If the Engineer approves Type III Portland High-Early-Strength Cement Concrete, then posts, poles and pedestals may be placed on foundations four (4) days after placement of the concrete. Plumbing shall be accomplished by adjusting the nuts on the anchor bolts. Shims or other similar devices for plumbing or raking are not permitted.

After each slip-base post, pole or pedestal is in position, grout conforming to Section 80.05, Article 5.3 – Grouting for Slip-Base Poles, shall be placed under the base plate as shown on the Drawings, and shaped to present a neat appearance. Contractor shall install metal skirting on all non-slip-base posts, poles, or pedestals.

**Article 4.3 Load Center Foundation**

Excavate sixty inches (60") for base and install eighteen inches (18") of coarse aggregate for drain. Backfill above gravel in six inch (6") lifts with non-frost-susceptible material, compacting to ninety-five percent (95%) in accordance with Division 20 – Earthwork.

Install base so that cast-iron cover is flush with pavement, sidewalk, or finished grade. Slope grade away from base with minimum slope of three percent (3%). Use a pre-molded bituminous joint between base and concrete sidewalk or paving.

Install a three-quarter inch by ten foot (3/4"x10') copper clad ground rod inside the base, readily accessible through the removable steel cover. Install an additional external three-quarter inch by ten foot (3/4"x10') copper clad ground rod eight feet (8') from the load center, and additional ground rods as required by Code or the electrical utility.

Connect cast-iron cover of load center base to the ground rod with six feet (6') of copper braid with eyelets every six inches (6") and approved connectors.

Access opening shall be finished with a twenty-four inch (24") square iron frame and cover, approximately 280 pounds total weight, as provided by Olympic Foundry, Part No. SM70 or approved equivalent.
Install four (4) each, three-quarter inch (3/4") ferrule loop inserts for lifting, two (2) on each long side.

Provide one inch (1") chamfer on all exposed concrete edges.

For two-piece units, seal joint with pre-molded plastic bituminous type joint sealer.

**Article 4.4 Controller Cabinet Foundation**

Contractor shall install controller cabinet foundation in conformance with Standard Details 80-5, 80-6, 80-7, and 80-8. The top surface of controller cabinet foundations shall be eighteen inches (18") above finished grade and provided with a one inch (1") diameter drain hole connected to the cabinet interior and emptying above the ground line. All conduits shall be placed in the front half (door side) of the foundation to provide adequate wiring terminal block clearances.

Controller cabinet anchor bolts shall be as recommended by cabinet manufacturer and set with a template. Install a three-quarter inch by ten foot (3/4"x10') copper clad ground rod inside the base, readily accessible through the removable steel cover.

Controller cabinet foundations shall be installed in accordance with Section 80.04, Article 4.3, SubArticles 1, 2, 6, 7 and 8.

**Article 4.5 Driven Pile Foundation**

Contractor shall supply drive pile foundations shall be of the size and length indicated. Contractor shall ensure that the top surface of the anchor plate is three inches (3") above finished grade at luminaire pole locations and five inches (5") above finished grade for signal poles, or as indicated in the Drawings.

After welding on the pile cap adapter and anchor plate to the driven steel pile, Contractor shall cold galvanize the pile cap, the pile cap adapted, anchor plate, and the top three feet (3') of the steel pile including pile cap and anchor plate. Contractor shall furnish galvanization that complies with Federal Specification DOD-P-210354A (Galvanizing Repair Spec) and is U.L. listed. Contractor shall prepare steel surfaces and apply the cold galvanizing compound in accordance with the manufacturers’ recommendations. Five days prior to applying the cold galvanizing compound, Contractor shall provide the Engineer a copy of the manufacturers’ instructions.

**Article 4.6 Measurement**

Foundations will be measured as units, complete and in place.
Article 4.7 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Cast-In-Place Luminaire Pole Foundations</td>
<td>Each</td>
</tr>
<tr>
<td>Driven Pile Luminaire Pole Foundations</td>
<td>Each</td>
</tr>
<tr>
<td>Controller Cabinet Foundation (Type)</td>
<td>Each</td>
</tr>
<tr>
<td>Signal Mast Arm Pole Foundation</td>
<td>Each</td>
</tr>
<tr>
<td>Signal Pedestal Pole Foundation</td>
<td>Each</td>
</tr>
<tr>
<td>Pedestrian Pushbutton Pole Foundation</td>
<td>Each</td>
</tr>
<tr>
<td>Load Center Foundation (Type)</td>
<td>Each</td>
</tr>
<tr>
<td>Driven Pile Signal Pole Foundation</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.05   POLES, STEEL PEDESTALS AND POSTS

Article 5.1   General
All traffic signal and street lighting poles and arms shall be designed in accordance with the requirements of the 1994 AASHTO design criteria. Minimum Design wind velocity shall be the greater of one-hundred miles per hour (100 mph) or the AASHTO recommendation based upon a fifty (50) year mean recurrence interval dependent upon project location. A factor of 1.3 shall be used in design calculations to account for wind gusts. The minimum design loading for signal mast arm poles shall be as follows:

<table>
<thead>
<tr>
<th>Load #</th>
<th>Description</th>
<th>Weight (Lbs)</th>
<th>Projected Area (Ft²)</th>
<th>Centerline Distance From End of Arm (Ft)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>Sign</td>
<td>65</td>
<td>7.5</td>
<td>0.5</td>
</tr>
<tr>
<td>2</td>
<td>Signal</td>
<td>80</td>
<td>14.7</td>
<td>3.5</td>
</tr>
<tr>
<td>3</td>
<td>Signal</td>
<td>60</td>
<td>8.7</td>
<td>14</td>
</tr>
<tr>
<td>4</td>
<td>Sign</td>
<td>80</td>
<td>26</td>
<td>25 (See note 1)</td>
</tr>
<tr>
<td>5</td>
<td>Signal</td>
<td>60</td>
<td>8.7</td>
<td>35 (See note 2)</td>
</tr>
<tr>
<td>6</td>
<td>Signal</td>
<td>125</td>
<td>7.6</td>
<td>See note 3</td>
</tr>
<tr>
<td>7</td>
<td>Street Name Sign</td>
<td>2.0</td>
<td></td>
<td>See note 4</td>
</tr>
<tr>
<td>8</td>
<td>Street Name Sign</td>
<td>2.0</td>
<td></td>
<td>See note 4</td>
</tr>
</tbody>
</table>

Notes:
1. Mast arms shorter than twenty-six feet (26’) in length may disregard load #4.
2. Mast arms shorter than thirty-six feet (36’) in length may disregard load #5.
3. Load #6 shall be shaft mounted fourteen feet (14’) from the base of the pole.
4. Load #s 7 and 8 are shaft mounted twelve feet (12’) from the base of the pole.

Should project plan loading develop shear or moments greater than those related to the above loading, special design poles are required. These “Special Design” poles should be designed per the Drawings. All “Special Design” or Type SD poles will require calculation submittal to the Engineer prior to approval for use on the project.

The design for luminaire poles shall include a traffic sign with an area of twelve (12) square feet, located with its centroid nine feet (9’) above the base of the pole.

Manufacturer’s design calculations and wind stress certification letter shall be submitted to the Traffic Engineer or designated representative prior to approval of all signal poles and mast arms. Such certification shall include the stamp of a registered professional engineer. It shall list the pole numbers and the name of the project, and state that all poles
and mast arms meet or exceed the ability to support the minimum loads specified in Section 80.05, Article 5.1 - General.

Pre-approval of designs is available for poles meeting minimum design loading conditions. Pre-approval is not available for Special Design Poles. Pre-approval of submitted designs shall be at the sole discretion of the Traffic Engineer. Withdrawal of pre-approval may occur at anytime at the sole discretion of the Traffic Engineer. All design calculation submittals shall include the stamp of a Professional Engineer registered in the State of Alaska.

Use of non-compact sections shall not be allowed for any signal mast arm pole or arm. No exceptions shall be made.

All steel and iron products which are incorporated into poles, including connection and anchor bolts, shall be manufactured in the United States. All manufacturing processes starting with initial mixing and melting through the final shaping, welding and coating processes must be undertaken in the United States. Manufacturing includes smelting, rolling, extruding, machining, bending, grinding, drilling, painting and galvanizing. This does not apply to raw materials such as iron ore, pig iron, and processed, pelletized and reduced iron ore.

Material certifications for all poles, mast arms, bolts, steel plates and sheet steel shall be submitted to the Traffic Engineer or designated representative prior to acceptance of the poles and mast arms for payment. All mast arm connection bolts shall meet ASTM A325 and be galvanized in accordance with ASTM A153.

The Contractor shall verify the shaft lengths and mast arm connector plate locations of all poles to insure the Drawing mounting heights of luminaires and traffic heads are met. Traffic Signal personnel shall be allowed to perform an inspection of the poles in conjunction with review of submittals.

Poles less than fifteen feet (15’) in length shall be round or multisided (greater than sixteen [16] sides), and constructed of No. 11 or heavier U.S. standard gauge steel or four inch (4”) standard (Schedule 40) pipe or conduit, with the top designed for a post-top slip-fitter. Standard pipe shall conform to the specifications of ASTM A53. The tops of tapered poles shall have a four and one-half inch (4 1/2”) outer diameter. Pedestrian pushbutton posts shall be constructed of two and one-half inch (2 1/2”) standard (Schedule 40) pipe and meet the requirements of ASTM A53. Multi-sided poles shall not be used without prior approval of the Traffic Engineer.

Poles fifteen feet (15’) or longer shall be round or multisided and fabricated from sheet steel of weldable grade.

Poles may be fabricated of full-length sheets or shorter sections. When two pieces are used, the longitudinal welded seams shall be directly opposite one another. When the sections are butt-welded together, the welded seams on adjacent sections shall be placed to form continuous straight seams from base to top of pole.
Poles, prior to installation, shall be straight, with a permissive variation in sweep not to exceed one-quarter inch (1/4") per ten feet (10') of pole length.

A backing plate consisting of a metal sleeve shall be provided at all butt-welded, transverse joints. The sleeve shall be No. 12 U.S. standard gauge steel minimum, and made from steel having the same chemical composition as the steel in the pole.

The metal sleeve shall have a minimum length of three inches (3"). The sleeve shall be centered at the joint and have the same taper as the pole outside the sleeve in full contact with the inside of the standard throughout the sleeve length and circumference. The weld metal at the transverse joint shall extend to the sleeve, making the sleeve an integral part of the joint. In round poles, standard steel pipe or tubing may be substituted for the tapered backing sleeve, at the discretion of the Engineer.

All welds shall be continuous. All welding practices shall conform to current AWS Code, AWS D1.1, latest edition.

All exposed welds, except fillet welds shall be ground flush with the base metal.

All exposed edges of the plates which make up the base assembly shall be finished smooth, and all exposed corners of such plates shall be neatly rounded to one and one-half inch (1 1/2") radius, unless otherwise shown on the Drawings. Anchor holes in the base plate shall be round. Slotted holes shall not be used. Slotted shafts shall be provided with slip-fitter shaft caps of either galvanized steel or cast aluminum.

Non-Destructive Testing (NDT) may be required by the Traffic Engineer on all newly manufactured poles for this project, prior to galvanization. A licensed technician shall perform all testing. NDT shall take the form or Magnetic Particle or Ultrasonic testing, and be performed as described in the current AASHTO standard. If required all NDT reports shall be submitted prior to acceptance for payment.

No exception to the practices mandated by AASHTO shall be allowed.

Poles shall not be relocated or re-used unless Contractor obtains written approval of the Traffic Engineer or designated representative. Damage to the galvanized or painted surface of existing poles to be relocated or reused in place shall be repainted in accordance with Section 80.16, Article 16.3 – Galvanizing or Article 16.4 – Painting for Steel Structures, as appropriate. Holes greater than five-eighths inches (5/8") in diameter in the shafts of existing poles, due to removal of equipment, shall be repaired. Holes shall be repaired by tapping the hole, coating all exposed edges with zinc rich paint, and plugging the hole with a screw-in type steel plug of the correct size. The plug shall be galvanized, or shall be completely covered with zinc rich paint. Holes less than five-eighths inch (5/8") diameter shall be ground smooth so there are no notches or cracks, and coated with zinc rich paint. Plugging holes and repainting damaged galvanized or painted surfaces shall be incidental to other Work.
Extent of additional repairs or replacements will be determined by the Traffic Engineer or
designee, and said repairs or replacements ordered will be paid for as extra Work as
provided in Division 10, Sections 10.05 – Control of Work and 10.07 – Measurement and
Payment.

New steel posts, poles, and mast arms shall be hot-dip galvanized after fabrication in
conformance with the ASTM A123. Any damage to the galvanized surfaces that occurs
during shipping, or during the construction process, shall be repainted in accordance with
Section 80.16, Article 16.3 - Galvanizing or Article 16.4 – Painting for Steel Structures, as
appropriate, prior to final acceptance of the poles and mast arms.

All poles and arms shall have permanent identification tags, readily visible, that identify
each pole and arm. One tag shall be mounted at the base of the pole shaft and one tag
shall be mounted at the base of the mast arm. The tag shall list the following information:

- Pole Number
- Shaft Length
- Mast Arm Length
- Pole Type (SD if applicable)
- Date of Manufacture
- Manufacturer Name.
- Luminaire Arm Length
- Manufacturer Order Number

**Article 5.2 Plumbing**

Plumbing shall be accomplished by adjusting the nuts on the anchor bolts prior to grouting.
A slight raking of the pole will be provided by plumbing the side away from the road.
Shims or other similar devices for plumbing or raking will not be permitted.

**Article 5.3 Grouting for Slip-Base Poles**

The Contractor shall use a premixed grout having a minimum twenty-eight (28) day
compressive strength of four thousand pounds per square inch (4000 psi). Proprietary
grout mixtures shall be utilized in accordance with the recommendations of the
manufacturer.

Concrete areas to be in contact with the grout shall be cleaned of all loose and foreign
matter that would in any way prevent bond between the mortar and the concrete surfaces.

Contractor shall not grout unless ambient temperature will remain a minimum temperature
of forty-five degrees Fahrenheit (45°F) for three days after grouting. All improperly cured
or otherwise defective grout shall be removed and replaced at the Contractor’s expense.
No load shall be placed on the grout until it has set for at least ninety-six (96) hours.
For concrete bases, after each post, pole or pedestal is in position, grouting conforming to this Article shall be placed under the base plate as shown on the Drawings, and shaped to present a neat appearance.

Article 5.4  Galvanizing

All signal poles, mast arms, and pedestal poles shall be hot dipped galvanized in accordance with Section 80.16, Article 16.3 – Galvanizing.

Article 5.5  Measurement

Fixed-base luminaire poles shall be measured as units complete and in place, including all hardware, all wiring within the poles, and grouting of the base.

Direct-bedded luminaire poles shall be measured as units complete and in place, including all hardware and all wiring within the poles.

Slip base luminaire poles shall be measured as units complete and in place, including slip base adapter, all hardware, and all wiring within the pole.

Signal mast arm poles and signal pedestal poles shall be measured as complete and installed with all hardware, all wiring within the pole, and either grouting of the base or base plate skirt as appropriate.

Combination signal-luminaire poles shall be measured as complete and installed with all hardware, luminaire brackets, all wiring within the pole, and base plate skirt.

Pedestrian push button poles shall be measured as complete and installed with all hardware, all wiring within the pole, and grouting of the base.

All luminaires, luminaire arms, signal heads, pedestrian signal heads, pedestrian pushbutton assemblies, signal mast arms, signs and optical preemption detectors shall be installed and accepted when poles are measured for payment, but shall not be included in payment for poles. These items shall be considered separate pay items, and measured under the appropriate Sections of these Specifications. All other hardware, including wiring within the pole and grouting of the base, shall be considered incidental to the pay items for poles, and shall not be measured for payment.
Article 5.6  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Ft. Slip Base Luminaire Pole</td>
<td>Each</td>
</tr>
<tr>
<td>Ft. Fixed Base Luminaire Pole</td>
<td>Each</td>
</tr>
<tr>
<td>Ft. Direct-Bedded Luminaire Pole</td>
<td>Each</td>
</tr>
<tr>
<td>Signal Mast Arm Pole</td>
<td>Each</td>
</tr>
<tr>
<td>Combination Signal/Luminaire Pole (MTG Height)</td>
<td>Each</td>
</tr>
<tr>
<td>Signal Pedestal Pole</td>
<td>Each</td>
</tr>
<tr>
<td>Pedestrian Pushbutton Pole</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.06  MAST ARMS

Article 6.1  General

Design of signal and luminaire mast arms shall be in accordance with the requirements of the 1994 Edition of the “Standard Specifications for Structural Supports for Highway Signs, Luminaire, and Traffic Signals,” as published by AASHTO. Minimum design wind velocity shall be the greater of one hundred miles per hour (100 mph) or the AASHTO recommendation based upon a fifty (50) year mean recurrence interval dependent upon project location. A factor of 1.3 shall be used in design calculations to account for wind gusts.

Arms less than fifteen feet (15’) in length shall be round or multisided, and constructed of No. 11 or heavier U.S. standard gauge steel, or four inch (4”) standard (Schedule 40) pipe or conduit. Standard pipe shall conform to the specifications of ASTM A53.

Arms fifteen feet (15’) or longer, shall be round or multisided, and fabricated from sheet steel of weldable grade.

Arms may be fabricated of full-length sheets or shorter sections. Each section shall be fabricated from not more than two (2) pieces of sheet steel for lengths up to forty feet (40’). Where two (2) pieces are used, the longitudinal welded seams shall be directly opposite one another. When the sections are butt-welded together, the welded seams on adjacent sections shall be placed to form continuous straight seams from base end of arm.

A backing plate consisting of a metal sleeve shall be provided at all butt-welded, transverse joints. The sleeve shall be No. 12 U.S. standard gauge steel minimum and made from steel having the same chemical composition as the steel in the pole. The metal sleeve shall have a minimum length of three inches (3”). The sleeve shall be in full contact with the inside of the standard throughout the sleeve length and circumference. The weld metal at the transverse joint shall extend to the sleeve, making the sleeve an integral part of the joint.

Pole plate to pole shaft connection shall be of the “closed box” type with top and bottom plates of the box forming a continuous stiffening ring around the pole. Gusset assemblies for this connection shall be butt-welded together. Vent holes, necessary for galvanizing, shall be used.

All welds shall be continuous. All welding practices shall conform to current AWS Code, AWS D1.1, latest edition.

Non-Destructive Testing (NDT) may be required by the Traffic Engineer on all newly manufactured poles for this project, prior to galvanization. A licensed technician shall perform all testing. NDT shall take the form of Magnetic Particle or Ultrasonic testing, and be performed as described in the current AASHTO standard. If required all NDT reports shall be submitted prior to acceptance for payment.
No exception to the practices mandated by AASHTO shall be allowed.

All exposed welds, except fillet welds and welds on top of mast arms shall be ground flush with the base metal.

All exposed edges of the plates which make up the base of the arm shall be finished smooth and all exposed corners of such plates shall be neatly rounded to one-eighth inch (1/8") radius, unless otherwise shown on the Drawings. Bolt holes in the mast arm base plate shall be round. Slotted holes shall not be allowed. Mast arm ends shall be provided with slip-fitter shaft caps of either galvanized steel or cast aluminum.

Damage to the galvanized or painted surface of existing arms to be relocated or reused in place shall be repainted in accordance with Section 80.16, Article 16.3 – Galvanizing or Article 16.4 – Painting for Steel Structures, as appropriate. Holes greater than three-eighths inch (5/8") in diameter in the shafts of existing arms, due to removal of equipment, shall be repaired. Holes shall be repaired by tapping the hole, coating all exposed edges with zinc rich paint, and plugging the hole with a screw-in type steel plug of the correct size. The plug shall be galvanized, or shall be completely covered with zinc rich paint. Plugging holes and repainting damaged galvanized or painted surfaces shall be incidental to other Work.

Extent of additional repairs or replacements will be determined by the Traffic Engineer or designee, and said repairs or replacements ordered will be paid for as extra Work as provided in Division 10, Sections 10.05 – Control of Work and 10.07 – Measurement and Payment.

**Article 6.2 Measurement**

Signal and luminaire arms will be measured as units complete and in place, including labor, equipment, and material necessary to make a complete and functioning unit.

All luminaires, signal heads, signs and optical preemption detectors shall be installed and accepted when mast arms are measured for payment, but shall not be included in payment for mast arms. These items shall be considered separate pay items, and measured under the appropriate sections of these Specifications. All other hardware, including wiring within the arms, shall be considered incidental to the pay items for signal mast arms or luminaire arms, and shall not be measured for payment.
Article 6.3 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Signal Mast Arm Ft. Length</td>
<td>Each</td>
</tr>
<tr>
<td>Luminaire Arm Ft. Length</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.07 CONDUIT

Article 7.1 General

Contractor shall run electrical conductors in conduit, except for overhead wiring, wiring inside poles and when otherwise called for in the Drawings. All conduit and fittings shall be galvanized, rigid type manufactured of mild steel or wrought iron conforming to U.L. Underwriters Laboratory Standard UL-6 and hot dip galvanized in accordance with American National Standards Institute specification ANSI C-80.1. If called for in the Drawings, rigid non-metallic type conduit shall conform to the UL Standard UL-651. Only one type of conduit shall be used in any one run from one junction box to another. Where non-metallic conduit is to be installed, the conduit runs between a load center and the nearest junction box shall be of the rigid metal type.

Conduit and fittings to be installed on the surfaces of poles or in structures and foundations shall be rigid metal type as specified above for underground installations.

Couplings for new rigid metal conduit shall be threaded. Where existing conduit is intercepted and extended, twist-on compression type couplings will be allowed. Set-screw couplings are not allowed on the project.

Conduit in foundations for ground rods shall be one inch (1") diameter.

The ends of all conduits, whether shop or field cut, shall be threaded and reamed to remove burrs and rough edges. Cuts shall be made square and true so that the ends will butt together for the full circumference. Slip joint or running threads will not be permitted for coupling conduit. When a standard coupling cannot be used for coupling metal type conduit, an approved threaded union coupling shall be used. The threads on all ferrous metal conduit shall be painted with rust preventative paint before couplings are made up. All couplings for metal type conduit shall be screwed up until the ends of the conduits are brought together, providing a good electrical connection throughout the entire length of the conduit run. Where the coating on ferrous metal conduit has been damaged, such damaged places shall be painted with rust preventative paint.

Until wiring is started, all conduit ends shall be capped with standard pipe caps or approved plug and coupling combinations. When caps are removed, the threaded ends shall be provided with approved conduit grounding bushings.

Contractor shall lay conduit to a minimum depth of thirty inches (30") below finished grade. See Division 20, Section 20.13 - Trench Excavation and Backfill for backfill requirements.

Clean all debris and moisture out of conduits before installing conductors or cables.

If the conduit is for thaw wire only, then:

Fittings for use in below-grade storm drains shall be suitably rated as NEMA 7, complete with gaskets for watertight installations.
Provide suitable conduit seals and sealant to make connections to junction boxes installed with manholes watertight.

Junction boxes for installation in manholes shall be NEMA Type 7, with gasketed covers for watertight installations.

Couplings and all threaded connections shall be provided with Teflon tape or approved water treatment applied to threads before tightening.

Bottom of trenches for non-metallic conduit shall be relatively free of sharp irregularities which would cause pinching and excessive bending of the conduit. The first six inches (6") of backfill shall be free of rocks exceeding the one inch (1") maximum dimension.

Conduit entering the bottom of concrete junction boxes shall terminate with a ninety degree (90°) sweep inside the box wall. Conduit openings shall terminate not less than five inches (5") above the bottom of all boxes and a minimum of six inches (6") below the top of the Type I and Type IA boxes and twelve inches (12") below the top of Type II and Type III boxes. Conduits entering through the junction box wall shall extend a minimum of two inches (2") inside the box wall, and be a minimum of six inches (6") above the bottom.

All foundations shall be furnished with conduits as shown in the Drawings. The conduits shall extend a maximum of four inches (4") vertically above the foundation and slope towards the hand-hole opening.

Conduit runs shall avoid drainage collection points where possible. At low points in all conduit runs, a one-half inch (1/2") drain hole shall be drilled in the bottom of the lower straight section of the sweep elbow and sump containing approximately two cubic feet of coarse concrete aggregate material shall be installed. Additional drains shall be placed adjacent to all junction boxes and structures, regardless of the method of conduit placement employed. Drilled holes in conduit shall be deburred inside and out to prevent scraping of conductors. The exterior of the one-half inch (1/2") hole shall be wrapped with approved filter cloth material and secured as directed or approved by the Engineer.

Conduits for future use shall be provided with grounding bushings, bonded to ground, and capped with an approved plastic insert type or expandable rubber plug. A polypropylene pull rope with two hundred pound (200 lb) minimum tensile strength shall be installed in all conduits which are to receive future conductors. At least two feet (2’) of pull rope shall be doubled back into the conduits at each end.

Contractor shall mark all underground conduits with a continuous strip of 4-mil-thickness, six inch (6") width polyethylene marker tape. Contractor shall mark the tape with a black legend on a red background and buried nine inches plus or minus three inches (9" ± 3") below the finished grade. Contractor shall place two strips of marker tape side-by-side under all road crossings.

Where new junction boxes are placed in existing rigid metal conduit runs, the conduit shall be fitted with threaded bushings and bonded.
Conduit leading to soffit, wall or other lights or fixtures below the grade of the junction box shall be sealed by means of an approved sealing fitting and sealing compound.

Existing underground conduit without conductors to be incorporated into a new system shall be cleaned with a mandrel or cylindrical wire brush and blown out with compressed air.

The Contractor, at his expense, may use conduit of larger size than shown on the Drawings, and where used, it shall be for the entire length of the run from outlet to outlet. Reducing couplings are not permitted.

When extending existing conduits or installing junction boxes in existing conduit runs, extend the conduit into the proposed junction box or foundation using drains, elbows and bonding as required for new installations. When adjusting junction boxes, shorten or lengthen existing conduits to meet clearance requirements. Complete extensions and modifications to existing conduits using the same size and types of materials.

Contractor shall clean all debris and moisture out of conduits before installing conductors or cables.

All abandoned conduits shall be removed from junction boxes.

All knockouts for new conduit or removed conduit shall be grouted.

All knockouts for conduits entering through the side of junction boxes shall be grouted.

Cut off abandoned conduits flush with the inside wall or bottom of junction boxes. Contractor shall remove all conductors prior to abandoning conduit.

**Article 7.2 Measurement**

Measurement for furnishing and installing conduit is per linear foot of the size and type set forth in the Drawings and Bid Schedule. Measurement is the horizontal distance from center of device to center of device, or from station to station. Measurement shall include all fittings, couplings, pull wires, caps and elbows, and bonding and grounding conductors, which shall be considered incidental to conduit installation.

Conduits installed in manhole and catch basins will not be measured, but rather the following distances will be considered standard unless determined otherwise by the Engineer:

- **Manhole**: forty feet (40’)
- **Catch Basin**: sixteen feet (16’)
Article 7.3  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section. Payment for trench, backfill, and wire are separate bid items.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Steel Conduit (Size)</td>
<td>Foot</td>
</tr>
<tr>
<td>Schedule 80 PVC Conduit (Size)</td>
<td>Foot</td>
</tr>
</tbody>
</table>
SECTION 80.08  JUNCTION BOXES

Article 8.1  General

The Work under this Section consists of performing all operations pertaining to removing and adjusting existing junction boxes to grade and for furnishing and installing a new junction box of the type specified. This Work shall include all excavation, bedding material, and bonding and grounding hardware.

Type I junction boxes shall not be used in traffic signal systems.

Junction boxes shall be installed at the approximate locations shown on the Drawings. The Contractor, at his expense, may install additional junction boxes to facilitate his Work. Junction boxes shall be located so they are not in the roadway, sidewalk, driveway, or pathway surfaces, unless otherwise noted in the Drawings. Where practical, junction boxes shown in the vicinity of curbs shall be placed adjacent to the back of curb. Junction boxes shall not be located in drainage collection areas.

Article 8.2  Materials

Contractor shall provide precast reinforced concrete boxes (junction boxes) with cast iron lids, of the sizes and details shown on the Drawings.

Contractor shall provide precast reinforced concrete additions (junction box extensions) of the sizes and details shown on the Drawings with dimensions confirmed by field measurements.

Illumination junction boxes shall be pre-cast reinforced concrete with cast iron lids, or polymer concrete boxes of the sizes and details shown on the Drawings.

All Portland concrete cement utilized in the adjustment of the Junction Box shall conform to the requirements as specified in Division 55, Section 55.05 - Manholes and Catch Basin Manholes. The joint sealing compound utilized to seal the joint between the electrical vault’s lid and walls shall be Ram-Nek Flexible Plastic Gasket or an approved equal.

Article 8.3  Construction

All junction boxes with metal covers shall have the covers effectively grounded with a four foot (4') tinned copper braid for Type I and Type IA Junction Boxes or a six foot (6’) tinned copper braid for Type II and Type III Junction Boxes. Use only stainless steel bolt assembly components to attach bonding braid to the cover (lid). Bond junction box lids to the grounding conductor using copper braid with a cross sectional area equal to an 8 AWG conductor and eyelets spaced at six inch (6”) intervals.

The entire bottom of all junction boxes shall be bedded in coarse concrete aggregate material of a minimum depth of eighteen inches (18”).
Top of junction boxes shall be one-quarter inch (1/4") below the sidewalk grade or top of adjacent curb. When located in an unpaved section adjacent to a paved shoulder the junction box shall be located one inch (1") below the finished grade and shall be installed one-quarter inch (1/4") below the surface in paved areas. Junction boxes located in areas requiring grading shall be adjusted as directed by the Engineer. Junction boxes located in seeded areas shall be adjusted to two inches (2") below the surface.

Junction boxes shall be located immediately adjacent to the pole or fixture they serve and at additional intervals to reduce the distance between junction boxes to:

1. 400 feet maximum for 25 pair interconnect cable.
2. 200 feet maximum for any other conduit runs.
3. If the limitations require additional junction boxes they shall be located on equal spacings subject to the above limitations.

Emboss the word LIGHTING on the lids of all junction boxes containing only lighting or thaw wire conductors, or only lighting and signal controller power conductors. Emboss the word TRAFFIC on the lid of all other junction boxes.

No later than forty-eight (48) hours prior to commencement of Work on adjustment of the Junction Box, Contractor shall contact the Traffic Department.

Prior to removal of the Junction Box associated with traffic detector loops, Contractor, Engineer, and a Traffic Department representative shall inspect and verify the condition of the Junction Box.

Prior to replacement of the Junction Box, Contractor, Engineer, and a Traffic Department representative shall inspect the vault lid and vault structure to verify adjustments. Any Work, personnel, and/or materials required to properly correct problems shall be at Contractor’s expense.

**Article 8.2 Measurement**

The method of measurement is the actual number of junction boxes removed, adjusted to grade and accepted, and the actual number of new junction boxes of the specified types furnished, installed, and accepted.

The unit cost for adjusting the Junction Box to finish grade shall include all labor, materials, and equipment. This shall include all required usable and unusable excavation, classified fill and backfill material, compaction, concrete cutting and removal, and required personnel.
Article 8.3  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Junction Box (Type)</td>
<td>Each</td>
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<tr>
<td>Adjust Junction Box to Grade</td>
<td>Each</td>
</tr>
<tr>
<td>Remove Junction Box</td>
<td>Each</td>
</tr>
<tr>
<td>Junction Box Extension (Type)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.09   EXPANSION FITTINGS

Article 9.1   General

Expansion fittings, as detailed on the structure Drawings, shall be installed where the conduit crosses an expansion joint in the structure. Each expansion fitting shall be provided with a bonding jumper of stranded, No. 6 AWG, copper wire.

Expansion-deflection fittings shall be waterproof and permit a three-quarter inch (3/4”) expansion and contraction and a three-quarter inch (3/4”) deflection without deformation.

Article 9.2   Measurement

Expansion fittings shall be considered as incidental to other Work.

Article 9.3   Basis of Payment

No separate payment for these items is allowed.
SECTION 80.10    CONDUCTORS

Article 10.1    General

Conductor sizes shall be based on the American Wire Gauge (AWG). Sizes shall conform to the Drawings or, when not shown, to the Conductor Termination Table below. Conductors shall be seven-conductor No. 14 AWG (7C-#14 AWG) for all vehicle heads, and five-conductor No. 14 AWG (5C-#14 AWG) for all pedestrian heads.

Conductor Termination Table

<table>
<thead>
<tr>
<th>CONDUCTORS/CABLE</th>
<th>CIRCUIT</th>
<th>WIRE COLOR</th>
<th>AWG NO</th>
<th>BAND LEGEND</th>
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<tbody>
<tr>
<td>7</td>
<td>Vehicle Red Arrow</td>
<td>Red</td>
<td>14</td>
<td>Head Number</td>
</tr>
<tr>
<td></td>
<td>Vehicle Yellow Arrow</td>
<td>Orange</td>
<td></td>
<td></td>
</tr>
<tr>
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<td>Vehicle Green Arrow</td>
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<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>Common Neutral</td>
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<td></td>
</tr>
<tr>
<td></td>
<td>Spare</td>
<td>White/Black</td>
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<td></td>
</tr>
<tr>
<td></td>
<td>Spare</td>
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<td>Spare</td>
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<td></td>
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<tr>
<td>7</td>
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<td>Red</td>
<td>14</td>
<td>Head Number(s)</td>
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<td>Vehicle Yellow</td>
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<td></td>
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<td></td>
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<td></td>
<td>Common Neutral</td>
<td>White</td>
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</tr>
<tr>
<td></td>
<td>Vehicle Yellow Arrow</td>
<td>White/Black</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>Vehicle Green Arrow</td>
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<td></td>
<td></td>
</tr>
<tr>
<td>5</td>
<td>Pedestrian Don't Walk</td>
<td>Red</td>
<td>14</td>
<td>Head Number</td>
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<tr>
<td></td>
<td>Pedestrian Walk</td>
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<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>Common Neutral</td>
<td>White</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>Spare</td>
<td>Orange</td>
<td></td>
<td></td>
</tr>
<tr>
<td></td>
<td>Spare</td>
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<td></td>
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<tr>
<td>5</td>
<td>Photo Elec. Control</td>
<td>Black</td>
<td>14</td>
<td>PEC</td>
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<td>PEC Load to Contactor</td>
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<td>Neutral</td>
<td>White</td>
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<td></td>
</tr>
<tr>
<td></td>
<td>Spare</td>
<td>Orange</td>
<td></td>
<td></td>
</tr>
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<td>Spare</td>
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<td></td>
</tr>
<tr>
<td>3</td>
<td>Pedestrian Pushbutton</td>
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<td>Head Number</td>
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<td>Neutral</td>
<td>White</td>
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<td>Spare</td>
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</tr>
</tbody>
</table>
Conductor Termination Table

<table>
<thead>
<tr>
<th>CONDUCTORS/ CABLE</th>
<th>CIRCUIT</th>
<th>WIRE COLOR</th>
<th>AWG NO.</th>
<th>BAND LEGEND</th>
</tr>
</thead>
<tbody>
<tr>
<td>3</td>
<td>Preemption Confirmation Light Neutral Spare</td>
<td>Black White Red</td>
<td>14</td>
<td>“PRE” Conf Lt</td>
</tr>
<tr>
<td>3</td>
<td>Highway Luminaire</td>
<td>Black Red White</td>
<td>8</td>
<td>Circuit Number</td>
</tr>
<tr>
<td>3</td>
<td>Service to Controller</td>
<td>Black White Red</td>
<td>6</td>
<td>“SIG” No Band</td>
</tr>
<tr>
<td>3</td>
<td>Sign Luminaire</td>
<td>Black Red White</td>
<td>8</td>
<td>SIGN</td>
</tr>
</tbody>
</table>

All insulated conductors shall consist of uncoated, stranded copper conforming to the specifications of ASTM B8, except for detector loop lead-in which shall consist of stranded, tinned copper.

Grounding conductors shall be bare copper of the gauge required by the Code and may be stranded, solid or braided.

Conductors used for the following purposes shall conform to the referenced specifications.

**Article 10.2 Control Cables**

Vehicular signal faces, pedestrian signal faces, pedestrian pushbutton detectors, flashing beacons, preemption devices, and photo electric controls shall be wired with signal cable conforming to IMSA 20-1.

The three-conductor No. 20 AWG (3C-#20 AWG) cable shown on the Drawings shall be used in an optically activated preemption system. The cable shall be sheathed in a black PVC jacket and include three (3) No. 20 AWG insulated conductors, and one (1) No. 20 AWG drain wire enclosed within an aluminized polyester shield. All conductors shall be stranded, individually tinned copper. The cable shall contain one yellow, one blue, and one orange insulated conductor. The cable shall be rated for 600 volts operation and be suitable for direct bury, installation in a conduit, and direct exposure to the atmosphere. Cable shall be a Minnesota Mining and Manufacturing 3M No. 138 Opticom cable, or approved equal.

**Article 10.3 Power Conductors and Cables**

Power conductors and cable shall conform to ICEA Publication No. S-66-524, NEMA Publication No. WC7, and U.L. Standards. Conductors shall be insulated with chemically
cross-linked polyethylene conforming to U.L. type XHHW or XHHW-2. Insulation shall be rated for 600 volt operation.

Three conductor cables shall have black, white, and red colored conductors.

All single-wire conductors and cables shall have clear, distinctive and permanent markings on the outer surface throughout the entire length giving the manufacturer's name or trademark, the insulation type-letter designation, the conductor size, voltage rating and the number of conductors if a cable.

Highway and sign illumination cable shall consist of insulated conductors with a low density, high molecular weight polyethylene jacket.

Power cables with conductors No. 6 AWG and larger shall be PVC or neoprene jacketed.

Load center control circuit wiring shall be No. 12 AWG XHHW.

Conductors in controller cabinets that carry the full signal load circuit shall be No. 10 AWG or larger.

All cabinets shall be wired with conductors sized to handle the amperage drawn under full cabinet use.

Illumination tap conductors that run from the fused disconnect kit in the pole base to the luminaire shall be No. 10 AWG.

**Article 10.4 Detector Loops and Lead-In Cables**

Conductors for detector inductive loops shall be UL listed as Tube loop detector wire #14 AWG stranded single conductor in PVC tube (IMSA specification 51-5).

Loop Lead-in Cables. Use a *tray* cable that conforms to the following specifications to connect the loop detectors to the terminal blocks in the controller cabinet. Furnish this cable, also known as Snyder Cable, manufactured according to UL Standard 1277. Third-party certify these cables as Type TC and certified for use in underground conduit or as an aerial cable supported by a messenger, rated for 600 volts AC operation and sunlight resistance.

Use size 18 AWG, sixteen (16) strand, tinned copper conductors per ASTM B-33 insulated with wet rated cross-linked polyethylene. Twist the conductors into seven (7) pairs colored to match the following: Black & Red, Black & White, Black & Green, Black & Brown, Black & Yellow, Black & Orange and Black & Blue.

Provide each twisted pair with an overall aluminum foil coated Mylar shield that provides one hundred percent (100%) coverage and a 20 AWG tinned copper drain that is in constant contact with the foil side of the shield. Apply a tight fitting PVC jacket over the conductor assembly.
Article 10.5 Telemetry Cable

Interconnect cable shall consist of solid copper #19 AWG conductors of the number of pairs called for in the Drawings meeting the requirements of REA specification PE-39 for filled telephone cables. The shield may be either copper or aluminum.

Telemetry interconnect cable shall contain the number of pairs as shown on the Drawings. The conductors shall be covered with a .005-inch copper shield that is electrically intact throughout the entire length of the new circuit. Grounding continuity of all copper shields shall be maintained at all termination points. T-Splices shall be made at the terminal block in the controller cabinet. Configuration and color coding shall be in accordance with the Interconnect Termination Table.

<table>
<thead>
<tr>
<th>Pair #</th>
<th>Tip</th>
<th>Ring</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>White</td>
<td>Blue</td>
</tr>
<tr>
<td>2</td>
<td>White</td>
<td>Orange</td>
</tr>
<tr>
<td>3</td>
<td>White</td>
<td>Green</td>
</tr>
<tr>
<td>4</td>
<td>White</td>
<td>Brown</td>
</tr>
<tr>
<td>5</td>
<td>White</td>
<td>Slate</td>
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<tr>
<td>6</td>
<td>Red</td>
<td>Blue</td>
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<td>7</td>
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<td>9</td>
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<tr>
<td>11</td>
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<td>Blue</td>
</tr>
<tr>
<td>12</td>
<td>Black</td>
<td>Orange</td>
</tr>
<tr>
<td>13</td>
<td>Black</td>
<td>Green</td>
</tr>
<tr>
<td>14</td>
<td>Black</td>
<td>Brown</td>
</tr>
<tr>
<td>15</td>
<td>Black</td>
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</tr>
<tr>
<td>16</td>
<td>Yellow</td>
<td>Blue</td>
</tr>
<tr>
<td>17</td>
<td>Yellow</td>
<td>Orange</td>
</tr>
<tr>
<td>18</td>
<td>Yellow</td>
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</tr>
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<td>19</td>
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<tr>
<td>21</td>
<td>Violet</td>
<td>Blue</td>
</tr>
<tr>
<td>22</td>
<td>Violet</td>
<td>Orange</td>
</tr>
<tr>
<td>23</td>
<td>Violet</td>
<td>Green</td>
</tr>
<tr>
<td>24</td>
<td>Violet</td>
<td>Brown</td>
</tr>
<tr>
<td>25</td>
<td>Violet</td>
<td>Slate</td>
</tr>
</tbody>
</table>

Article 10.6 Measurement

All conductors and cables are measured in linear feet from center of device to center of device, or from station to station. All terminations, markings, slack and other incidental supplies required to meet the provision of the Specifications are not measured, and are considered incidental to the Contract.
Wire and cable within poles, cabinets, and other devices are included under those bid units.

**Article 10.7 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>(No. of Conductors) (Size of Conductors) (Type of Cable)</td>
<td>Foot</td>
</tr>
</tbody>
</table>
SECTION 80.11 WIRING

Article 11.1 General

Conductors in fixtures and cabinets shall not be spliced and shall be cabled together with self clinching nylon ties. All conductors, including spares shall be attached to terminal blocks with "spade" type terminal lugs.

Conductors shall not be pulled into conduit until junction boxes are set to grade, crushed rock sumps installed, grout placed around the conduit, and rigid metal conduits are bonded to ground.

Conductors shall be pulled by hand or by commercially built cable pulling equipment that is specially designed for that purpose. The cable pulling device shall be equipped with a force limiting circuit and force gauge. The cable-pulling device shall be approved by the Engineer before it is allowed to be used. Powdered soapstone, talc, or other inert lubricant shall be used in placing the cables and conductors in conduit.

When new conductors are to be added to a conduit with existing conductors, all conductors shall be removed and the conduit cleaned with a mandrel or brush. Then both old and new conductors shall be pulled through as a unit. In a new installation, all conductors shall be pulled through the conduit as a unit.

Contractor shall leave at least three feet (3') of slack, but not more than four feet (4'), for each conductor at each lighting and combination pole, and in each junction box, splice location, and controller base. Where lighting conductors are contained in a conduit within the pole, the slack is not required.

Contractor shall install a nylon pull string in all conduits where cable is replaced and/or removed.

The neutral for pedestrian push button circuits shall be separate from the signal light circuit neutral.

All signal conductors shall be run continuously without splices from a terminal block located in a cabinet, compartment, or signal head, to a similarly located terminal block.

Contractor shall splice illumination cable in pole bases and shall use approved fuse kits only. Contractor shall not use wire binding screws, studs or nuts.

With the prior approval of the Engineer, Contractor may use illumination cable splices in junction boxes. Contractor shall join the individual conductors by the use of non-insulated, overlap type pressure connectors insulated with mastic-lined shrink tubing. Contractor shall not use wire binding screws, studs or nuts. Contractor shall stagger splices to minimize overall diameter.
Contractor shall encapsulate illumination cable conductor splices in a rigid, two-piece, transparent, snap together, plastic mold specifically designed for each splice type. Molds shall have dimensions suitable for the splice, encase the cable outer protective jackets, be rated for 600 volts, and have fill and vent funnels for epoxy resin. Contractor shall fill the splice mold bodies, with epoxy resin, that are resistant to weather, aromatic and straight chain solvents, and do not sustain combustion.

Splices shall be insulated by: 1) a heat shrink tubing internally coated with an approved sealing compound or, 2) a cast of self-curing epoxy resin which is compatible with the wire insulation to form a weatherproof joint. Each insulated kit shall encompass only one cable and include the outer protective jacket(s).

Loop lead-in cable shall be run in a continuous manner without splices from the controller cabinet to the curbside detection junction box. Splicing of the loop conductors to the lead-in cable shall be in conformance with Section 80.18, Article 18.1 – Loop Detectors.

When multiple pair loop lead-in cables are specified, each detector loop shall be soldered to an individual shielded pair. Each pair shall then run without splices to the terminal block in the controller assembly, where all series or parallel connections shall be made. At a splice location, a short section of the cable jacket shall be removed and only the shielded pairs dedicated to the loops being spliced shall be cut. Where allowed by the wiring diagram, loops calling different phases shall be connected to separate lead-in cables. The soldered connections shall be insulated with mastic lined heat shrink tubing or two layers of one-half lapped UL listed electrical tape and encapsulated in a waterproof splice kit with re-enterable fill.

Illumination cable conductor splices and lead-in cable splice shall be encapsulated in a rigid, two piece, transparent, snap together, plastic mold specifically designed for each splice type. Molds shall have dimensions suitable for the splice, encase the cable jackets, be rated for 600 volts, and have fill and vent funnels for epoxy resin. The splice mold bodies shall be filled with epoxy resin that is resistant to weather, aromatic and straight chain solvents, and which shall not sustain combustion. Reenterable fill shall be utilized for detector splices.

All cables and single wire conductors shall be permanently identified using labels in all pole bases and cabinets, at each detector loop tail/lead in cable and illumination cable splices and in junction boxes.

Contractor shall furnish the two types of identification tags listed below that require a written legend, and write the legends specified neatly and legibly, using a black marking pen specified by the manufacturer. Contractor shall ensure that legends conform to Section 80.10, Conductor Termination Table, or as shown on the Drawings or detailed in the Special Provisions. Contractor shall replace, at no expense to the Owner, all identifications tags that the Engineer deems are illegible.

1. Use identification cable ties for labeling loop detector tails and for each set of paired loop lead-in conductors in the controller cabinet. Furnish identification
cable ties made of nylon that have a nonmagnetic stainless steel locking device embedded in the head and a tag attached “flag style” to the head. The cable ties shall consist of a single strap with a minimum size tag of three-quarters inch by five-sixteenth inch (3/4” x 5/16”).

2. To label all other cables, use cable tags made of nylon reinforced vinyl that is impervious to the elements and will not tear. Provide tags with a four inch by one and three-quarters inch (4” x 1 3/4”) minimum size that are attached flag style at one corner to a single strap. Furnish yellow tags for labeling all signal and interconnect cables and red tags for labeling lighting and feeder cables.

Contractor shall remove abandoned conductors/cables.

The control and power cables shall be terminated as shown in 80.10, Conductor Termination Table. Three conductor power cables shall always have a spare. The white or red conductor shall be left as a spare, when the circuit is either 480 volt or a neutral is required, respectively.

Terminate all spare conductors on terminal blocks.

**Article 11.2 Measurement**

Work performed under this article is considered incidental to other Work.

Removal and disposal of abandoned conductors is not measured for payment and is incidental to other Work. All splices, pull wire-string, cable tags, connectors, and fused disconnects are also considered incidental and no payment shall be made.

The Traffic Signal Electronics personnel will test and perform tie-down for all traffic loop detectors. The Contractor shall prepare the lead-in cables for tie-down, including labeling, insulation stripping and fitting with termination connectors.

When an existing active signal system is being modified, the Traffic Signal Electronics personnel will terminate all control cables within the traffic signal controller cabinet. It shall be the Contractor’s responsibility to prepare the cables for termination.

The Traffic Signal Electronics personnel will splice, test and perform tie-down on all interconnect wiring operations.

**Article 11.3 Basis of Payment**

No separate payment is allowed for this item.
SECTION 80.12  FUSED SPLICE CONNECTORS

Article 12.1  General

A fused, quick disconnect, splice connector shall be installed between the line and luminaire ballast tap conductors in the base of every pole equipped with a luminaire.

The connector shall be weather tight and consist of two halves: a single unit line side socket and load side plug. The plug and socket assembly shall be designed so that the fuse remains in the load side plug without exposing live metal parts when the connector separates. Coil springs shall not be a part of the current carrying circuit.

Contractor shall provide fuses that are ten (10) ampere, midget (13/32" x 1 1/2") ferrule type with a fast acting current limiting (KTK type) design.

The Contractor shall install the fused connectors so they are readily accessible from the handhole. Tap conductors shall be installed so there is no slack when their ends touch the top of the foundation.

Article 12.2  Measurement

Work performed under this article is considered incidental to other Work.

Article 12.3  Basis of Payment

No separate payment is allowed for this item.
SECTION 80.13 BONDING AND GROUNDING

Article 13.1 General

Metallic cable sheaths, metal conduit, non-metallic conduit grounding wire, ballast and transformer cases, service equipment, sign switches, metal poles and pedestals shall be made mechanically and electrically secure to form a continuous system, and shall be grounded. Bonding and grounding jumpers shall be copper wire or copper braid of the same cross-sectional area as No. 8 AWG for all systems.

Bonding of slip-base type standards and pedestals shall be by means of two conductors from the conduit, one attached with a ground rod clamp to an anchor bolt and the other connected to the lower portion of the shaft. Bonding of standards with frangible coupling type bases shall be made by attaching one conductor from the conduit to the lower portion of the shaft. The attaching bolt shall be weather resistant and be a minimum of three-sixteenth inches (3/16”) in size. The conductor for the shaft shall be forty-eight inches (48”) long.

One side of the secondary circuit of step-down transformers shall be grounded.

Grounding of metal conduit, service equipment and neutral conductor at service point shall be accomplished as required by the Code and the serving utility, except that grounding electrode conductor shall be No. 6 AWG, or equal.

Unless otherwise sized on the Drawings, Contractor shall install a bare #8 AWG copper wire in all non-metallic and metallic type conduits for bonding purposes. When wire is pulled into or out of existing conduit and the conduit does not have an existing bare #8 AWG copper wire, Contractor shall install the ground wire.

Contractor shall bond metallic conduit by means of threaded malleable iron insulated throat grounding bushings and bonding jumpers. Contractor shall use copper lugs that are wet location rated. Contractor shall clearly identify lugs rated for more than one conductor.

Contractor shall splice grounding conductors with irreversible compression type connectors listed for the purpose.

Contractor shall install grounding bushings on all metallic conduits. All non-metallic conduits, except for detector loop home runs, shall have a bushing installed. Contractor shall allow for bushings when installing conduits in foundations.

Contractor shall replace all missing or damaged conduit bonding jumpers and junction box lid braided bonding jumpers.

Contractor shall provide a minimum #10 AWG green grounding insulated conductor in the pole shaft of all poles with luminaires, and shall terminate the conductor in the lighting fixture.
Bond junction box lids to the grounding conductor using copper braid with a cross sectional area equal to an 8 AWG conductor and eyelets spaced at six inch (6”) intervals.

An integral bare ground shall not be used in any cable.

Contractor shall ensure that the grounding conductor, between all ground rods, is continuous or spliced with irreversible ground rated splices.

Contractor shall install a three-quarter inch by ten foot (3/4” x 10’) copper clad steel ground rod in the foundation space of a two-piece vault style traffic signal controller foundation. If two-piece vault style controller foundation isn’t being installed, then install ground rod within Type 3 junction box adjacent to controller cabinet base.

Contractor shall use only stainless steel bolt assembly components to attach bonding braid to the cover (lid).

Furnishing and installing bonding and grounding conductors for electrical installations is incidental to this Contract and no additional payment is made.

**Article 13.2 Measurement**

Work performed under this article is considered incidental to other Work.

**Article 13.3 Basis of Payment**

No separate payment is allowed for this item.
SECTION 80.14 LOAD CENTERS

Article 14.1 General

When the positioning of the load center is not detailed on the Drawings, the location shown is approximate and the Contractor shall determine the exact location from the Engineer or the serving utility.

Where Contractor is required to install the service on a utility-owned pole, the positioning of the riser and service equipment is determined by the serving utility.

The serving utility shall approve load center meters, complete with manual circuit closing device and sealing rings. Contractor shall not mount meter sockets on doors.

All accessible sections containing non-metered conductors shall have sealing provisions that will accept Brooks Type 623 seal (0.047 stainless bail).

Contractor shall ensure that the load center is located ten to fifteen feet (10' to 15') from the power source, with a two inch (2") minimum conduit stubbed to within two feet (2') of the power source, and at a minimum depth of forty-two inches (42"). The conduit shall contain a pull-rope, and the end capped and marked with a two by six inch (2" x 6") board. Contractor shall coordinate exact location with the serving utility.

Contractor shall stub service conduit through base as shown on the Drawings.

Prior to the load center being energized by the serving utility, Contractor shall arrange to have it inspected and approved by a Municipal Electrical Inspector. The certificate of electrical inspection, attached to the load center, indicates approval.

At all new and existing load centers, which require modification, the Contractor shall furnish conduit, conductors, contactors, breakers, transformers, and all necessary materials to complete the installation of the service, and upgrade to current code requirements.

Contractor shall label the load center as a unit by an Approved Independent Electrical Testing Laboratory (such as UL, ETL, CSA, etc.) defined by ANSI Standard Publication Z34.1 "Third Party Certification Programs for Products, Processes and Services" and conform to applicable published standards noted herein, the Drawings, and Special Provisions. Contractor shall label the load center as service entrance equipment. All Work shall conform to the latest edition of the National Electric Code as last amended and adopted by the Municipality of Anchorage.

All lighting load centers shall contain a multi-pole, 3-position control switch to provide selection of photocell operation. Contractor shall label switch positions "Auto," "Off" and "On." In the "Off" and "On" positions of the switch, Contractor shall ensure all leads to the photo control device are de-energized. Contractor shall install the switch inside the load center, accessible only through one of the lockable doors.
Contractor shall provide UL-approved and listed circuit breakers. Contractor shall provide an enclosed operating mechanism that is:

1. trip-free from operating handle on overload
2. trip-indicating
3. plainly marked with trip and frame size.

Multiple-pole circuit breakers shall have a common trip. Contractor shall ensure that all circuit breakers are quick-make, quick-break on either automatic or manual operation, and shall meet the requirements of the serving utility. Contractor shall ensure that the contacts are silver alloy enclosed in an arc-quenching chamber. An ambient temperature range of from -40° to +160° Fahrenheit shall not influence overload tripping of breakers.

The contactors shall have contacts rated to switch thirty (30) or sixty (60) AMP inductive loads as the Drawings specify, and are normally open. Contractor shall provide mechanical armature type contactors consisting of an operating coil, a laminated core, a laminated armature, contacts, and terminals with contacts made of fine silver, silver alloy, or superior alternative materials and rated for 480V.

Contractor shall provide the lighting contactor coil(s) rated for operation at 240 VAC.

Contractor shall connect ground bus of load center to ground rod(s) with #6 soft drawn bare copper and approved connectors.

Dimensions given are typical. Slight variations are allowable, subject to Engineer's approval.

Contractor shall submit four (4) copies of manufacturer's shop drawings for Engineer approval.

Contractor shall indicate the interrupting rating on panel schedules for each location.

On panel schedules for each location, Contractor shall indicate service rating of 120/240V, 3 wire; 240/480V, 3 wire; 100 AMP or 200 AMP.

Contractor shall provide a typed circuit directory for each load panel inside of the load center door, protected with a plastic cover, describing each circuit, with even and odd numbered circuit breaker positions shown on separate parts of the directory. Contractor shall provide a power and control 1-line diagram protected by a laminated plastic cover inside the load center. Contractor shall include the following information on the directory and one-line diagram: Load Center Identification (A, B, etc.), Project Name, Municipal Project Number and Service Voltage.

Contractor shall ensure that the wiring configuration conforms to the appropriate electrical diagram, and as the panel schedule indicates for each intersection. Contractor shall
complete a load center summary per appropriate detail drawing for each load center location.

Contractor shall ensure that all terminals are suitable for AL/CU termination, sized in accordance with ampere ratings.

Contractor shall provide #12 AWG XHHW as the load center control wiring.

Contractor shall ensure that the utility section is isolated from main load section and the distribution load sections by non-removable metal barriers, and equipped with landing lugs for utility termination.

The meter section shall contain a meter safety socket with safety shield and provisions for manual bypass of the meter. Contractor shall provide a link or lever type bypass with no external screws, bolts, or nuts. Horn and sliding types are not acceptable.

External screws, bolts, and nuts are not acceptable.

Contractor shall provide exterior nameplate, safety labels, interior identification labels, wiring diagram, and installation instructions with the pad-mounted load centers.

Contractor shall label in a prominent manner all switches and circuit breakers for circuit and direction.

Contractor shall ensure the lighting contactor coil is rated for operation at 240 VAC.

Contractor shall install load centers having 30 milliamp (ma) ground fault circuit breakers with ratings for all heat trace circuits as indicated on the Drawings.

**Article 14.2 Illumination Control**

Contractor shall install photoelectric controls capable of switching multiple lighting systems directly.

The photoelectric control shall consist of a photoelectric unit that shall cause a contactor to be energized, thus controlling the lighting circuit. Contractor shall install photoelectric units on the load center, unless the Engineer requires pole mounting of the photoelectric unit because a load center mounted unit will not work properly due to ambient light sources. If required, Contractor shall provide photoelectric units for pole top mounting with a slip fitter, terminal block and with cable supports or clamps to support pole wires. There will be no separate payment for providing the required photoelectric units.

Photoelectric Unit:

1. The photoelectric unit shall consist of a light sensitive element connected directly to a normally closed, single pole throw control relay without intermediate amplifications.
2. The unit is either the horizontal sensing or zenith sensing type and shall conform to the following:
   a. The supply voltage rated is 60 hertz (Hz), 105-277 volts.
   b. The maximum rated load is a minimum of 1,800 volt-amperes.
   c. The operating temperature range is from -40°F. to +150°F.
   d. The power consumption is less than 10 watts.
   e. The base of the unit has a 3-prong, EEI-NEMA standard, twist-lock plug mounting.

3. Units for highway lighting shall have a "turn-on" between one (1) and five (5) foot candles and a "turn-off" at between one and one-half and five (1½ and 5) times "turn-on."

4. Contractor shall ensure measurements conform to the procedures set forth in EEI-NEMA Standards for Physical and Electrical Interchangeability of Light-Sensitive Control Devices Used in the Control of Roadway Lighting.

5. The photoelectric control unit shall plug into a phenolic resin twist lock receptacle, adjusted to north sky set in a cast aluminum-mounting bracket with a threaded base. When installed on the load center, Contractor shall ensure the bracket is coupled to the end of a rigid metal conduit. When installed on the top of steel poles, Contractor shall ensure the bracket is installed in the center of the rain cap, secured with a locknut and made watertight with a fillet of silicone caulking compound. When installed inside the load center, Contractor shall ensure the installation conforms to the manufacturer's recommendations and that all load center penetrations/openings are silicon sealed.

6. Contractor shall screen photoelectric units to prevent artificial light from causing cycling.

The load center shall contain a 2-pole, 3-position on/off auto switch. In the "on" and "off" positions, Contractor shall ensure the switch interrupts all hot leads to the photocell.

**Article 14.3 Step Up/Step Down Transformer**

Step up/step down transformers in 480 volt circuits shall be 240-120 volt, 60 Hz type with volt-ampere ratings as shown on the Drawings. Transformers shall carry rated volt-amperes continuously without exceeding 85°C temperature rise above 25°C ambient.

Where installed outside of the load center, use a non-ventilated transformer fabricated of aluminum, stainless steel or galvanized steel. Coat enclosures fabricated of sheet metal with moisture resistant paint.
The case shall be fabricated of aluminum, brass, or galvanized steel. The case shall be coated with moisture resistant paint.

The unit shall be filled with a high melting point insulating compound and shall be hermetically sealed to insure satisfactory operation under continuous submersion in water.

Transformer leads shall be insulated with non-hygroscopic material and shall extend at least nine inches (9”) outside the case seal.

The primary and secondary sides of the transformer shall be "protected" in the load center.

**Article 14.4 Pad-Mounted Load Center**

All doors shall be equipped with continuous stainless steel pin hinges, coin latches, and hasp for padlock.

Meter section door shall have a clear lexan meter reading window, 0.187” minimum thickness, with a minimum size of eight by eight inches (8.0” x 8.0”), and shall include a silicon seal to door.

The load center shall be provided with internal mounting facilities for a one-half inch (1/2”) anchor bolt installation as well as for use with a standard factory mounting base assembly.

Construction shall be of zinc-coated A60 finish steel with minimum thickness as follows:

- Exterior Shell - 12 GA.
- Interior Doors - 14 GA.
- Interior Panels - 14 GA.
- Interior Covers - 16 GA.

The load center shall be painted with a two-part urethane paint undercoating inside and out. The final finish shall be a two-part urethane paint, standard white for removable panels and non-gloss silver-gray, Benjamin Moore GN-42, for the enclosure.

The required location for the hand-off-auto switch and the contactor is in the distribution load section.

All non-current carrying parts shall be bonded to ground.

**Article 14.5 Post-Mounted Load Center, Type 2 - Underground Service**

A post-mounted load center, Type 2, shall be defined in the Construction Drawings by reference to appropriate Standard Details for the load center, wiring diagram, and panel schedule.
Article 14.6 Post-Mounted Load Center, Type 3 - Overhead Service

A post-mounted load center with overhead service, Type 3, shall be defined in the Construction Drawings by reference to appropriate Standard Details for the load center, wiring diagram, and panel schedule.

Article 14.7 Single-Meter Pad-Mount Load Center, Type 1 & 1A

A single-meter pad-mount load center, Type 1A, shall be similar to Circle AW CMP-4111MN mounted on MB-1514 base or equivalent approved by the Municipal Traffic Engineer. It shall be defined in the Construction Drawings by reference to appropriate Standard Details for the load center, wiring diagram, and panel schedule.

A single-meter pad-mount load center, Type 1, shall be similar to Circle AW CMP-4900 series mounted on MB-2820 base or equivalent approved by the Municipal Traffic Engineer. It shall be defined in the Construction Drawings by reference to appropriate Standard Details for the load center, wiring diagram, and panel schedule.

Article 14.8 Measurement

Load centers shall be measured as units, complete and in place. Bases for pad-mounted load centers shall be a separate bid item under "foundations."

Photoelectric units mounted on the load center shall not be measured separately for payment. The Work performed under Article 14.2 – Illumination Control, is considered incidental to Work performed under Articles 14.5 through 14.7, unless a pole mounted photoelectric unit is required.
**Article 14.9 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Separate payment will be allowed for pad-mount bases.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Post-Mounted Load Center Overhead Service, Type 3</td>
<td>Each</td>
</tr>
<tr>
<td>Post-Mounted Load Center Underground Service, Type 2</td>
<td>Each</td>
</tr>
<tr>
<td>Single-Meter Pad-Mount Load Center, Type 1</td>
<td>Each</td>
</tr>
<tr>
<td>Single-Meter Pad-Mount Load Center, Type 1A</td>
<td>Each</td>
</tr>
<tr>
<td>Pole-Mounted Photocell Installation, Complete</td>
<td>Each</td>
</tr>
<tr>
<td>120/240 - 240/480, Transformer</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.15 WOOD POLES

Article 15.1 General

Wood poles for service or temporary installations shall be of the class shown on the Drawings. Wood poles used in temporary installations shall meet or exceed ANSI class 4 for poles used for temporary illumination only, and ANSI class 1 for poles used for temporary signalization.

Poles shall not have more than 180 degrees twist in grain over the full length. Sweep shall be no more than four inches (4”). Poles shall be placed in the ground to a depth of at least six feet (6’). The lengths of poles shall be twenty-five feet (25’) for service poles and thirty-five (35’) feet for other poles, unless otherwise specified.

After each wood pole is set in the ground, the space around the pole shall be backfilled with selected earth or sand, free of rocks four inches (4”) or larger, or deleterious material, placed in layers approximately four inches (4”) thick and thoroughly compacted with mechanical tampers.

Mast arms and tie rods for wood pole installations shall conform to the provisions of Section 80.05 – Poles, Steel Pedestals, and Posts, and to the details shown on the Drawings. Each mast arm shall be provided with an insulated wire inlet and wood pole mounting bracket for mast arm and tie rod cross arm.

Overhead equipment shall provide a minimum vertical clearance of eighteen feet (18’) from bottom of equipment to the pavement.

Wood poles, not to be painted, shall be pressure treated after fabrication with creosote, pentachlorophenol (oil borne), or copper naphthenate (oil borne) in accordance with the latest applicable standards of the American Wood Preservers Association. Where it is impractical to obtain the specified retentions because of the character of the wood in the charge, the treatment shall be to refusal. The retentions may be determined either by gauge or scale readings or by assay. Treated poles shall be coated in conformance with current EPA regulations.

Wood poles shall not be used for permanent installations.

Article 15.2 Measurement

Wood poles used for temporary support of signals, signs and illumination shall be measured as temporary wood pole structures installed and removed.
**Article 15.3 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Temporary Wood Pole Structures</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.16  MISCELLANEOUS

Article 16.1  Sign Switches

Each sign illumination installation shall be provided with a disconnect switch mounted on the sign standard or structure, as shown on the Drawings. Where the sign lighting is served from a multiple service, each sign structure shall have a 120-volt, 240-volt, or 480-volt circuit breaker, approved by UL as service equipment, and rated as shown on the Drawings.

Enclosures for the sign breaker shall be galvanized or baked enamel NEMA Type 3R, and shall be provided with top hinged cover, hasp for sealing cover and provisions for locking.

Article 16.2  Field Tests

Prior to acceptance of the Work, the Contractor shall perform the following tests on all traffic signal, sign illumination, and lighting circuits, in the presence of the Engineer.

A. Tests

Any fault in any material or in any part of the installation revealed by these tests shall be replaced or repaired by the Contractor at his expense in an approved manner, and the same test shall be repeated until no fault appears.

1. Continuity

Each circuit shall be tested for continuity.

2. Grounds

The test for grounds in each circuit shall consist of the physical examination of the installation to insure that all required grounding bushings, bonding jumpers, and ground rods have been installed and are mechanically firm.

3. Insulation

A megohm test shall be made on each circuit, between circuits and between the circuit and a ground. The insulation resistance shall not be less than 100 megohms or the minimum specified by the manufacturer, measured at 500 volts direct current (VDC). All lamps and magnetometer sensing probes shall be disconnected prior to performing the megger test.

4. Circuit

Every signal indication circuit shall be energized with lamps installed prior to installation of the load switches.
5. **Functional**

The following tests shall be performed on each signal and lighting system after all of the component circuits have satisfactorily passed the tests for continuity, grounding, insulation integrity and circuitry.

**B. Functional Testing**

During the test periods, the Contractor will maintain the system or systems. The cost of any maintenance necessary, except electrical energy, shall be at the Contractor's expense.

1. The functional test for each new or modified traffic signal system shall consist of not less than twenty-four (24) hours nor more than five (5) days of continuous flashing operation.

2. During the functional tests, signals shall not be switched from flashing operation to normal, continuous operation on a Saturday, Sunday, Monday, a Holiday, or the day after a Holiday.

4. The functional test for each highway lighting system and sign illumination system shall consist of an operational test for five (5) consecutive nights according to the regular lighting schedule.

5. The functional test for each flashing beacon system shall consist of not less than five (5) days of continuous, satisfactory operation.

6. A continuous five (5) day burning test shall be made on each pedestrian overcrossing and undercrossing lighting system before final acceptance.

The initial turn-on shall be made between 9:00 a.m. and 2:00 p.m. unless specified otherwise in the Special Provisions. Prior to turn-on, all equipment shown on the Drawings shall be installed and operable. This includes, but is not limited to, pedestrian signals and push buttons, signal face backplates and visors, vehicle detectors, highway lighting and all regulatory, warning and guide signs. All signal faces shall be aimed as required by Sections 80.19 – Signal Heads and 80.20 – Pedestrian Signals.

**Article 16.3 Galvanizing**

Standards, pedestals, posts and cabinets of ferrous materials shall be galvanized in accordance with the provisions of ASTM A123 except that cabinets and cut out boxes may be constructed of material galvanized prior to fabrication.

Iron or steel pipe standards and mast arms shall be hot-dip galvanized after fabrication in conformance with the ASTM A123.
Tie-rods, nuts, washers, clamps, anchor bolts and other miscellaneous ferrous parts shall be hot-dip galvanized after fabrication in accordance with the provisions of ASTM A153. Anchor bolts shall be fully galvanized.

After galvanizing, the bolt threads shall accept galvanized standard nuts without requiring tools or causing removal of protective coatings.

Rigid metal conduit shall be hot dip galvanized in accordance with American National Standards Institute specification ANSI C-80.1.

Galvanized coatings that have been cut or damaged shall be repaired in conformance with ASTM A780.

**Article 16.4 Painting for Steel Structures**

A. General Requirements. Ship paint in strong, substantial containers, plainly marked with the name, weight, and volume of the paint content, together with the color formula, batch number, and the name and address of the manufacturer.

Reduction and clean-up thinners will be as approved by the coating manufacturer. Ship all thinners in the manufacturer’s original containers.

B. The paint shall conform to the requirements outlined below:

1. **Prime Coat.** A generic single component, moisture cure, polyurethane (SC-MC-U) containing not less than 78% by weight zinc powder. Volume of solids shall be 60% minimum. Pigment type shall be zinc dust. Zinc in dry film shall be 83% minimum, by weight. Weight per liter shall be 2750 g, minimum. Volatile organic compounds (VOC’s) shall not exceed 450 g/L.

2. **Intermediate Coat.** A SC-MC-U containing not less than 480 g/L micaceous iron oxide (MIO). Volume of solids shall be 60% minimum. Pigment color shall contrast between the intermediate and prime coat and the intermediate and top coat. Weight per liter shall be 1550 g minimum. VOC’s shall not exceed 450 g/L.

3. **Top Coat.** A single component, moisture cure, aliphatic polyurethane (SC-MC-ALIP-U), containing not less than 480 g/l micaceous iron oxide (MIO). Volume of solids shall be 60% minimum. Pigment color of the top coat shall be FSS FED-STD-595B, color number 26492. The color match shall be evaluated as a general match under a daylight source using ASTM D 1729. Weight per liter shall be 1550 g minimum. VOC’s shall be less than 450 g/L.

3. All coatings shall pass the following tests:

   a. **Corrosion Resistance, ASTM B 117, Salt Spray Test.** Minimum of 4000 hours with less than 2 mm creep from scribe. Panels shall be 3
mm minimum thickness cold rolled steel, having SSPC Near White Blast with 25 to 50 μm angular profile.

b. **Accelerated Weathering, ASTM G 53.** Minimum 400 hours QUV B bulb with no chalking, cracking, or gloss loss greater than 20%.

c. **Forward Impact, ASTM D 2794.** Minimum 17 Nm impact.

d. **Abrasion Resistance, ASTM D 4060.** Less than 90 mm loss on CS-17 wheel, 1000 g/load, 1000 cycles.

e. **Moisture Resistance, ASTM D 4585.** Minimum 1000 hours at 38° C with no change in appearance.

f. **Flexibility, ASTM D 522, Cylindrical Mandrel Bend Test.** Bend around 12.5 mm diameter mandrel with no cracking.

g. **Adhesion, ASTM D 4541.** Minimum 3.5 Mpa on a certified pull test.

C. **New Equipment.**

1. Signal heads, signal head mountings, brackets and fittings, outside of visors, pedestrian push button housings, pedestrian signal head housings and visors, and back faces of backplates, shall be factory finished with two (2) coats of dark olive green enamel. Painting is not required where the color is an integral part of the component material, or powder coated.

2. Interior of signal visors, louvers, and front faces of backplates shall be factory finished with two (2) coats of lusterless black enamel.

3. After erection, all exterior surfaces shall be examined for damage, and such damaged surfaces shall be cleaned and spot coated with primer and finish coat.

4. Two factory finishing coats of aluminum paint shall be applied to controller cabinets.

5. Controller cabinet shall be painted white inside and silver-gray outside, with under coating inside and out.

D. **Reused Equipment.**

1. Existing non-galvanized, damaged equipment shall be painted in the field, including Owner-furnished equipment. The equipment shall be washed with a stiff bristle brush using a solution of water containing two tablespoons (2 tbsp/gal) of heavy duty detergent powder per gallon. After rinsing, all surfaces shall be wire brushed to remove all poorly bonded paint, rust, scale,
corrosion, grease or dirt. Any dust or residue remaining after wire brushing shall also be removed prior to priming.

2. Factory or shop cleaning methods for metals will be acceptable if equal to the methods specified herein.

3. Immediately after cleaning, all bare metal shall be coated with Pre-Treatment, Vinyl Wash Primer, followed by two (2) prime coats of Zinc Chromate Primer for Metal.

4. Signal equipment, excluding standards, shall be given a spot finishing coat on newly primed areas, followed by one (1) finishing coat over the entire surface.

5. Ungalvanized standards shall be given two (2) spot finish coats on newly-primed areas.

6. All paint coats may be applied either by hand brushing or by approved spraying machines. The Work shall be done in a neat and workmanlike manner. The Engineer reserves the right to require the use of brushes for the application of paint, should the Work done by the paint spraying machine prove unsatisfactory or objectionable.

Galvanized equipment with rusted areas shall be repaired as provided for in Article 16.3 - Galvanizing.

**Article 16.5 Measurement**

The Work performed under this section is considered incidental to other Work.

**Article 16.6 Basis of Payment**

No separate payment is allowed for Work performed under this Section.
SECTION 80.17  CONTROLLER ASSEMBLIES

Article 17.1  General

Each solid-state, traffic controller assembly shall operate various traffic signal devices as shown on the Drawings, to provide right-of-way, clearance and other indications with duration and sequence as determined by preset programming.

Details of operation for the complete controller assembly shall be in accordance with the traffic phases, preferential phase sequence and concurrence, signal indications, signal indication sequence, detection requirements and other details shown on the Drawings or as specified herein.

All controller assemblies shall conform to the requirements of NEMA STANDARDS PUBLICATION/No. TS-1-1989 unless otherwise specified. All Load Switches, Flashers and Flash Transfer Relays shall conform to the requirements of NEMA STANDARDS PUBLICATION/No. TS-2-1998.

Indicators for all electronic devices covered under this Section shall be of the non-incandescent type.

All controllers must be compatible with the existing Anchorage traffic control systems and contain necessary internal communication modems. In the Central Business District (CBD), six-phase dual ring NEMA controllers will be used. Outside the CBD, fully actuated eight-phase dual ring NEMA controllers will be used. Separate controller phases for each movement provide timing flexibility although the displayed phases to the driver may appear to be less than eight.

Article 17.2  Controller Unit

Controller units shall conform to NEMA STANDARDS for microprocessor-based, solid state, digital timed, eight-phase traffic actuated signal controller providing up to eight phases of signal control, internal preemption, time base coordination, internal time-of-day programming, and data base management by an IBM PC.

The signal controller shall be constructed in accordance with the requirements of NEMA Publication No. TS1-1989, unless otherwise noted. The Contractor shall submit documentation with his bid detailing the brand and model number of the signal controller proposed to be furnished. This documentation shall include catalog cuts, operational details and NEMA certification reports. The agency reserves the right to reject any and all equipment that does not meet these Specifications.

Since controller interchangeability at the connector level is foremost in these specifications, the NEMA pin assignments for connectors A, B, and C shall not be modified or changed in any manner. External features beyond NEMA shall be provided on a fourth connector; PROVIDED, however, that all logic signals shall be low state (nominal 0 volts)
for the True (operate) state. Each pin in every connecting cable shall be terminated on a separate terminal and be accessible for troubleshooting or employment of special features.

The traffic signal controller shall be solid state, modular by function, microprocessor based, eight-phase device. Each NEMA connector (A, B, and C) shall be mounted on individual modules and shall be configured so that it is possible to remove them to reconfigure the eight-phase unit into a two-, four- or six-phase controller. No other modifications to the controller shall be required to reconfigure the controller as a two- or four-phase unit.

The controller shall be capable of being configured for either single or dual ring operation with phases occurring in any order in either ring. Phases in the same timing ring shall not time concurrently. Phases not in the same timing ring may time concurrently, provided they are on the same side of the barrier or compatibility line.

All cabinet control equipment shall be designed to operate under the following physical conditions:

- An ambient temperature range of -30° to +165° Fahrenheit.
- A relative humidity range of 5%-95% (without condensation).
- A voltage range of 95 to 135 VAC RMS with a corresponding frequency range of 57-63 HZ.

The design life of all components shall not be less than five (5) years of continuous twenty-four (24) hour operation.

All printed circuit boards shall be conformal coated after assembly.

The controller shall have all sixteen (16) or more pin integrated circuit components socket mounted.

The design of the controller shall incorporate a motherboard design with all functional modules plugging into the motherboard assembly. All modules shall be equipped with connectors which prevent the module being inserted upside down. The motherboard shall be completely removable without desoldering or disconnecting individual wires.

All printed circuit modules shall be aligned with printed circuit card guides and held firmly in place by a captive screw or other locking device when installed in the controller.

The entire controller unit shall be designed so as to permit complete disassembly of the unit. No welds, pop rivets, or other permanent fasteners may be employed which would preclude complete disassembly.

The controller power supply shall be removable as a unit and shall be plug connected. Removal shall not require the disconnection of individual wires.
The design of the controller shall incorporate the use of non-volatile electrically erasable read-only memory (EEPROM) for the storage of timing and operating parameters. The controller data base shall not require battery and the only allowed use of a battery is to power the backup clock during power outages.

All components on the module circuit boards shall be easily accessible and shall be arranged in a functional grouping. Printed circuit boards shall have the following minimum quality requirements:

1. Boards shall be NEMA Grade A, G10 Glass Cloth Base Epoxy Resin with a minimum thickness of one-sixteenth inch (1/16”).
2. Intercomponents wiring shall be copper track with a minimum weight of two ounces per square foot and have adequate cross-section of the current being carried by each circuit. All components mounting holes and circuit pass-through holes shall be plated through.
3. Printed circuit design shall be such that components may be removed and repaired without permanent damage to the printed circuit board when proper soldering techniques are used.
4. All components on each printed circuit board shall be permanently identified on the circuit side of the board.

The controller shall be completely enclosed in a protective case with a durable paint finish.

All front panel indicators on the controller shall have minimum design life of 5,000 hours at their normal operating voltage level. Front panel indicators of the controller shall be automatically extinguished after a one-hour period of non-use of the keyboard.

A. The following features shall be provided as part of the controller software, and as such shall be available for operator selection from the front panel keyboard:

1. Conditional service.
2. MUTCD flash.
3. Time-of-day clock.
4. Sequence control.
5. Programmable overlaps.
   a. Standard NEMA overlaps.
   b. Protected/permissive overlaps.
   c. Negative overlaps.
   d. Auxiliary timed overlaps.
6. Programmable start-up sequence.
7. Simultaneous gap-out.
8. Internal coordination including:
   a. Time base coordination.
   b. External system coordination.
   c. "ON LINE" command from central computer.
   d. Designated master output functions.
   e. Critical intersection card (DSA).
9. Full or semi-actuated coordination.
10. Dual entry by phase.
11. Soft recall selectable by phase.
13. Programmable dimming by phase and color.
15. Internal emergency vehicle preemption:
   a. High priority vehicle.
   b. Low priority vehicle.

B. It shall be possible to observe and adjust the controller operation via the front panel while the intersection is in full operation and without placing the intersection in flash. The front panel keyboard, through which all instructions to controller memory shall pass, shall be of "scratch pad" type, making it necessary to depress an ENTER key before any stored programs are altered. A display shall be provided, showing that the data has been edited, validated, and accepted into operating memory. Additional displays shall be provided for all vehicle and pedestrian calls such that they may be viewed simultaneously with displays for phase timing intervals of both timing rings of the controller. Phase timing interval displays shall show a countdown time remaining in the active interval of each ring simultaneously with phase data. Displays provided for phase configuration and timing data shall be alphanumeric displays using mnemonic designations readily understood by traffic engineering personnel.

The controller shall provide the standard NEMA eight phase dual ring sequence and, in addition, allow the selection of the following alternate sequences:

1. Quad sequential.
2. 8-phase sequential.
3. Exclusive phase 1 or exclusive phase 2.
4. Dual 4-phase without barrier restraints.
5. Phases 1 & 6 and 3 & 8 do not time concurrently.

Each phase shall be separately programmable to provide the following functions:

1. Locking and non-locking vehicle detector memory.
2. Vehicle recall to minimum green.
3. Soft recall to minimum green (selected phases recalled only in the absence of other serviceable calls).
4. Vehicle recall to maximum green.
5. Pedestrian recall.

The time interval setting per phase shall provide, as a minimum, the following increments and ranges, each in seconds:

<table>
<thead>
<tr>
<th>FUNCTION</th>
<th>RANGE</th>
<th>INCREMENT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Minimum Green (initial)</td>
<td>0-255 seconds</td>
<td>1 second</td>
</tr>
<tr>
<td>Walk</td>
<td>0-255 seconds</td>
<td>1 second</td>
</tr>
<tr>
<td>Pedestrian Clearance</td>
<td>0-255 seconds</td>
<td>1 second</td>
</tr>
<tr>
<td>Passage Time (Gap)</td>
<td>0-25.5 seconds</td>
<td>0.1 second</td>
</tr>
<tr>
<td>Maximum Green #1</td>
<td>0-255 seconds</td>
<td>1 second</td>
</tr>
<tr>
<td>Maximum Green #2</td>
<td>0-255 seconds</td>
<td>1 second</td>
</tr>
<tr>
<td>Yellow Clearance</td>
<td>3-25.5 seconds</td>
<td>0.1 second</td>
</tr>
<tr>
<td>All Red Clearance</td>
<td>0-25.5 seconds</td>
<td>0.1 second</td>
</tr>
<tr>
<td>Red Revert</td>
<td>2-25.5 seconds</td>
<td>0.1 second</td>
</tr>
<tr>
<td>Actuations B4 Added Initial</td>
<td>0-255 actuation</td>
<td>1 actuation</td>
</tr>
<tr>
<td>Seconds Per Actuation</td>
<td>0-9.9 second/actuation</td>
<td>0.1 second</td>
</tr>
<tr>
<td>Time Before Reduction</td>
<td>0-255 seconds</td>
<td>1 second</td>
</tr>
<tr>
<td>Time to Reduce</td>
<td>1-60 seconds</td>
<td>1 second</td>
</tr>
<tr>
<td>Minimum Gap</td>
<td>0-8.0 seconds</td>
<td>0.1 second</td>
</tr>
<tr>
<td>Conditional Minimum Green</td>
<td>0-255 seconds</td>
<td>1 second</td>
</tr>
</tbody>
</table>

The front panel displays for each ring shall include, as a minimum, the following:

1. Phase(s) timing.
2. Interval(s) timing countdown.
3. Phase(s) next.
5. Pedestrian calls.

The controller shall be capable of resting in "all red" in any phase in the absence of vehicle or pedestrian calls. This feature shall be internal time-of-day selection.

The controller shall be equipped with a separate time interval setting for "Red Revert" which shall provide a minimum period that any phase must display red prior to returning to a green display of the same phase. The increments for this setting shall be one-tenth (1/10) second and the range shall be two to twenty-five (2-25) seconds selectable for each phase independently.

C. Conditional service, when enabled, causes the following controller operation: If two even numbered concurrent phases are timing and a call exists on the other side of the barrier, and one of the phases is prepared to terminate due to gap out or max time out, the ring containing the timed out phase shall revert to its preceding off numbered vehicle phase if:

1. A call exists on that odd numbered phase.
2. There is sufficient time remaining before the max time of the concurrent even numbered phase has elapsed.

The criteria for determining if sufficient time remains for conditional servicing is, if the clearance time plus the conditional service minimum green time is less than the time remaining on the max timer of the even numbered phase timing concurrently.

Conditional service applies to vehicle phases only. For phases with concurrent pedestrian service, the pedestrian display will remain Don’t Walk. Any pedestrian calls placed prior to reservice or during the reservice will be stored until the phase is serviced normally.

If a conditionally serviced phase has been reserviced, the ring containing that phase will not return to the even numbered phase but will remain in the conditionally serviced phase until the complimentary ring's even numbered phase has gapped out or reached its max time and the minimum conditional service time, then the controller shall revert to its normal phase sequence operation.

The gap times of the conditionally served phase does not extend the green time of that phase when it is being reserviced.

Conditional service shall be selectable manually from the front panel by use of the keyboard or by the internal time-of-day program.
D. The controller software shall be capable of providing MUTCD flash control. This control shall consist of selectable main street green phases and function as follows:

1. Upon the receipt of the flash command, the controller shall continue normal operation until the next All Red interval in the sequence.

2. At the conclusion of the All Red interval the intersection shall be placed in flashing operation and the controller held at the beginning of Main Street Green until the flash command is removed.

3. The controller program shall have a minimum flash time to protect against short flashing operation. This minimum flash time shall be programmable from 0 to 255 minutes in one minute increments.

4. The return from programmed flash shall be at the beginning of the selected Main Street Green intervals.

5. Programmed flash shall be capable by remote input to the controller or by the internal time-of-day program.

E. The controller software shall contain a complete time-of-day program supported by a real time clock. The real time clock shall use the 60 Hz power line frequency as a time base and, in the absence of line power, use a battery supported clock, to maintain accurate time during power failures of up to six months. The real time clock shall incorporate automatic leap year correction, and provide an optional automatic daylight savings time correction.

The time-of-day program shall provide for a minimum of 48 separate programs with each program capable of multiple use on individual days, combination of days, or one time events. Time-of-day commands shall be executed with a one minute resolution, and the program shall be capable of looking back and selecting the correct program following power outage or controller startup.

Each time-of-day program shall provide for control of the following functions:

1. Flash ON/OFF (MUTCD).
2. Omit All Red by phase.
3. Call to Semi-actuated operation.
4. Min Recall by phase.
5. Max Recall by phase.
6. Ped Recall by phase.
7. Conditional Service ON/OFF.
8. Phase rotation.
10. Coordinated or Free Operation.
11. Coordination Parameters (Cycle, Split, Offset).
12. Phase omit/Ped omit by phase.
13. Red Rest by phase.
14. Max II by phase.
15. Special Functions ON/OFF.
16. Enable detector sampling reports.
17. Enable dynamic split allocation (CIC operation).

F. The controller shall have a selectable feature commonly known as "Simultaneous Gap-Out", which requires both concurrently timed phases to reach a rest state (by having gapped out) simultaneously before they terminate their Green display and advance across a barrier. Selection of this option shall allow a phase to retime the gap interval from a Green Rest state upon vehicle actuation.

G. The controller shall provide for overlap programming by a NEMA Overlap Card, as required by the NEMA standards and, in addition, shall have the option of programming from the front panel keyboard.

Standard overlap programming shall allow the inclusion of any of the eight (8) NEMA phases and provide the proper control of the four (4) sets of NEMA Red, Yellow and Green overlap outputs.

In addition to standard overlaps, the controller shall provide additional overlap features as follows:

1. Protected/permissive overlaps.
2. Right turn overlap w/special sequencing.
3. Auxiliary timing for overlaps including:
   a. Auxiliary overlap Green (0 to 255 sec.).
   b. Auxiliary overlap Yellow (3 to 25.5 sec.).
   c. Auxiliary overlap Red clearance (0 to 25.5 sec.).

H. It shall be possible to change the NEMA phase sequencing by reversing the order of any two (2) sequential phases which are in the same ring and on the same side of the barrier. This phase-rotation feature shall be accessible through the special function module and the 4th connector assembly. They shall also be selectable by the internal time-of-day program or by the internal coordination program.
I. An internal software coordination package shall be included in the controller which shall have the following features:

1. Six cycle lengths (30 to 255 seconds in one-second steps).
2. Five offsets per cycle length (30-255 seconds in one-second steps).
3. Separate vehicle and pedestrian permissive periods for each phase. Permissive periods shall be automatically calculated by the controller using the split settings and the minimum vehicle and pedestrian timings for each phase.
4. Selectable semi-actuated or fully actuated coordinated operation.
5. Selectable free or coordinated operation.
6. An ON-LINE input from a remote master. The ON-LINE command from a remote central computer shall inhibit the internal coordination program and allow the controller to respond immediately to the central computer or remote master commands. Upon loss of on-line the controller shall revert to time-base coordination.

The coordination package shall be capable of short route transitioning to get in synch or change timing Drawings. Programmable shrinkage or expansion of each cycle length shall be able to be set from the front panel. During transition no clearance interval shall be changed nor any minimum green time violated.

The coordination shall work with the internal time-of-day program and provide time-of-day selection of the following operational parameters:

1. Coordinate/Free.
2. Semi Actuated Coordination.
3. Fully Actuated Coordination.
4. Cycle Length (1 to 6).
5. Split (1 to 3).
6. Offset (1 to 5).
7. Phase Sequence (0 to 15).
8. Coordinated Phases by cycle.

From the front panel it shall be possible to read and load all programmable coordinated functions. All coordinated displays shall be in alphanumeric displays using mnemonic designations readily understood by traffic engineering personnel, e.g., CYL = Cycle, SPL = Split, YLD = Yield.
With the controller running in the coordinated mode it shall be possible to use the front panel displays to observe the coordination program in effect. As a minimum, it shall be possible for the operator to observe the following functions on a continuous basis:

1. Cycle in effect.
2. Offset in effect.
3. Split in effect.
4. Type of coordination.
7. Force offs by phase.
8. Vehicle permissive period by phase.
9. Pedestrian permissive period by phase.
10. Synch pulse (System zero).
11. Synch pulse (Local zero).

The software shall make validity checks on all phase timing and coordination data being entered by the user before accepting the data into permanent memory, e.g., summation of splits per ring equal the cycle length or split value is greater than minimum phase timing.

If the validity checks detect data error, the operator shall be notified of the cause of the error by an indication on the front panel displays.

During coordination operation a time-of-day option shall be available to enable the dynamic split allocation (CIC control). This feature will automatically increase or decrease the split values, on a cycle by cycle basis, as indicated by traffic data obtained from the phase detectors.

The internal coordination software shall provide a front panel programmable method of adjusting the operational offset value by field observation during active operation.

J. It shall be possible to program one railroad priority control sequence and four completely independent priority vehicle sequences. Railroad preemption shall have priority over vehicle preemption. Each vehicle preemption shall consist of two states i.e. high priority and low priority. A high priority command shall terminate a low priority command. The internal priority control shall provide outputs for programmable displays to indicate priority operation.

K. The controller program shall contain an optional start-up all red interval. Upon completion of the powerup flash interval, or the application of the NEMA startup
command, the controller shall time a programmable ALL RED interval before beginning operation in the start-up phases. This interval shall be programmable from 0 to 255 seconds in one second increments.

L. The controller program shall provide for the selection of startup in selected phase(s) Green, Yellow, or ALL RED intervals.

M. A special function (4th connector) cable shall provide eight (8) auxiliary detector inputs. These eight (8) detectors may be used as traffic count stations, system sampling stations or assigned to the NEMA phases. They may also be used to obtain one minute average speed data of traffic over the detectors.

When assigned as a NEMA phase input, each detector shall have provision for the following operation:

1. Delay time before call (0-255 seconds in 1 second increments).
2. Extension time of call (0-25.5 seconds in 0.1 second increments).
3. Disconnect mode selection:
   - Type 1: Disconnect during parent phase Green after first call.
   - Type 2: Disconnect when unoccupied for extension time.
   - Type 3: Combination of Type 1 and Type 2.
   - Type 4: Detector switching; after termination of the protected turn phase, turn phase detectors are switched to extend the permissive concurrent thru phase.
   - Type 5: Detector switching; turn phase detectors call the concurrent thru phase until the left turn phase is active.

N. Intersection status and traffic count information shall be accumulated for 72 count periods detailing the volume and occupancy data from the eight (8) NEMA detector inputs and the eight (8) Auxiliary detector inputs. The accumulation period shall be adjustable from 1 to 255 minutes in one-minute increments, and may be enabled or disabled by the internal time-of-day program.

The report shall be accessed through the RS-232C port of the controller and may be printed using a serial dot-matrix printer or transmitted to a remote master for viewing on a CRT.
O. An event log shall be available, internal to the controller software which will record and log the exact minute specific operational events occur. The log will show the number of events, the first time the even occurred and the last time the event occurred. Keyboard entry shall be provided for the operator to view the log and to reset the log to "0" after inquiry. Provisions shall be made to log the following events:

1. Internal clock updates.
2. Communication error.
3. Conflict status.
4. Flash Status.
5. Diagnostic test failure.
7. Controller access.
8. Short power loss (less than 0.5 sec.).
9. Long power loss (greater than 0.5 sec.).

P. The controller shall be furnished with a RS-232C communication interface port to allow the remote access of the complete controller data base with a serial printer, portable programmer and/or system modem. A DB25 female connector shall be provided for this function. The entire data base, including traffic count information, coordination parameters, all preemption intervals and all NEMA timing intervals shall be accessible through the RS-232C communication port.

Q. The controller shall be capable of optional self-diagnostic tests while in the normal run operation. These automatic tests shall consist of diagnostics to check the CPU, RAM, EEPROM and PROM memory of the controller, shall be halted, the intersection placed in flashing operation using the NEMA voltage monitor output, and a failure message shall be displayed on the front panel.

Article 17.3 Standard Auxiliary Equipment

Load Switches, Flashers and Flash Transfer Relays shall conform to the requirements of NEMA Standards Publication No. TS-2-1998.

Conflict Monitors shall conform to the requirements of NEMA Standards Publication No. TS-1-1989, unless otherwise specified.

The following auxiliary equipment shall be furnished and installed:

A. Flasher: Flasher shall be NEMA Type III.

B. Load Switches: Load switches shall be supplied with an LED (light emitting diode) circuit indicator.
C. Flash Transfer Relay: Flash transfer relays shall be plastic encapsulated 30 amp, 120 volt. The relays shall transfer the signal light circuits from controller unit to the flasher.

D. Conflict Monitors:

1. Conflict monitors shall meet or exceed NEMA specifications. They shall be microprocessor based with NVRAM memory, switch selectable options, liquid crystal display and an RS-232 serial port that allows the monitor to download information through an external dial-up multi-port modem or to a personal computer using the Microsoft Windows NT operating system. Each monitored input and each fail mode shall be displayed as a separate indication.

Faults and histories shall be recorded in Non-Volatile Random Access Memory (NVRAM) and be viewable via the front panel display and retrievable via the RS-232 communications connector.

Internal logic shall be such that should an error occur all active inputs at the time of the error and the fail mode type shall be illuminated. The indications shall remain illuminated until the unit is manually reset.

2. The Contractor shall not install any device to the input of the unit that would inhibit the unit from sensing a high impedance load or loss of load.

3. The unit shall monitor each yellow signal display for a minimum display period of three (3) seconds. A yellow less than three (3) seconds shall energize a fault circuit and place the intersection in emergency flash.

4. The unit shall provide switch selectable options to allow for dual indication monitoring with a channel or phase. When selected, the monitor shall detect as a conflict the following combinations:
   a. Green or walk and yellow
   b. Green or walk or yellow and red.
   c. Either of 2 or 3 of the above

5. The unit shall have a switch selectable option allowing for the modification of the NEMA red failure error. When selected, the WALK input shall not be considered in the RED FAILURE logic.

6. The unit shall have an internal "watchdog" circuit to test continually for unit failure. In the event of unit failure, the unit shall cause the intersection to enter the flash mode of operation. Monitor failure must be indicated.

7. The unit shall test for proper operation of its memory devices. The memory test must occur at least at power-up and reset. If memory failure occurs, the
unit will cause the intersection to enter flash mode and indicate monitor failure.

E. Communications: Contractor shall provide and install one Actelis Intelligent Transportation Systems model ML688 with 4x10/100M copper Ethernet ports and a 100Base-FX optical SFP port or equivalent in each controller assembly. All equivalent substitutions shall be submitted to the Traffic Engineer for testing and approval prior to installation.

**Article 17.4 Special Auxiliary Equipment**

The following special auxiliary equipment shall be furnished and installed:

A. Inductive Loop Detectors. Inductive loop detectors shall conform to the following:

1. All "P" and "R" controller cabinets shall be wired to accommodate five 4-channel inductive loop detector units and two 2-channel inductive loop detector units, wired per the latest controller assembly wiring diagram available from the Traffic Signal Electronics Shop Foreman. All "M" or other sized cabinets shall be wired in accordance with the latest controller assembly wiring diagram available from the Traffic Signal Electronics Shop Foreman.

2. The detector unit shall be an electronic device capable of providing closure of an output circuit when a vehicle stands or passes over a loop or one (1) of several loops connected to the input circuitry of the sensor control unit. The output shall be electrically isolated from the loop and the electronic circuitry.

3. Performance Characteristics. The detector shall operate with the specified configurations of loops and leadwire, including series and parallel combinations, having a total inductance of not less than forty-five (45) microhenries, nor more than one thousand (1,000) microhenries. Each detector channel shall provide continued operation with a loop shorted to ground and place a continuous call in the event of an open loop or a failed power supply.

   a. The detector shall "rephase" (resume detection of vehicles on unused portions of the loop) after a vehicle has parked over the loop for a pre-selected time. This is usually referred to as "presence time." Presence time shall be selected by a front panel switch. For determining presence times, a test loop of 120 microhenry inductance, having a minimum area of ninety (90) square feet and a 750 foot lead-in shall be used. Number 14 PVC with nylon jacketed loop wire shall be used for the loop. Either No. 12 AWG or No. 18 AWG instrumentation cable shall be used for the lead-in. The detector shall be operated continuously on the test loop for a
minimum of ten (10) minutes before measuring presence time. With a loop and lead inductance shift of five percent (5%), long presence time shall be longer than ten (10) minutes, medium presence time shall be longer than three (3) minutes, and short presence time shall be less than one (1) minute. Short presence time shall have a pulse output of 100+50 milliseconds. Sensitivity shall not be affected by changing the presence time.

b. Sensitivity measurements shall be made on a loop configuration as defined in the preceding paragraph. The detector shall provide reliable detection of all vehicles when the inductance shift of the loop, caused by the vehicle's presence, is two-hundredth percent (0.02%) of the total inductance of the loop and leadlines. In the normal sensitivity mode, an inductance shift of one-tenth percent (0.1%) shall provide reliable detection. Count accuracy shall be better than ninety-nine percent (99%) of all vehicles passing over the loops at speeds of zero miles per hour (0 mph) to speeds greater than eighty miles per hour (80 mph).

c. There shall be no false detections when the detector is subjected to power line transients in accordance with NEMA Standard TS-1. Sections 2, 7 & 11 Test Procedures. The detector shall withstand the discharge of a two (2 MFD) capacitor charged to one thousand (1000) volts directly across the loop input lines or from either loop input line to chassis. An open loop circuit shall provide a constant vehicle call output.

d. Multiple channel units shall scan loop input circuits to eliminate cross-talk and have frequency switch to reduce cross-talk between units.

e. Single channel and two channel units shall have a Delay-Call feature provided. This feature shall delay the detector call output for an adjustable zero to thirty (0-30) seconds. The associated green phase (120 VAC) shall be monitored and inhibit the delay times, thus causing no time delay between the sensing and outputting of the vehicle call.

f. Single channel and two channel units shall have an Extension feature provided. The detector units shall, upon actuation, be capable of timing the delay setting and then immediately begin timing the extension setting without interruption. Upon expiration of the extension timing due to gaps between vehicles, the unit shall again begin timing the delay setting with the next vehicle actuation. This shall be accomplished without the use of any external circuitry. The detector units shall be Detector System, Inc., Model 921-2T, or
approved equal meeting the other applicable requirements of these Specifications.

4. Control Unit Construction

a. The control unit shall contain an integral power supply which shall operate from a fused one hundred twenty (120) VAC-60Hz source. Fuse capacity shall be identified adjacent to fuse holder on the front panel.

b. Circuit design shall be all solid state digital except for the output which will be provided by means of a plug-in relay. The relay shall have isolated normally open (N.O.) and normally closed (N.C.) contacts capable of switching all loads in the range from two (2) MA at eighteen (18) VDC to at least three (3) AMPS at one hundred thirty-five (135) VAC. In case of power failure, either internal or external to the detector unit, the relay shall provide a constant detection signal.

c. The input/output connector(s) shall be of the Military Specification (MS) type and conform to the following pin configurations:

(1) Single Channel Detector Unit

<table>
<thead>
<tr>
<th>Pin</th>
<th>Standard Function</th>
<th>Delay Function</th>
</tr>
</thead>
<tbody>
<tr>
<td>A</td>
<td>AC Neutral</td>
<td>AC Neutral</td>
</tr>
<tr>
<td>B</td>
<td>Relay (Arm)</td>
<td>Relay (Arm)</td>
</tr>
<tr>
<td>C</td>
<td>AC Line</td>
<td>AC Line</td>
</tr>
<tr>
<td>D</td>
<td>Loop Input</td>
<td>Loop Input</td>
</tr>
<tr>
<td>E</td>
<td>Loop Input</td>
<td>Loop Input</td>
</tr>
<tr>
<td>F</td>
<td>Relay (N.O.)</td>
<td>Relay (N.O.)</td>
</tr>
<tr>
<td>G</td>
<td>Relay (N.C.)</td>
<td>Relay (N.C.)</td>
</tr>
<tr>
<td>H</td>
<td>Chassis Ground</td>
<td>Chassis Ground</td>
</tr>
<tr>
<td>I</td>
<td>Not Used</td>
<td>Not Used</td>
</tr>
<tr>
<td>J</td>
<td>Not Used</td>
<td>Phase Green</td>
</tr>
</tbody>
</table>

(120 VAC)
(2) Two Channel Detector Unit

Two MA 3102A-18-1P Connectors

Each channel utilizes the same pin configuration and functions listed in Section 660-3.04, c(3)(a).

(3) Four Channel Detector Unit

MS 3106A-22-14P Connector

<table>
<thead>
<tr>
<th>Pin</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>A</td>
<td>AC Neutral</td>
</tr>
<tr>
<td>B</td>
<td>Channel 4 Relay (Arm)</td>
</tr>
<tr>
<td>C</td>
<td>AC Line</td>
</tr>
<tr>
<td>D</td>
<td>Channel 1 Loop Input</td>
</tr>
<tr>
<td>E</td>
<td>Channel 1 Loop Input</td>
</tr>
<tr>
<td>F</td>
<td>Channel 2 Loop Input</td>
</tr>
<tr>
<td>G</td>
<td>Channel 2 Loop Input</td>
</tr>
<tr>
<td>H</td>
<td>Chassis Ground</td>
</tr>
<tr>
<td>J</td>
<td>Channel 3 Loop Input</td>
</tr>
<tr>
<td>K</td>
<td>Channel 3 Loop Input</td>
</tr>
<tr>
<td>L</td>
<td>Channel 4 Loop Input</td>
</tr>
<tr>
<td>M</td>
<td>Channel 4 Loop Input</td>
</tr>
<tr>
<td>N</td>
<td>Channel 1 Relay (N.C.)</td>
</tr>
<tr>
<td>P</td>
<td>Channel 1 Relay (Arm)</td>
</tr>
<tr>
<td>R</td>
<td>Channel 2 Relay (N.C.)</td>
</tr>
<tr>
<td>S</td>
<td>Channel 2 Relay (Arm)</td>
</tr>
<tr>
<td>T</td>
<td>Channel 3 Relay (N.C.)</td>
</tr>
<tr>
<td>U</td>
<td>Channel 3 Relay (Arm)</td>
</tr>
<tr>
<td>V</td>
<td>Channel 4 Relay (N.C.)</td>
</tr>
</tbody>
</table>

The operation of the control unit shall not be affected by changes in the inductance of the loop caused by environmental changes. The operating temperature range of the control unit shall be -35°F to +165°F, with the rate of temperature change not exceeding 1°F per three (3) minutes. The unit shall operate within all limitations of this specification. The control unit shall be capable of tracking loop inductance shift of ten percent (10%) with no loss of sensitivity. An "off" switch shall be capable of blocking loop detection. All relays and large scale integrated circuits (LSI's) shall be socket mounted.

Each control unit shall be housed in a metal case. Control units may be furnished with either an integral power supply or
a separate power supply common to more than one (1) sensor unit.

B. Preemption Equipment

All preemption equipment shall be 3M Company Opticom Control System equipment or equivalent. All emergency preemption communication equipment shall be Actelis Intelligent Transportation Systems equipment or equivalent. All equivalent substitutions shall be submitted to the Traffic Engineer for testing and approval prior to installation.

During signal preemption, the controller unit rather than the phase selector or auxiliary logic shall perform all interval timings, signal sequences, and phase skips.

The controller assembly shall be furnished with:

1. Two (2) Model 754 Phase Selectors (one for emergency backup), except that Model 752 Phase Selector may be used for 2-Phase operations when shown on the Drawings.

2. One Model 760 Card Rack.

3. One Opticom Panel Assembly, U.S. Traffic Corporation part number 103303 or approved equal, which interfaces the card rack to the controller cabinet.

4. One model ML688 with 4x10/100M copper Ethernet ports and a 100Base-FX optical SFP port.

The Model 760 card rack and Opticom Panel Assembly shall be wired in accordance with the latest controller assembly wiring diagram available from the Traffic Signal Electronics Shop Foreman.

C. Traffic Logging Equipment

With each controller assembly containing detector amplifiers, a Detector System Inc. "Traffic Logging System" TLS-1-C1 harness and Model TLS-1 unit shall be provided.

A 120 VAC receptacle shall be provided on the left-hand side of the cabinet.

D. Radar Detectors

Radar detectors shall not be used unless approved by the Traffic Engineer.

Any substitutions of cable, material or equipment must be submitted to the Traffic Engineer for testing and approval prior to installation.
At the locations shown on the Drawings, install sensors that operate in the X-band radar frequency and use microprocessor analyzed Doppler-microwave detection method for detecting vehicles moving toward or away from the unit. Furnish sensor units that:

1. Shall be installed in accordance with the manufacturers written installation instructions.

2. Shall be Microwave Sensors Model TC26B sensors that operate in the X-band radar frequency and use microprocessor analyzed Doppler-microwave detection method for detecting vehicles moving toward or away from the unit.

3. Install a sixty-hertz (60-hz) transformer with one-hundred-twenty-volt (120V) AC primary and twenty-four-volt (24V) AC secondary in the controller cabinet. Follow the manufacturer’s requirements for current output per detector. Furnish a UL-listed, class two (2) rated transformer with built-in overload and short circuit protection.

4. A fuse block shall be provided with four (4) fuses to protect the twenty-four-volt (24V) AC transformer secondary and isolation relay from damage due to faults outside of the controller cabinet assembly.

5. For each detector, a socket mounted isolation relay shall be provided for the controller cabinet assembly. The relay shall be an RH1B-UAC24V with snap mount socket and aluminum din rail. One leg of the relay coil shall connect to the twenty-four-volt (24V) AC transformer secondary while the other leg shall be fused and connected to the detector unit relay.

E. Ultrasonic Presence Sensors

Ultrasonic presence sensors shall not be used unless approved by the Traffic Engineer.

Any requests for substitutions of cable, material or equipment must be submitted to the Traffic Engineer for testing and approval prior to installation.

Ultrasonic presence sensors shall be Microwave Sensors Model TC30C.

At the locations shown on the Drawings, install sensors that operate at an ultrasonic frequency and sample for the presence of stationary and moving vehicles at a rate of ten times per second. Furnish sensor units that:

1. Feature solid-state circuitry and a high-speed transducer with the sensitivity to detect motorcycles. The cone of coverage produced by the transducer should measure at least four (4) feet in diameter at least twenty-one and one-half (21.5) feet from the transducer. The unit shall provide a continuous
call to the controller unit with the presence of a vehicle in its cone of coverage and whenever the unit loses power.

2. Include an external control to adjust the length of the cone of coverage, an external detection light emitting diode, and mounting brackets suitable to install the units on traffic signal pole mast arms.

3. Require no external amplifiers, seasonal tuning, or special cabling, and operate at all temperatures from -31°F to 167°F (-35°C to 75°C). Wire each sensor using two (2) pairs of our six-pair loop lead-in cable.

4. Operate on a voltage of twelve (12) to twenty-four (24) volts AC. Install a sixty-hertz (60 hz), forty (40) volt-ampere transformer with 120-volt AC primary and 24-volt AC secondary in the controller cabinet. Furnish a UL-listed, Class 2 rated, transformer with built in overload and short circuit protection.

F. External Special Logic circuits shall not normally be used.

**Article 17.5 Controller Cabinet**

A. The Contractor shall supply the following standard features:

1. **Cabinet Construction**

   Each controller unit and all auxiliary equipment shall be housed in a weatherproof, aluminum or steel, approved metal cabinet(s).

   The cabinet shall have a main door equipped with a construction core lock. The lock shall be capable of accepting a Best CX series core that will be installed by the Municipality after completion of the Contract. It shall have an auxiliary door equipped with lock and police key. The door hinge pin shall be continuous and made of stainless steel material. Two (2) keys shall be furnished for each lock. The police key shall have a shaft at least one and three-quarters inches (1 3/4") in length.

   Each controller cabinet door which is twenty-two inches (22"), or larger, in width or six (6) square feet, or larger, in area shall be provided with a stop to limit door opening to both ninety (90) and one hundred eighty (180) degrees, plus or minus ten (10) degrees. The stop shall be provided with a catch which can be operated when the door reaches the extreme open position and will hold the door open securely until released.

   Substantial metal shelves or brackets shall be provided to support controller unit and auxiliary equipment.

   Machine screws and bolts shall not protrude beyond the outside wall of the cabinet.
2. Cabinet Ventilation

Each controller cabinet shall be provided with louvered vents with a permanent metal mesh air filter held firmly in place, which will permit the fan to pass the volume of air specified.

Each controller cabinet shall be equipped with an electric fan with ball or roller bearings and a capacity of at least one hundred (100) cubic feet per minute. The fan shall be mounted near the top of the controller cabinet.

The fan shall be thermostatically controlled and shall be manually adjustable to turn on between 90°F and 150°F, with a differential of not more than 10°F between automatic turn on and turn off. The cabinet fan circuit shall be fused at one hundred twenty-five percent (125%) of the ampacity of the fan motor installed.

The fan and cabinet vent shall be located with respect to each other to direct the bulk of the air flow over the controller unit or through the ventilating holes of the controller unit where such holes exist.

The fan and cabinet vent shall be furnished with internally mounted, metal covers, fabricated to close off the flow of air during winter operation.

The cabinet shall be equipped with a selectable, 750/1500 watt cabinet heating device. The heating device shall be equipped with a remote air sensing thermostat. The contacts shall be rated twenty (20) amps, one hundred twenty (120) volts. The thermostat shall be constructed so that contacts close on descending temperature, adjustable between 0°F and 30°F±5°F. The contacts shall open on rising temperature at 15°F above closing temperature. The adjustment shall have an indicating pointer. The thermostat shall be connected in series with an electrical resistance heater and blower fan. The blower fan shall be rated for continuous duty. The heater and fan shall be connected in parallel and rated one hundred twenty (120) volts, sixty (60) hertz. The unit shall be mounted as shown on the latest controller assembly layout diagram available from the Traffic Signal Electronics Shop Foreman. Care shall be taken not to block the air intake or outlet. The unit shall be provided with an SPST manual override switch which shall bypass the thermostat enabling the fan and heater to operate at warmer temperatures.

3. Cabinet Wiring

Wiring within controller cabinets shall be neatly arranged and conform to the requirements of Sections 80.10 through 80.13.
Wiring shall be color coded as follows:

For DC voltages:
- Logic Ground: Gray
- +24VDC: Red with White tracer

For AC voltages:
- AC+: Black
- AC-: White
- AC Chassis Ground: Green

Load switch wiring shall be color coded as follows:
- Vehicle Load Switch: Red
- Vehicle Load Switch: Yellow
- Vehicle Load Switch: Green

- Pedestrian Load Switch: Don’t Walk: Orange
- Pedestrian Load Switch: Walk: Blue

Transfer Relay: Brown

All wiring, switches, surge protectors, flash relays, flashers, etc., shall be sized to handle the necessary amperage required under full cabinet use.

Intersection flash operation shall be programmable by rearranging interconnecting jumpers between the load switches, flash transfer relays and field signal wire terminal blocks. Changes shall be accomplished by moving the jumpers at the terminal blocks.

Each standard function of the controller unit shall be brought to an external terminal through the regular controller connector and wiring harness. Logic ground shall not be connected to AC neutral.

Calling detector operation, when called for on the Drawings or in the Special Provisions, shall be accomplished by connecting the detector amplifier relay to parent phase red drive circuit, without the use of external logic cards or relays.

Terminals and Terminal Blocks

a. Two (2) or more insulated terminal blocks shall be provided for terminating field conductors. Each block shall be provided with twelve (12) poles with 10-32 screw type terminals and 10-32x3/8" screws. The terminal block shall be a barrier type with removable shorting bars in each of the twelve (12) positions and with integral type marking strips. All conductors shall be terminated to a terminal block.
b. All terminal blocks shall be provided with nickel, silver, or cadmium plated brass binder head screw terminals.

c. Conductors from the controller unit shall terminate in "ring" type terminal lugs or shall be soldered to a through panel solder lug on the rear side of the terminal. All other conductors shall terminate in "spade" type terminal lugs.

d. Terminal blocks rated 1,000 volts, RMS maximum, twenty (20) amperes, may be provided for connecting cables from the controller unit and conductors from the external solid-state logic circuits. Conductors shall be secured to the terminal blocks with 6-32x1/4 inch binder head screws.

e. No more than three (3) conductors shall be brought to any one (1) terminal. Two (2) flat metal jumpers, straight or U-shaped, may also be placed under a terminal screw. At least two (2) full threads of all terminal screws shall be fully engaged when the screw is tightened. No live parts shall extend beyond the barrier.

f. Field terminals shall be installed on a channel support running from one side of the door opening to the other. Terminals shall be under the front edge of the shelves and shall be oriented for screwdriver operation straight in from the door opening. Terminals shall be a minimum of six inches (6") above the foundation.

g. A Type 66 B3-50 terminal block shall be installed for telemetry cable terminations.

h. The right hand side of the cabinet shall have a sixteen (16) position neutral buss bar and a sixteen (16) position ground buss bar. The left-hand side of the cabinet shall have a thirty-two (32) position neutral buss bar and a thirty-two (32) position ground buss bar.

All controller cabinets shall be wired to handle the full capacity of the controller unit specified on the Drawings (6 or 8 phase).

4. Cabinet Accessories

a. Circuit Breakers

   (1) Control circuit breakers shall be fifty (50) ampere rating for eight- (8-) phase cabinets or thirty (30) ampere rating for six- (6-) phase cabinets.

   (2) Auxiliary circuit breakers shall be rated at twenty (20) amperes. One breaker shall provide protection for fan, light
and ground fault interruptor (GFI) convenience outlet. One breaker shall provide protection for the heater.

b. Radio Interference and Transient Suppressors

Each traffic controller unit, flasher, and other current interrupting device shall be equipped with a suitable metal oxide varistor in advance of a suitable radio interference suppressor installed at the input power point. Interference suppressors shall be of a design which will minimize interference in both broadcast and aircraft frequencies, and shall provide a minimum attenuation of fifty (50) decibels over a frequency range of two hundred (200) kilohertz to seventy-five (75) megahertz when used in connection with normal installations. The interference suppressor shall be hermetically sealed in a substantial metal case filled with a suitable insulating compound. Terminals shall be nickel-plated, 10-24 brass studs of sufficient external length to provide space for connecting two (2) No. 8 AWG conductors, and shall be so mounted that the terminals cannot be turned in the case. Ungrounded terminals shall be properly insulated from each other, and shall maintain a surface leakage distance of not less than one-quarter (1/4”) inch between any exposed current conductor and any other metallic part, with an insulation factor of 100-200 megohms dependent on external circuit conditions. Suppressors shall be designed for one hundred twenty-five percent (125%) of the total connected load and in no event less than twenty-five (25) amperes, one hundred twenty (120) volts, sixty (60) Hz, single-wire circuits, and shall meet standards of the UL and the EIA.

c. Surge Protection

The power supply to the controller shall have transient voltage protection conforming to the following minimum requirements:

A Gas Tube, TII-317B or approved equivalent, shall be installed on the load side of the main circuit breaker immediately before the radio interference suppressor (RIS) filter.

A Metal Oxide Varistor, shall be connected to the load side of the RIS filter.

The transient voltage suppressor shall be a solid-state high energy circuit, containing no spark gap, gas tube, or bar component. The protection provided shall be a transient suppression of 200 volt peak, a transient response of less than five (5) nanoseconds, a power dissipation of 10,000 watts.
The protector shall function with a 10 x 1000 microsecond waveform clamping no greater than 200 volts peak. Transient voltage surge supressor (TVSS) self sacrifice shall be indicated by an external lamp, but the load shall not be disconnected from the AC power.

d. Light Fixture

Light fixture. The cabinet light fixture shall be an incandescent type porcelain lamp holder rated for 660W-250V AC/CA. The lamp shall be 110W.

e. Control Panel

A control panel assembly, readily accessible from the front of the cabinet, shall be provided and labeled. The control panel assembly shall consist of:

(1) A "Controller Power" switch shall be provided to energize the controller while the signal lights are off or being operated by the flasher. The switch shall be labeled and rated for load current.

(2) A convenience outlet shall be provided. The convenience outlet shall be a three- (3-) wire, twenty (20) ampere, one hundred twenty (120) VAC plug receptacle, in duplex configuration, with ground fault protection.

(3) A "Signal-Flash" switch which, when placed in the "Flash" position, shall provide flashing operation, without interrupting controller unit power. When said switch is placed in the "Signal" position, the controller unit shall provide normal operation.

(4) A "Stop-Time" switch which, when placed in the on position, will cause the controller unit to stop time. In the "Off" position, the controller unit shall continue normal timing regardless of external commands. In the "Auto" position, timing shall be normal, but subject to external command interruptions.

(5) A "Heater Bypass" switch shall be connected to bypass the remote heater thermostat.

(6) A "Cabinet Light" on-off switch. (A door activated switch shall also turn off the light when the door is closed.)
(7) Momentary contact test switches shall be provided to place calls on each vehicle and pedestrian phase. Switches shall provide tactile feedback and shall be rated at one ampere, minimum, for a resistive load at one hundred twenty (120) volts AC and at twenty-eight (28) volts DC. Contacts shall be coin silver or gold plated, and shall be enclosed and labeled as to their function.

(8) "REM/TOD/FREE" switch: In "REM" position will cause the local TOD and CRD to follow instructions of the remote master. In "TOD" position will allow the local TOD to operate independent and the local CRD to follow the local TOD instructions. In "FREE" position the local CRD shall have no effect on the local controller operation and the local TOD shall not be affected.

(9) "Force-Off" switch: A "Force-Off" switch shall be provided for each 'Ring.' The switches shall be the momentary contact type, provide tactile feedback and shall be rated at one (1) ampere minimum for a resistive load at one hundred twenty (120) volts AC and at twenty-eight (28) volts DC. Contacts shall be coin silver or gold plated, and shall be enclosed and labeled as to their function.

f. Police Panel Assembly

A labeled police panel assembly, located behind the auxiliary door, shall be provided. The police panel assembly shall consist of:

(1) A "Flash-Automatic" switch which, when placed in the "Flash" position, shall cause the intersection displays to go into the flashing mode. When placed in the "Automatic" position, it shall cause the signal system to resume normal operation.

(2) A "Signal ON/OFF" switch which, when placed in the "OFF" position shall remove AC power from the load switches and disable control, by any means, of the Flash Transfer Relays. The cabinet shall not be allowed to output AC power to any external device.

(3) A removable, rigid metal cover shall be provided on the back of the police panel to cover the live switch terminals.

g. The cabinet shall contain a conspicuous warning sign against operation without the conflict monitoring device being installed.
Article 17.6 Operation

Unless otherwise shown on the Drawings, the following operations shall be provided:

A. The controller cabinet shall be wired to flash the yellow signals on the main street or highway and the red signals on the cross streets and left turn lanes.

B. The flashing circuit shall be independent on the controller unit and shall remain in operation upon shutdown of the controller or removal of the controller from the cabinet.

C. The controller cabinet shall be wired so that removal of the conflict monitor shall cause the intersection to go into flashing operation.

D. Transfer to flashing operation shall be accomplished by relays between the normal load switching device and the field terminals.

E. Pedestrian pushbuttons shall not be operated at more than 24 volts.

F. Controller Priorities. The drives, controls and equipment shall have priorities and each device, control or item of equipment shall override the operation of those items listed below it:

1. Power failure
2. Power restart
3. Flashing
4. Railroad preemptor
5. Emergency vehicle preemptor
6. Phase selector
7. Interconnect
8. Time switch
9. Normal controller unit operation

Article 17.7 Shop Tests

Traffic controller cabinet, cabinet wiring diagrams, controller units, and auxiliary equipment shall be shipped to the Traffic Signal Electronics Lab at 3650 East Tudor Road, Building C. The Traffic Signal Electronics personnel will inspect cabinet wiring, burn in signal cabinet equipment, customize cabinets(s) for desired operation and test in accordance with the following specifications.

The Contractor shall allow six (6) weeks to allow for shop testing. All required equipment including wiring diagrams shall arrive in one shipment. Partial shipments will not be accepted and will be returned to the vendor.
Traffic signal equipment shall meet the operational and functional requirements of the Drawings and Specifications when tested in accordance with NEMA Standards Publication Number TS-1-1989, Traffic Control Systems.

If a partial failure occurs at any step in the test - physical, environmental, or operational - the Contractor will be permitted to make on-the-site repairs within ten (10) days after notification of the malfunction. The test will then be restarted at beginning of category in which malfunction occurred. Failure to make repairs in ten (10) days after notification will result in rejection.

If equipment malfunctions twice in the same category, the equipment will be rejected. When equipment is rejected, the entire package, including cabinet, will be returned freight collect to the vendor. New equipment, with a different serial number, must then be submitted for testing. Rejected equipment shall not be used on signal projects within the Municipality.

Testing subsequent to rejection of the equipment for failure to comply with specification requirements will be at the expense of the Contractor. Deductions to cover the costs of such testing will be made from any monies due or which may come due the Contractor under this Contract.

A failure shall be defined as any occurrence which results in other than normal operation of the equipment. The equipment is considered to have failed if any of the following occur:

1. The controller unit malfunctions.
2. The load switch produces incorrect signal indications.
3. The signal conflict monitor fails to satisfy the requirements of the Contract Specifications.

After satisfactory completion of the shop test, the Contractor will be notified to pick up the tested and marked equipment at the test site. The Contractor shall pick up successfully tested equipment within two (2) weeks of notification. The Contractor shall deliver said equipment to the Work site. Successful completion of the test does not relieve the Contractor of equipment warranty obligations as specified in Section 80.01, Article 1.4 – Warranties, Guarantees, and Instruction Sheets, or field testing as specified in Section 80.16, Article 16.2 – Field Tests.

Copies of the shop test results will be sent to the Contractor and associated vendor.

**Article 17.8 Installation**

A. Typically, cabinets shall be positioned so that the corner formed by the cabinet back and the side wall, to which the door hinge is attached, is the nearest point of the cabinet to the center of the intersection. Cabinet positioning shall be subject to the review and acceptance of the Traffic Signal Electronics Foreman.
B. Where the cabinet is mounted on a concrete pedestal foundation, a one inch (1") drain hole or pipe, with screen, shall be placed in the foundation, connecting to the cabinet and emptying above the ground line.

C. A three-eighths inch (3/8") fillet of silicone caulking shall be placed between each controller cabinet and the concrete slab foundation to prevent dust and dirt from entering the cabinet.

D. When called for in the Drawings or Special Provisions, two inches (2") of approved foam insulation shall be added within the bottom of the cabinet, between the control equipment and the concrete base. All wiring, terminals, and other items shall be designed to allow sufficient room for the insulation.

E. On Precast Controller Foundations. When called for in the Drawings or Special Provisions, a three-eighths inch (3/8") thick, two- (2-) piece exterior grade plywood board shall be placed on the bottom of the cabinet and under the foam insulation. Holes shall be made to allow for the conduits entering the cabinet. A pliable sealant composed of silicone caulking compound shall be placed between the plywood board and the cabinet and between the plywood board and all the conduits.

**Article 17.9 Measurement**

Traffic signal controllers will be measured as units, complete and in place, including labor, cabinet, hardware, controller unit, standard and auxiliary equipment, all as required by this Section and by the Contract Drawings Special Provisions to provide a complete and working system.

**Article 17.10 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Six-Phase Controller Unit</td>
<td>Each</td>
</tr>
<tr>
<td>Eight-Phase Controller Unit</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.18  VEHICLE DETECTORS

Article 18.1  Loop Detectors

For the purpose of this Section, the "home run" and the "home run loop wires" are defined as the pair of wires from the loop in the traveled lane to the nearest junction box. The "lead-in" or "lead-in cable" is defined as the shielded twisted pair cable from the junction box to the controller cabinet.

All connections between the loop wire and shielded lead-in cable shall be made as follows:

A. Shielded lead-in cable pairs shall not be cut except when they are to be spliced to a loop and then only at the junction box where the splice shall be made. Used and unused ends of the cut pairs should be of equal length.

B. Place tubular heat shrink insulation over the foil and drain wire on all cut ends of the shielded lead-in cable pairs to insure drain wire and conductive side of foil are insulated from all other foils and drain wires.

C. Twist the loop and lead-in wires together and solder.

D. Insert the wires into one end of an insulated tinned copper compression fitting and crimp securely.

E. Seal the connection in a clear re-enterable-encapsulating splice kit. Cable splices shall be inserted with a half inch (1/2") of free space from the end of the protective covering. A half inch (1/2") of free space, measured from the outside dimensions of the splice cables to the inside dimensions of the protective covering, shall be left around the circumference and at the ends of all splice cables. Free space shall be filled with a re-enterable encapsulating gel.

The Contractor shall conduct a megohm meter test at five hundred (500) volts DC on all loops. The ends of the loops shall be temporarily spliced together and the resistance to ground measured. The resistance shall not be less than one hundred (100) megohm. If the resistance is less than one hundred (100) megohm, the Contractor shall be responsible for correcting the problem(s), and the test shall be repeated until passed. The test shall be performed at the time of the pre-final inspection or when the signal is put into operation, whichever comes first; however, the Contractor is encouraged to test the loops to his own satisfaction prior to inspection.

The DC resistance between two (2) ends of the same loop wire shall be measured and shall not exceed one and one-half (1.5) ohms per one hundred (100) circuit feet. Where existing lead-in cable is being used, the Contractor shall perform the above tests on the lead-in cable prior to splicing with the loop wires. If the tests on the existing lead-in cable
are not satisfactory, the Engineer may request that it be replaced at an appropriate negotiated price.

All the above tests are to be performed, and the results recorded, by the Contractor in the presence of the Engineer. The Contractor shall supply all the test equipment and deliver signed legible copies of the test reports to the Engineer.

Conduit-encased loop wires shall be No. 14 AWG stranded copper wire with PVC tube (IMSA Spec. 51-5).

Each detector loop shall be connected to its own detector lead-in pair of seven (7) pair #18 shielded conductor cable to be used for detector lead-ins. All parallel and series connections shall be made at the cabinet terminal strip.

The upstream and intermediate loop home runs shall be routed to the nearest junction box along a path perpendicular to the direction of travel. Home runs for adjacent loops less than sixteen feet (16') apart shall be routed to the nearest junction box in the same trench to the extent possible to minimize excavation of the pavement.

The stop bar loop home runs will generally be routed to the same junction box. All the home runs shall be routed parallel and adjacent to each other along a path perpendicular to the direction of travel. A path parallel to the direction of travel may be needed from the individual loop to the common perpendicular routing.

Loop locations may be staggered plus or minus six inches (±6") to accommodate home run placement.

The loop conduit shall be PVC Schedule 80, conforming to ASTM D1785, except that the "X" connection of the loop to the home run shall be a hot dip galvanized steel conduit. The Contractor shall use one inch (1") diameter conduit.

The home run conduit shall be PVC Schedule 80.

For installation of loops in new construction, loops will be placed in compacted leveling course bedding material conforming to Division 20, Section 20.22 – Leveling Course. There shall be a minimum thickness of one inch (1") of leveling course beneath the loop conduit and condulet. A minimum of one inch (1") of compacted leveling course shall be placed on top of the loop conduit and condulet prior to paving.

Sand bedding shall be non-frost-susceptible, consisting of naturally occurring fine mineral aggregates free of clays, silt, or organic matter.

In constructing conduit encased detector loops, the Contractor shall be responsible for devising his own methods of cutting and restoring the existing asphalt concrete pavement, provided that his methods:

A. Allow for the maintenance of traffic through the construction zone, and;
B. When installing loop detectors in existing pavement, cut the asphalt with a saw and remove all asphalt within the saw cut, and;

C. Replace asphalt concrete in two (2) equal lifts, with a minimum thickness of three inches (3") or the existing pavement thickness, whichever is greater, and;

D. Provide for reconstruction of the base material and asphalt concrete, in accordance with Division 20 - Earthwork and Division 40 - Asphalt Surfacing, and;

E. Are approved by the Engineer.

Full-lane-width asphalt patching is required. Any remaining section less than twelve feet (12') between loops must be replaced to provide a continuous patch. The Contractor shall adhere to the following guidelines:

A. Where existing pavement will not be overlaid, enclose all loops that enter a common junction box within a trapezoidal saw-cut.

B. Cut to within one foot (1') of the lane and edge lines, preserving the pavement markings.

C. Remove the asphalt to the lip of the gutter when there are no edge lines.

D. Cut across lane lines when loops in adjacent lanes are side by side.

E. Cut trenches a minimum of three feet (3') wide installing loop tails across a lane.

F. Trenches crossing a shoulder only may be a minimum of one foot (1') wide.

G. Contractor shall saw-cut asphaltic concrete pavement for loops in existing pavement.

H. All existing pavement between detection loops within twelve feet (12') of another loop shall be saw-cut and removed, then repaved continuously.

I. All traffic markings removed as a result of loop installation in existing pavement shall be replaced to the original alignment and messages.

After the asphalt concrete pavement has been removed according to the Contractor's approved method, the area where the conduit-encased detector loop is to be placed shall be cleared of all rocks and protrusions which may damage the conduit. Sand bedding or leveling course may be required by the Engineer to attain an adequate surface. The Contractor shall take care that the subgrade is not disturbed. If the subgrade material is disturbed, the Contractor shall compact the surface to the satisfaction of the Engineer.
The conduit-encased detector loop shall then be installed and sand bedding or leveling course shall be placed over the loop to a minimum of one inch (1") compacted depth.

An asphalt tack coat per Division 40, Section 40.04 – Tack Coat, shall be applied to all edges of the existing pavement prior to placing new asphalt. Asphalt concrete pavement shall be placed and compacted in a minimum of two (2) equal lifts per Division 40 – Asphalt Surfacing.

When loops are installed in existing pavement or as new Work, traffic shall not be allowed to drive over the loops until the first layer of asphalt has been placed.

**Article 18.2 Optical Detectors - Preemption**

Any substitutions of cable, material or equipment in this Article must be submitted to the Municipal Traffic Engineer for testing and approval prior to installation.

Before installing the Optical Detectors, gain approval of their final lateral location on the mast arms from the Municipal Traffic Engineer or assigned designee.

Install the following components of the 3M Company’s Opticom Priority Control System in accordance with 3M’s written installation instructions at each signalized intersection listed on the Drawings:

A. Install the quantity of Model 711, 721, or 722 Optical Detectors on the traffic signal pole mast arms as shown on the Drawings. Mounting hardware shall consist of Pelco Products, Inc. "Astro Mini-Brac" Band Mount Clamp Kits, or approved equal, and 3M Company "M575 Confirmation Light Hardware" Installation Kit, or approved equal. See the Drawings for installation details.

B. Install Model 138 Optical Detector lead-in cable and a three-conductor #14 AWG signal cable between each optical detector and the controller cabinet. Furnish enough slack in these cables to extend two feet (2’) beyond the end of each signal mast arm and to leave ten feet (10’) of slack in the controller cabinet. Label each cable.

C. Before attaching the conductors to the optical detector, strip the insulation from the conductors in the Model 138 cable and attach all four (4) conductors to chassis ground in the controller cabinet. Attach the signal cable to the confirmation light. The Traffic Signal Electronics Shop will make final terminations of the conductors in the controller cabinet.

D. When retrofitting existing controller cabinets, furnish each controller assembly with two (2) Model 754 phase selector, a Model 760 card rack, and an Opticom Panel Assembly, U.S. Traffic Corporation part number 103303. Deliver these parts to the Traffic Signal Electronics Shop Foreman.
New controller cabinets shall include these parts installed by the controller assembly manufacturer per Section 80.17, Article 17.4 - Special Auxiliary Equipment.

**Article 18.3 Radar Detectors**

Radar detectors may not be used without prior approval of the Traffic Engineer. Any substitutions of cable, material or equipment in this Article must be submitted to the Traffic Engineer for testing and approval prior to installation.

Radar detectors shall be Microwave Sensors Model TC26B sensors that operate in the X-band radar frequency and use microprocessor analyzed Doppler-microwave detection method for detecting vehicles moving toward or away from the unit. They shall be installed in accordance with the manufacturers written installation instructions and the following requirements:

A. Install a sixty-hertz (60 hz) transformer with one hundred twenty (120) volt AC primary and twenty four (24) volt AC secondary in the controller cabinet. Follow the manufacturer's requirements for current output per detector. Furnish a UL-listed, Class 2 rated transformer with built-in overload and short circuit protection.

B. A fuse block shall be provided with four (4) fuses to protect the twenty-four (24) volt AC transformer secondary and isolation relay from damage due to faults outside of the controller cabinet assembly.

C. **Home-Run Cable**

Wire each sensor using a color-coded three (3) pair cable such as Alpha Wire Company cable part # 6314 or Beldon part # 9883, or approved equal.

Pair 1 shall be black paired with red and shall be used to supply power to the unit.

Pair 2 shall be black paired with white and shall be tied back at both ends.

Pair 3 shall be black paired with green and shall connect to the relay contacts in the unit. In the cabinet, this pair shall be connected to the twenty-four (24) volt AC transformer secondary and the isolation relay.

D. **Isolation Relay**

For each detector, a socket-mounted isolation relay shall be provided for the controller cabinet assembly. The relay shall be an RH1B-UAC24V with snap mount socket and aluminum din rail. One leg of the relay coil shall connect to the twenty-four (24) volt AC transformer secondary while the other leg shall be fused and connect to the detector unit relay.
Article 18.4 Ultrasonic Detectors - Ultrasonic Presence Sensors

Ultrasonic detectors may not be used without prior approval of the Traffic Engineer. Any substitutions of cable, material or equipment in this Article must be submitted to the Traffic Engineer for testing and approval prior to installation.

Ultrasonic detectors shall be Microwave Sensors Model TC30C. They shall be installed in accordance with the manufacturer’s written installation instructions.

At the locations shown on the Drawings, install sensors that operate at an ultrasonic frequency and sample for the presence of stationary and moving vehicles at a rate of ten (10) times per second. Furnish sensor units that:

A. Feature solid-state circuitry and a high-speed transducer with the sensitivity to detect a motorcycle. The cone of coverage produced by the transducer should measure at least four feet (4’) in diameter at twenty one feet (21’) from the transducer. The unit shall provide a continuous call to the controller unit with the presence of a vehicle in its cone of coverage or whenever the unit loses power.

B. Include an external control to adjust the length of the cone of coverage, an external detection light emitting diode, and mounting brackets suitable to install the units on traffic signal pole mast arms.

C. Require no external amplifiers, seasonal tuning, or special cabling, and which operate at all temperatures from -31°F to 167°F (-35°C to 75°C).

D. Operate on a voltage of twelve (12) to twenty-four (24) volts AC.

E. Install a sixty-hertz (60 hz) transformer with one hundred twenty (120) volt AC primary and twenty-four (24) volt AC secondary in the controller cabinet. Follow the manufacturer requirements for current output per detector. Furnish a UL-listed, Class 2 rated, transformer with built in overload and short circuit protection.

F. A fuse block shall be provided with four (4) fuses to protect the twenty-four (24) volt AC transformer secondary and isolation relay from damage due to faults outside of the controller cabinet assembly.

G. Home-Run Cable

Wire each sensor using a color coded three (3) pair cable such as Alpha Wire Company cable part # 6314 or Beldon part # 9883, or approved equal.

Pair 1 shall be black paired with red and shall be used to supply power to the unit.

Pair 2 shall be black paired with white and shall be tied back at both ends.
Pair 3 shall be black paired with green and shall connect to the relay contacts in the unit. In the cabinet, this pair shall be connected to the twenty-four (24) volt AC transformer secondary and the isolation relay.

H. Isolation Relay

For each detector, a socket-mounted isolation relay shall be provided for the controller cabinet assembly. The relay shall be an RH1B-UAC24V with snap mount socket and aluminum din rail. One leg of the relay coil shall connect to the twenty-four (24) volt AC transformer secondary while the other leg shall be fused and connect to the detector unit relay.

Article 18.5 Video Detectors

Video Detectors shall not be used without prior approval of the Traffic Engineer.

Article 18.6 Measurement

Loop detectors will be measured as units, complete and in place, including all labor, equipment, specified materials and miscellaneous materials to provide a complete and functioning unit, including the home run to the nearest junction box. Bid item “Install Loop Detectors in Existing Pavement” will include all costs of cutting and restoring existing pavement.

Optical preemption detectors, radar detectors and ultrasonic detectors shall be measured as units, complete and in place, including all labor, equipment, specified materials and miscellaneous materials to provide a complete and functioning unit, including all wiring to the controller cabinet assembly and all hardware installed in the cabinet.

Article 18.7 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07, Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Install Loop Detector - New Work</td>
<td>Each</td>
</tr>
<tr>
<td>Install Loop Detector - Existing Pavement</td>
<td>Each</td>
</tr>
<tr>
<td>Install Optical Preemption Detector (Type)</td>
<td>Each</td>
</tr>
<tr>
<td>Install Radar Detector</td>
<td>Each</td>
</tr>
<tr>
<td>Install Ultrasonic Detector</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.19  VEHICLE SIGNAL HEADS

Article 19.1  General

All signal heads shall conform to the ITE Specification "Adjustable Face Vehicular Traffic Control Signal Heads." Each signal head shall be adjustable, vertical type with the number and type of sections as specified herein and shown on the Drawings; shall provide an indication in one direction only; shall be adjustable through 360 degrees about a vertical axis; and shall be mounted at the location and in the manner shown on the plans.

Vehicle Signal Heads shall have Light Emitting Diode (LED) lamps for all red, green and yellow indications. Red, green and yellow lamps shall conform to Article 19.4 – Light Emitting Diode (LED) Optical Units.

Each lens shall be provided with a removable tunnel visor, with an open slot at the bottom. The visor shall be constructed of aluminum. The color shall be Econolite green and black.

Vehicle signal housings shall conform to the following:

1. All parts of the housing, including the doors and end plates, shall be of die cast aluminum conforming to the specifications of ASTM B85, and all parts shall be clean, smooth, and free from flaws, cracks, blow holes, or other imperfections, unless otherwise called for on the plans.

2. The housing of each signal section shall be one piece with integral top, bottom, and sides, with square doors.

3. All exposed bolts, screws, hinges pins, and door locking devices shall be stainless steel. All interior screws and fittings shall be stainless steel or approved non-ferrous, corrosion-resistant material.

4. The top and bottom of each housing shall have an opening to accommodate standard one and one-half inch (1-1/2") pipe fittings and brackets.

5. The top and bottom opening of the housing shall have an integral serrated boss that will provide positive positioning of the signal head in five (5) degree increments to eliminate undesirable rotation or misalignment of the signal head as well as between sections. A total of seventy-two (72) teeth shall be provided in the serrated boss. The teeth shall be clean and sharp to provide positive positioning with the grooves of the mating section or framework.

6. Individual signal sections shall be fastened together with a cadmium-plated tri-stud connector, lock washers, and nuts with access holes for the passage of electrical conductors from one section to another.

7. Each signal housing shall have two (2) integral hinge lugs located on the left side for mounting the door.
8. One (1) or two (2) latches shall be provided on the right side of each signal housing with stainless steel wing nut assemblies for the purpose of engaging the door latches.

9. Each signal housing shall have a door opening for the periphery which shall have a soft, closed-cell neoprene gasket to provide a weather-tight seal with the mating door.

10. The door of each signal housing shall be provided with a round opening designed to accommodate any standard traffic signal lens.

**Article 19.2 Installation**

A. Vehicle signal heads shall not be installed at any intersection until all other signal equipment, including the controller, is in place and ready for operation at that intersection, unless the faces are not directed toward traffic or unless the faces are adequately covered. Contractor shall cover heads with beige colored canvas shirts sized to fit the signal faces shown in the Drawings. Each shirt shall feature elasticized openings that fit over the visors and at least two straps to secure it to the signal. Provide shirts with a legend that reads “out of service” and a center section that allows an operator to see the indications during system tests.

B. Signal heads, backplates, visors, mounting brackets and fittings shall be painted as specified in Section 80.16, Article 16.4 - Painting for Steel Structures.

C. Vehicle signal head mounting hardware shall be attached to the side of pole that faces away from traffic unless otherwise approved by the Engineer.

D. All conductor access holes drilled for side mounted vehicular signal heads shall be deburred inside and out to prevent scraping of the conductors. The holes shall be cleaned and painted with two (2) coats of zinc chromate primer for metal.

E. Through phase vehicle signal faces shall be aimed at a point located a distance from the face as shown in the following table. If two (2) through signal faces are not visible from this point at an eye height of three and one-half feet (3.5’) above finished grade, then the Traffic Engineer shall be consulted for corrective measures.
Signal Head Visibility Distances

<table>
<thead>
<tr>
<th>Posted Speed Limit (mph)</th>
<th>Minimum Visibility Distance (ft.)</th>
</tr>
</thead>
<tbody>
<tr>
<td>20</td>
<td>328</td>
</tr>
<tr>
<td>25</td>
<td>394</td>
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<td>55</td>
<td>820</td>
</tr>
<tr>
<td>60</td>
<td>902</td>
</tr>
</tbody>
</table>

F. Backplates shall be attached to the signal head using plated steel rivets with large flange button heads. The rivet shall be two-tenths inches (0.2”) in diameter and 0.575 inches long and at least 2380 N and 3000 N shear and tensile strength, respectively. Bore out the mounting holes in the backplates and signal heads to the diameter recommended by the rivet manufacturer.

G. Each lens shall be provided with a removable tunnel visor, with an open slot at the bottom, constructed of aluminum and securely installed using machine screws. The threads of the machine screws shall be coated with an antiseizing compound before installation.

H. Removal and relocation of existing signal heads, as shown on the Drawings, shall utilize new mounting hardware.

I. When installing LED signal heads, the Contractor shall clearly and permanently mark the date installed on the back of each unit.

Article 19.3 Signal Head Mounting

A. Mast Arm Signals

Signal head assembly for suspension from mast arm shall be equipped with a bronze plumbizer.

All holes in mast arms shall be field drilled to ensure proper location of signal heads with respect to traffic lanes. Arms shall be drilled for wire accesses after installation on the base of the pole. The wire hole locations shall be directly centered on the mounting bracket on the signal head side of the mast arm.
All mast arm mounted signal heads shall be mounted using "Astro-Brac" band mount clamp kits part number AB-3007-L (with stainless upgrade option), brackets manufactured by Pelco Products Inc., or an approved substitute. The mounting nipple shall be a two inch (2") rigid metal conduit, cut to a length of six inches (6"). The mounting nipple shall have one inch (1") of tapered thread on one end, be drilled to accept the plumbizer through bolt and all openings shall be deburred.

B. Pole and Post Mounted Signals

Bracket mounted signal heads, as shown on the Drawings, shall be supported by mounting brackets consisting of watertight assemblies of one and one-half inch (1 1/2") standard steel pipe and malleable iron or brass pipe fittings. All members shall be either plumb or level, symmetrically arranged, and securely assembled. Construction shall be such that all conductors are concealed within the assembly.

The horizontal nipples shall be a minimum of twenty-two inches (22") in side-mounted frames and eleven inches (11") in post-top installations.

Where five- (5-) section vertical signal heads are side mounted on poles, a steel conduit hanger shall be installed on the vertical framework pipe, six inches (6") down from the upper horizontal framework pipe. A conduit hanger shall be mounted to the pole with a five-sixteenth inch (5/16") bolt and lock washer.

Pole side mounted traffic signal heads shall be mounted on the back side of pole unless noted or detailed otherwise on the Drawings. The pole/post-mounted traffic signal heads shall be mounted so that no portion of the backplates are hidden by the pole/post.

At each signal location, unless otherwise shown on the Drawings, a terminal compartment shall be constructed into the mounting brackets.

For post-top mounting of bracket mounted signals, the terminal compartment shall be cast with an integral slip-fitter.

For post-top mounting of a one-way signal head, a slip-fitter without a terminal compartment may be used.

Post-top signal heads with backplates shall be mounted with an offset slip-fitter to allow the signal head backplate to clear the signal pole.

C. Programmed Visibility Traffic Signal Heads

The Contractor shall program the head as recommended by the manufacturer and as directed.
When programmed, each signal face’s indication shall be visible only in those areas or lanes to be controlled, except that during dusk and darkness a faint glow to each side will be permissible.

**Article 19.4 Light Emitting Diode (LED) Optical Units**

All LED vehicle lamps shall be GELcore, Dialight or an approved equal. Any substitutions must be submitted to the Traffic Engineer for testing and approval prior to bid award. LED vehicle lamps shall meet the following specifications:

**A. Definition**

1. Eight inch (8”) and twelve inch (12”) indications for all vehicle traffic signals faces shall utilize light emitting diode signal modules.

2. Light emitting diode (LED) signal modules shall consist of an assembly that utilizes light emitting diodes as the light source in lieu of an incandescent lamp for use in traffic signal sections.

**B. General**

1. LED signal modules shall be designed to be installed in the doorframe of a standard traffic signal housing. The lamp socket, reflector, reflector holder and lens used with an incandescent lamp shall not be used in a signal section in which an LED signal module is installed.

2. LED signal modules shall be a sealed unit with two (2) conductors for connecting to power, a printed circuit board, power supply, a lens and gasket, and shall be weather proof after installation and connection. The circuit board and power supply shall be contained inside the module.

3. Conductors for modules shall be forty inches (40”) in length, with insulated quick disconnect terminals.

4. The lens of the module shall be integral to the unit, shall be convex with a smooth outer surface and made of ultraviolet stabilized plastic or of glass. The lens shall be capable of withstanding ultraviolet (direct sunlight) exposure for a minimum period of five (5) years without exhibiting evidence of deterioration.

5. The module shall be sealed in the door frame with a one-piece EPDM (ethylene propylene rubber) gasket.

6. The LEDs shall utilize AllInGaP technology and shall be the ultra bright type rated for 100,000 hours of continuous operation from -40°F to 165°F (-40°C to 74°C). AlGaS LEDs will not be allowed.
7. The failure of an individual LED in a string shall only result in the loss of that LED, not the entire string or indication.

8. Furnish LED signal modules rated for a minimum useful life of sixty (60) months and that meet current ITE Standards for LED signal modules.

C. Physical and Mechanical Requirements

1. LED traffic signal modules shall be designed as retrofit replacements for existing optical units of signal lamps and shall not require special tools for installation. LED signal modules shall fit into existing traffic signal section housings built to the VTCSH standard without modification to the housing.

2. Installation of an LED signal module shall only require the removal of the optical unit components, i.e., lens, lamp module, gaskets, and reflector; shall be weather tight and fit securely in the housing; and shall connect directly to electrical wiring.

**LED Signal Module Lens.** The LED signal module shall be capable of replacing the optical unit. The lens may be tinted or may use transparent film or materials with similar characteristics to enhance ON/OFF contrasts. The use of tinting or other materials to enhance ON/OFF contrasts shall not affect chromaticity and shall be uniform across the face of the lens.

If a polymeric lens is used, a surface coating or chemical surface treatment shall be used to provide front surface abrasion resistance.

**Environmental Requirements.** The LED signal module shall be rated for use in the operating temperature range of -40°F to 165°F (-40°C to 74°C).

The LED signal module shall be protected against dust and moisture intrusion per the requirements of NEMA Standard 250-1991 for Type 4 enclosures to protect all internal components.

The LED signal module lens shall be UV stabilized.

**Construction.** The LED signal module shall be a single, self-contained device, not requiring on-site assembly for installation into an existing traffic signal housing. The power supply for the LED signal module shall be integral to the unit.

**Module Identification.** Each LED signal module shall have the manufacturer's name, trademark, and other necessary identification permanently marked on the back of the module. Each individual LED signal module shall be identified for warranty purposes.

The following operating characteristics shall be identified: rated voltage, power consumption, and volt-ampere.
Each LED signal module shall have prominent and permanent vertical marking(s) for correct indexing and orientation within signal housing. The markings shall consist of an up arrow, or the word "UP" or "TOP."

D. Photometric Requirements

1. LED traffic signal modules shall meet at least eighty-five percent (85%) of the minimum VTCSH intensity requirements while operating throughout the operating temperature range of -40°F to 165°F (-40°C to 74°C).

2. The minimum initial luminous intensity values for LED traffic signal modules shall be as defined in Section 11.04 of the VTCSH standard at 77°F (25°C).

3. The measured chromaticity coordinates of LED signal modules shall conform to the chromaticity requirements of Section 8.04 and Figure 1 of the VTCSH standard.

E. Electrical

1. LED signal modules shall operate from a sixty-hertz (60 Hz) ±3 Hz AC line over a voltage ranging from eighty (80) volts to one hundred thirty-five (135) volts. The LED circuitry shall prevent perceptible flicker over the voltage range specified above. The fluctuations of line voltage shall have no visible effect on the luminous intensity of the indications. Rated voltage for all measurements shall be one hundred twenty (120) volts. The LED traffic signal lamp unit shall not be capable of functioning at less than forty-five (45) volts.

2. All wiring and terminal blocks shall meet the requirements of Section 13.02 of the VTCSH standard. Two (2) secured, color-coded, 1 m long 600 V, 20 AWG minimum, jacketed wires, conforming to the National Electric Code, rated for service at 221°F (+105°C), are to be provided for electrical connection for each LED signal module.

3. The signal module on-board circuitry shall include voltage surge protection to withstand high-repetition noise transients as stated in Section 2.1.6 of NEMA Standard TS-2, 1992.

4. LED signal modules shall be operationally compatible with currently used controller assemblies (solid state load switches, flashers, and conflict monitors).

5. LED signal modules and associated on-board circuitry must meet Federal Communications Commission (FCC) Title 47, Subpart B, Section 15 regulations concerning the emission of electronic noise.
6. The LED signal module shall provide a power factor of nine-tenths (0.90) or greater.

7. Total harmonic distortion (current and voltage) induced into an AC power line by a LED signal module shall not exceed twenty percent (20%).

F. Production Testing Requirements

1. Each new LED traffic signal lamp unit shall be energized for a minimum of twenty-four (24) hours at operating voltage and at a temperature of 140°F (60°C) in order to cause any electronic infant mortality to occur, and to ensure electronic component reliability prior to shipment.

2. After the burn-in procedure is completed, each LED traffic signal lamp unit shall be tested by the manufacturer for rated initial intensity at rated operating voltage.

G. Warranty

The manufacturer shall provide a written warranty against defects in materials and workmanship for the LED signal modules for a period of sixty (60) months after installation of the modules. Replacement modules shall be provided promptly after receipt of modules that have failed at no cost to the Owner except cost of shipping of the failed modules. Provide the Engineer with all warranty documentation prior to installation. Deliver the replacement modules to Traffic Signal Electronics Shop within five (5) working days of notification. The warranty does not include the costs associated with removing and reinstalling units that are replaced or repaired.

Article 19.5 Incandescent Optical Units

A. Each optical unit shall consist of a lens, a reflector, a lamp holder, and a clear incandescent traffic signal lamp.

B. Lenses shall be circular in shape and glass construction.

C. The lamp holder shall be secured to the reflector to provide a dust and moisture proof seal.

D. The reflector and lamp holder shall be held securely in place with the outer edge of the reflector engaging the lens gasket to provide a light and moisture seal for the entire optical assembly.

E. All lamp holders shall be so wired that a white wire will be connected to the shell of the lamp holder and a black or colored wire to the bottom or end terminal of the lamp holder. These wires shall, in turn, be connected to the terminal block mounted inside at the back of the housing. The terminal block shall have sufficient screw type terminals to terminate all field wires and lamp wires independently, with separate screws. The terminals to which field wires are attached shall be
permanently identified or wiring shall be color-coded to facilitate fieldwork. The terminal block shall be located in the center section of the head.

F. Lamps for the twelve inch (12”) units shall be 1,950 lumen minimum initial output, 120 volt, 6,000 hour rated life, clear, traffic signal lamps, unless specified otherwise in the Drawings.

G. Lamps for the eight inch (8”) units shall be 665 lumen minimum initial output, 120 volt, 6,000 hour rated life, clear, traffic signal lamps.

H. Reflectors shall be made of Alzak finished aluminum, the thickness of the anodic coating to be a minimum of 0.0001 inches, or its equivalent, spun or drawn from metal not less than 0.025 inches thick equipped with a bead or flange on the outer edge to stiffen the reflector and insure its being held true to shape. The reflecting surface shall be totally free of flaws, scratches, defacements or mechanical distortion.

Article 19.6 Backplates

Backplates shall be furnished and attached to all vehicle signal heads, including programmed-visibility signal heads, except for post-mounted flashers which will be installed without backplates.

Backplates shall be aluminum and the color shall be Econolite green and black. Backplate extensions (borders) shall be five inches (5”) wide. Where a backplate consists of two (2) or more sections, they shall be fastened together with aluminum rivets or bolts and peened after assembly to prevent loosening.

Construct backplates of 0.063-inch minimum thickness aluminum alloy sheet meeting ASTM B 209, alloy 3003-H14.

Backplates shall be painted as specified in Section 80.16, Article 16.4 – Painting for Steel Structures.

Article 19.7 Signal Head Mounting Hardware

A signal head assembly for suspension from mast arm shall be equipped with a bronze plumbizer.

All mast arm mounted signal heads shall be mounted using "Astro-Brac" band mount clamp kits part number AB-3007-L (with stainless upgrade option) brackets manufactured by Pelco Products, Inc., or an approved substitute. The mounting nipple shall be a two inch (2”) rigid metal conduit, cut to a length of six inches (6”). The mounting nipple shall have one inch (1”) of tapered thread on one end, be drilled to accept the plumizer through bolt, and all openings shall be deburred.
A. Pipe and Fittings

Bracket mounting hardware shall be one and one-half inch (1 1/2") standard steel pipe and malleable iron or brass pipe fittings. Construction shall be such that all conductors are concealed within a watertight assembly.

Signal heads shall be equipped with positive brass lock rings and fittings designed to prevent heads from turning due to external forces. Lock ring and connecting fittings shall have serrated contacts.

B. Terminal Compartment

Terminal compartments shall be bronze of sufficient strength to remain intact in event the pole is knocked down.

For post-top mounting of bracket mounted signals, the terminal compartment shall be cast with an integral slip-fitter. Each terminal compartment shall be fitted with a terminal block containing twelve (12) poles, each with two (2) screw type terminals. Each terminal shall accommodate at least three (3) No. 14 AWG conductors. A raintight cover shall be provided, giving ready access to the terminal block.

C. Slip-fitters

Slip-fitter shall fit over a four inch (4") standard pipe or four and one-half inch (4 1/2") outside diameter end of tapered standard. Each slip-fitter shall be provided with two (2) rows of steel set screws, with three (3) screws in each row to secure the assembly in plumb position. Set screws shall be cadmium plated.

Slip-fitters, where used without integral terminal compartment, shall be of cast-iron.

Post-top signal heads with backplates shall be mounted with an offset slipfitter to allow the signal head backplate to clear the signal pole.

D. Painting

Mounting brackets and fittings shall be painted as specified in Section 80.16, Article 16.4 – Painting for Steel Structures.

Article 19.8 Programmed Visibility Traffic Signal Heads

Each programmed visibility signal face and the installation thereof shall conform to the provisions of Article 19.1 - General; Article 19.2 - Installation; Article 19.3 - Signal Head Mounting; and Article 19.7 - Signal Head Mounting Hardware, except as modified in this subsection.

Each programmed visibility signal section shall provide a nominal twelve inch (12") diameter circular or arrow indication. Color and arrow configuration shall conform to the latest I.T.E. Specification.
Each section shall be provided with a sun visor.

Each signal section shall be provided with an adjustable connection that permits incremental tilting from zero (0) to ten (10) degrees above or below the horizontal while maintaining a common vertical axis through couplers and mounting axis in five (5) degree increments.

The signal shall be mountable with ordinary tools and capable of being serviced without tools. Adjustment shall be preset at four (4) degrees below the horizontal, unless otherwise specified.

The visibility of each programmed visibility signal face shall be capable of adjustment or programming within the face. When programmed, each signal face's indication shall be visible only in those areas or lanes to be controlled, except that during dusk and darkness a faint glow to each side will be permissible.

Prior to programming, each signal section with a yellow indication shall provide a minimum luminous intensity of three thousand (3,000) candela on the optical axis, and a maximum intensity of thirty (30) candela at fifteen (15) degrees horizontal from the axis. Each such signal section shall be capable of having its visibility programmed to achieve the following luminous intensities: a minimum of 3,000 candela on the optical axis, a maximum of one hundred (100) candela at from one-half (1/2) to two (2) degrees horizontal from the axis and a maximum of ten (10) candela at from two (2) to fifteen (15) degrees horizontal from the axis. Under the same conditions, the intensities of the red indication and the green indication shall be at least nineteen percent (19%) and thirty-eight percent (38%) respectively of the yellow indication.

Each signal face or each signal section shall include integral means for regulating its luminous intensity between limits in proportion to the individual background luminance. Lamp intensity shall not be less than ninety-seven percent (97%) of uncontrolled intensity at 1000 foot-candles, and shall reduce to 15±two percent of maximum intensity at less than one foot-candle. The dimming device shall operate over an applied voltage range of ninety-five (95) to one hundred thirty (130) volts, sixty hertz (60 Hz) and a temperature range of -40°C to 74°C.

The Contractor shall supply the material required for programming of the head. The programming of the head shall be accomplished by the Traffic Signal Electronics Shop.

**Article 19.9 Measurement**

New and relocated signal heads will be measured as units, complete and in place, including all labor, mounting hardware, equipment and materials to provide a complete and functioning unit. Measurement shall be for the actual number of existing signal heads removed and relocated, and the actual number of new signal heads having the specified number of indicators of the specified size. Left arrow, right arrow, and round ball faces of the same size will be considered identical pay items.
Article 19.10 Basis of Payment

Payment for this work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall be full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>(8 or 12) inch (3, 4, 5) Face Signal Head</td>
<td>Each</td>
</tr>
<tr>
<td>Remove and Relocate Existing Signal Head</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.20 PEDESTRIAN SIGNALS

Article 20.1 General

Pedestrian signals shall be the Light Emitting Diode (LED) Type.

Pedestrian signals shall show the following:

1. Steady "WALKING PERSON" during the pedestrian interval.
2. Flashing "HAND" during the pedestrian clearance interval.
3. Steady "HAND" after the pedestrian clearance and during the associated phases yellow and all red vehicle clearance intervals.
4. Dark during intersection flash.

Pedestrian signal mounts shall be two-piece hinge connected type (clamshell) unless otherwise shown on the Drawings.

Conductors shall be #14 AWG or larger conforming to IMSA Specification 20-1.

Article 20.2 Installation

Pedestrian signal heads shall be directed at the center of the crosswalk on the opposite side of the street.

Pedestrian signal heads shall not be installed at any intersection until all other signal equipment, including the controller, is in place and ready for operation at that intersection, unless the faces are not directed toward traffic or unless the faces are adequately covered. Contractor shall cover heads with beige colored canvas shirts sized to fit the signal faces shown in the Drawings. Each shirt shall feature elasticized openings that fit over the visors and at least two straps to secure it to the signal. Provide shirts with a legend that reads "out of service" and a center section that allows an operator to see the indications during system tests.

Pedestrian signal head mounting hardware shall be attached to the side of pole that faces away from traffic unless otherwise approved by the Traffic Engineer.

All conductor access holes drilled for side mounted pedestrian heads shall be deburred inside and out to prevent scraping of the conductors. The holes shall be cleaned and painted with two (2) coats of zinc chromate primer for metal.

Removal and relocation of existing signal heads, as shown on the Drawings, shall utilize new mounting hardware.
When installing LED signal heads, the Contractor shall clearly and permanently mark the date installed on the back of each unit.

**Article 20.3 Mounting**

Side mounted pedestrian signal hardware shall consist of a two-piece, hinge connected, cast aluminum bracket that mounts directly between the pole and the side of the signal head housing. Clearance between the head and pole shall not exceed three inches (3”). The bracket shall properly fit any round pole four inches (4”) or greater in diameter and be capable of being rotated a minimum of thirty (30) degrees when mounted on a four inch (4”) pole. A raintight, three (3) position terminal block shall be contained within the mounting bracket.

Bracket mounted signal heads, as shown on the Drawings, shall be supported by mounting brackets consisting of watertight assemblies of one and one-half inch (1 1/2”) standard steel pipe and malleable iron or brass pipefittings. All members shall be either plumb or level, symmetrically arranged, and securely assembled. Construction shall be such that all conductors are concealed within the assembly. A terminal compartment shall be constructed into the mounting bracket.

At each signal location, unless otherwise shown on the Drawings, a terminal compartment shall be constructed into the mounting brackets.

Terminal compartments shall be bronze of sufficient strength to remain intact if the pole is knocked down.

Mounting brackets and fittings shall be painted as specified in Section 80.16, Article 16.4 – Painting for Steel Structures.

**Article 20.4 Housing**

A. The case shall be a one-piece, corrosion-resistant, aluminum-alloy die-casting complete with integrally cast top, bottom, sides and back. Four (4) integrally cast hinge lug pairs, two (2) at the top and two (2) at the bottom of each case, shall be provided for operation of a swing-down door.

B. The case for pedestrian signals shall be dustproof, weatherproof, corrosion resistant, and shall provide for easy access to, and replacement of, all components.

C. Three (3) versions of the case shall be available. The first version shall be supplied with clamshell mounting hardware installed (ordered concurrently) for installation of "pole left of message." The second version shall be the "pole right of message." The third version shall contain upper and lower openings as described below suitable for either post top or bracket mounting. The first and second versions need not include upper and lower openings, but when provided shall be plugged to be weathertight.
D. The third version shall accommodate standard one and one-half inch (1 1/2") pipe brackets, top and bottom. The bottom opening of the signal case shall have a shurlock boss integrally cast into the case. The dimensions of the shurlock boss shall be as follows:

- **Outside Diameter**: 2.625 inches
- **Inside Diameter**: 1.969 inches
- **Number of Radial Teeth**: 72
- **Depth of Teeth**: 5/64 inch

The teeth shall be clean and sharp and provide full engagement to eliminate rotation or misalignment of the signal.

E. The door frame shall be a one-piece, corrosion-resistant, aluminum-alloy die-casting, complete with two (2) hinge lugs cast at the bottom and two (2) latch lugs cast at the top of each door. The door shall be attached to the case by means of two (2) Type 304 stainless steel spring pins. Two (2) stainless steel hinged bolts with captive stainless steel wingnuts and washers shall be attached to the case with the use of stainless steel spring pins. Hence, latching or unlatching of the door shall require no tools.

F. "Z-crates" or "Egg-crates" type filters shall not be used.

G. All machine screws, studs and washers shall be stainless steel.

H. Gaskets shall conform to the provisions in ASTM D-1056, Grade RE 42.

I. The outside of the housing shall be painted in accordance with the provisions of Section 80.16, Article 16.4 – Painting for Steel Structures.

J. The housings shall accept a sixteen inch by eighteen inch (16" x 18") pedestrian module.

**Article 20.5 Light Emitting Diode (LED) Pedestrian Signal Modules**

A. General

LED traffic signal modules shall meet the current ITE standards.

B. Installation

1. LED pedestrian signal modules shall be designed as retrofit replacements for the existing pedestrian signals (ICC 4090 and/or 4094).

2. LED pedestrian signal modules shall not require special tools for installation.
3. LED pedestrian signal modules shall fit into the existing traffic housings built to the PTCSI standard without any modification to the housing.

4. LED pedestrian signal modules shall be weathertight, fit securely in the housing and shall connect directly to existing electrical wiring.

5. Installation of a replacement LED module into the existing pedestrian housing shall only require removal of the existing optical unit components, i.e., lens, lamp, gaskets, and reflector.

6. Each retrofit shall include all necessary components to complete conversion, including a one-piece gasket.

7. Each pedestrian module shall have a sticker attached stating compliance to the ITE standard for color.

C. LED Signal Lens

1. The lens of the LED pedestrian signal modules shall be field replaceable.

2. The lens of the LED pedestrian signal modules shall be polycarbonate UV stabilized and a minimum of six millimeters (6 mm) thick.

3. The exterior of the lens of the LED pedestrian signal module shall be smooth and frosted to prevent sun phantom.

D. LED Pedestrian Signal Module Construction

1. The LED pedestrian signal module shall be a single, self-contained device, not requiring on-site assembly for installation in the existing traffic signal housing.

2. All Portland Orange LEDs shall be “AlInGaP” technology or equal, and rated for 100,000 hours or more at +165°F (+25°C) and twenty milliamps (20 mA). “AlInGaS” technology is not acceptable.

3. Each individual LED traffic module shall be identified for warranty purposes with the manufacturer’s trade name, serial number and operating characteristics, i.e., rated voltage, power consumption, and volt-ampere.

E. Environmental Requirements

1. The LED pedestrian signal modules shall be rated for use in the ambient operating temperature range of -40°F to +140°F (-40°C to +60°C).

2. The LED pedestrian signal modules, when properly installed with gasket, shall be protected against dust and moisture intrusion per requirements of
NEMA Standard 250-1991, Sections 4.7.2.1 and 4.7.3.2, for Type 4 enclosures to protect all internal LED, electronic, and electrical components.

F. Luminous Intensity

1. Pedestrian LED signal modules shall be designed so, that when operated over the specified ambient temperature and voltage range, the signal shall attract the attention of, and be readable to, a viewer (both day and night) at all distances from ten feet (10') to the full width of the area to be crossed.

2. The luminous intensity of the LED pedestrian signal module shall not vary more than ±10% for voltage range of 80 VAC to 135 VAC.

G. Chromacity

The measured chromaticity coordinates of the LED signal modules shall conform to the chromaticity requirements of Section 5.3 and Figure C of PTCSI standard.

H. Electrical

1. The secured, color-coded, one meter (1 m) long, 600V, 20 AWG minimum, jacketed wires, conforming to the National Electrical Code, rated for service at +221°F (+105°C), twelve millimeter (12 mm) stripped and tinned are to be provided for electrical connection.

2. The LED pedestrian signal module shall operate from a 60 ±3Hz AC line over a voltage range of 80 VAC to 135 VAC. Rated voltage for all measurements shall be 120 ±3 volts rms.

3. The LED circuitry shall prevent perceptible flicker over the voltage range specified above.

4. The LED pedestrian signal module circuitry shall include voltage surge protection against high-repetition noise transients as stated in Section 2.1.6, NEMA Standard TS-2, 1992.

5. Catastrophic failure of one LED light source shall not result in the loss of more than the light from that one LED.

6. The LED pedestrian module shall be operationally compatible with the currently used controller assemblies. The LED pedestrian module shall be operationally compatible with conflict monitors.

7. The LED pedestrian module including its circuitry must meet Federal Communications Commission (FCC) Title 47, Subpart B, Section 15 regulations concerning the emission of noise.
8. The LED pedestrian module shall provide a power factor of 0.90 or greater over the operating voltage range and temperature range specified above for modules with six (6) watts or more.

9. Total harmonic distortion (current and voltage) induced into an AC power line by an LED pedestrian module shall not exceed twenty percent (20%) over the operating voltage range and temperature range specified above.

I. Production Testing Requirements

1. Each new LED traffic signal lamp unit shall be energized for a minimum of twenty-four (24) hours at operating voltage and at a temperature of 60°C in order to cause any electronic infant mortality to occur, and to ensure electronic component reliability prior to shipment.

2. After the burn-in procedure is completed, each LED traffic signal lamp unit shall be tested by the manufacturer for rated initial intensity at rated operating voltage.

J. Warranty

The manufacturer shall provide a written warranty against defects in materials and workmanship for the LED signal modules for a period of sixty (60) months after installation of the modules. Replacement modules shall be provided promptly after receipt of modules that have failed, at no cost to the Owner except cost of shipping the failed modules. All warranty documentation shall be given to the Engineer prior to installation. The replacement modules shall be delivered to the Traffic Signal Electronics Shop, within five (5) working days after notification. The warranty does not include the costs associated with removing and reinstalling units that are replaced or repaired.

Article 20.6 Measurement

New and relocated pedestrian signal heads will be measured as units, complete and in place, including all labor, mounting hardware, equipment, and materials to provide a complete and functioning unit. Measurement shall be for the actual number of existing pedestrian signal heads removed and relocated, and the actual number of new signal heads installed.
Article 20.7  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
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<tr>
<td>Pedestrian Signal Head</td>
<td>Each</td>
</tr>
<tr>
<td>Remove and Relocate Pedestrian Signal Head</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.21 PEDESTRIAN PUSHBUTTONS

Article 21.1 General

Where shown on the Drawings, pedestrian push buttons of substantial tamper-proof construction shall be furnished and installed. Pedestrian pushbuttons shall be ADA-compliant and either DCC-4EVR 120 (rectangle), or Bulldog RBDLM2-B-4H. Substitutions must be approved by the Traffic Engineer or designated representative.

The assembly shall be weatherproof and so constructed that it will be impossible to receive an electrical shock under any weather conditions.

Where a pedestrian pushbutton is attached to a pole, the housing shall be shaped to fit the curvature of the pole and secured to provide a rigid installation. Saddles shall be provided to make a neat fit when required.

Pushbutton and sign shall be positioned on the sidewalk or pathway side of the pole. Arrows on signs shall point in the same direction as the corresponding crosswalk. Pushbutton and sign shall not contain indicator lights unless specified on the Drawings.

Pedestrian pushbutton signs shall be as detailed in the Manual on Uniform Traffic Control Devices (MUTCD) and the Alaska Sign Design Specifications (ASDS). The R10-3B (9”X12”) pushbutton sign shall be installed above each pushbutton with the arrow pointing in the direction of the appropriate crosswalk. When channel is used for mounting pushbutton signs the top sign bolt shall be tapped into the pole.

Article 21.2 Measurement

New and relocated pedestrian pushbuttons will be measured as units, complete and in place, including all labor, equipment, signs, and other material to provide a complete and working unit. Measurement shall be for the actual number of existing pushbutton assemblies removed and relocated, and the actual number of new pushbutton assemblies installed. Pedestrian pushbutton signs shall not be measured separately, and shall be considered part of the pushbutton assembly.

Article 21.3 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
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</thead>
<tbody>
<tr>
<td>Pedestrian Pushbutton Assembly</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.22  FLASHING BEACONS

Article 22.1  General

A.  Beacons

1.  Intersection Control and Hazard Beacons

Each beacon shall consist of one or more single section traffic signal heads, in accordance with the provisions in Section 80.19 - Signal Heads, with yellow or red LED modules as shown on the Drawings.

2.  Warning Sign Beacons

Each Warning Sign Beacons shall consist of twelve inch (12”) diameter yellow LED signal indications. The number of units, unit configuration and unit installation shall be as shown on the Drawings.

3.  Speed Limit Sign Beacons

The ‘SCHOOL SPEED LIMIT 20 WHEN FLASHING’ (S5-1) sign beacon assembly shall consist of four (4) signal heads with eight inch (8”) or twelve inch (12”) diameter yellow LED signal indications as shown on the Drawings. They shall be mounted horizontally directly above the S5-1 sign. The two upper beacons shall be illuminated alternately and face oncoming traffic. The lower beacons shall face the opposite direction and be illuminated alternately also.

4.  Mast Arm Mounted Sign Beacons

Each mast arm mounted sign beacon assembly shall consist of four (4) twelve inch (12”) diameter yellow LED signal indications with backplates, and two signs of the type and size indicated on the plans. All signs and signals shall be mounted on the mast arm, with two (2) signals and one (1) sign facing each direction. Any two (2) signals facing the same directions shall be illuminated alternately. Signal heads shall meet the requirements of Section 80.19 - Signal Heads.

B.  Control Unit

1.  Flashing Beacon Control Unit

The Flashing Beacon Control Unit is to be used for the following: Intersection Control Beacons, Hazard Beacons and Crosswalk Warning Sign Beacons. Each control unit shall be a complete flasher cabinet assembly consisting of an On/Off switch, 15-ampere circuit breaker, surge protector, terminal blocks, flasher socket and NEMA flasher. The enclosure
shall be a NEMA Type 3R, and shall be provided with a right side hinged door with locking mechanism.

2. Speed Limit Sign Beacon Control Unit

Each control unit shall be a complete flasher cabinet assembly consisting of a 20-ampere circuit breaker, solid state surge protector, Radio Interference Suppressor, MOV (Metal Oxide Varistor) surge protector, thermostatically controlled incandescent cabinet light with door activated bypass switch, terminal blocks, flasher socket, NEMA flasher and digital time clock. The enclosure shall be a NEMA Type 3R, and shall be vented and provided with a right side hinged door with locking mechanism.

C. Control Unit Component Specification

Terminal blocks shall be in accordance with Section 80.17 - Controller Assemblies, except that a single three- (3-) position Box Lug type terminal block capable of accepting three (3) No. 6 AWG wires for terminating power cables must be supplied in all control units.

Switches shall be 15 ampere, single-pole, 120 volt AC.

The Metal Oxide Varistor (MOV), surge protector shall be a V130PA20A.

The cabinet light fixture shall be an incandescent type porcelain lamp holder rated for 660W-250V AC/CA. The lamp shall be 75W.

Flasher socket shall be Cinch-Jones socket S-406-SB, or equivalent.

Flasher shall be a NEMA 2 circuit, solid state, rated at 15 amperes per circuit flasher.

Digital time clock shall be an RTC AP41, or equivalent as approved by the Traffic Signal Electronics Foreman.

All other components shall meet the requirements of Section 80.17 - Controller Assemblies.

Article 22.2 Measurement

Flashing beacons and flashing beacon control units will be measured separately as units, complete and in place, including all labor, equipment, and material to provide a complete and working unit.
Article 22.3 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment shall be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Intersection Control Beacon</td>
<td>Each</td>
</tr>
<tr>
<td>Hazard Beacon</td>
<td>Each</td>
</tr>
<tr>
<td>Warning Sign Beacons</td>
<td>Each</td>
</tr>
<tr>
<td>Speed Limit Sign Beacon</td>
<td>Each</td>
</tr>
<tr>
<td>Mast Arm Mounted Sign Beacon</td>
<td>Each</td>
</tr>
<tr>
<td>Flashing Beacon Control Unit</td>
<td>Each</td>
</tr>
<tr>
<td>Speed Limit Sign Beacon Control Unit</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.23  LUMINAIRES

Article 23.1 General

Luminaires shall be high-pressure sodium, horizontal burning type as shown on the Drawings.

Article 23.2 Light Distribution

Furnish luminaires having standard I.E.S. light distribution patterns as specified in the Contract Documents.

Prior to installation, Contractor shall check the socket position in the luminaire to verify that it corresponds to the setting indicated in the instructions for the light distribution type shown on the Drawings.

Vertical light distribution shall be short (s), medium (m), or long (l).

Cutoff shall be cutoff (c), semi-cutoff (s), or non-cutoff (n).

Lateral light distribution shall be Type 1, Type 1-4 way, Type II, Type II-4 way, Type III or Type IV.

When cutoff fixtures are specified in the Special Provisions or shown on the Drawings, the optical assembly shall provide ninety-degree (90°) cutoff and shielding. The reflector shall be specifically designed to produce the specified ANSI and IES light distribution when used with one hundred fifty (150) through four hundred (400) watt high pressure sodium lamps. The fixture shall have a flat plate glass lens and no part of the lens shall project below the luminaire's metal housing.

Mast arm mounted luminaires shall be provided with slip-fitters designed for mounting on two-inch (2") standard pipe.

Contractor shall ensure all lenses are of the refractor type, and that the refractors are made of polycarbonate resin.

Contractor shall ensure the polycarbonate resin lenses are molded in a single piece. Contractor shall not use reworked compound whose properties have been impaired by previous molding operations. Contractor shall provide lenses free from cracks, blisters, burns and flow lines, furnished with the natural molded surface, uniform density throughout, free from air, gas, or moisture pockets, and uncured areas, as consistent with good manufacturing practice. Contractor shall provide transparent lenses having a clear bluish tint and produced from resin, which has been suitably ultraviolet stabilized to reduce the effects of ultraviolet radiation on their color properties. Resins used shall meet the requirements for the self-extinguishing classification of ASTM D 635. Resin shall have a minimum impact strength, Izod notched of twelve foot pounds per inch (12.0 ft. lbs./inch).
when tested in accordance with ASTM D 256, Method A, using a one-quarter by one-half inch (1/4” x 1/2") bar molded in accordance with ASTM-recommended practice.

The Contractor shall furnish a certificate of compliance from the lens fabricator that all requirements contained in the paragraph above have been met.

The refractor shall be mounted in a doorframe assembly which shall be hinged to the luminaire at the house side and fastened at the street side by means of an automatic type latch.

The refractor and doorframe assembly shall be forced upward at the street side by spring pressure, against the gasket seat, when in the closed and latched position.

All gaskets shall be composed of a material capable of withstanding the temperature involved and they shall be securely held in place.

All parts of the luminaire shall be manufactured from corrosion-resistant materials.

Manufacturer’s luminaire specifications, shop drawings, and photometric data shall be submitted and approved before installing any luminaire on the project.

**Article 23.3 Measurement**

Luminaires will be measured as units complete and in place, including all labor, equipment, and materials to provide a complete and functioning unit. No measurement for payment will be made until the functional test has been completed in accordance with Section 80.16, Article 16.2 – Field Tests.

**Article 23.4 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Luminaire (Wattage) (Vertical) (Cutoff) (Lateral)</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.24 BALLASTS

Article 24.1 General

Ballasts for high-intensity discharge lamps shall be an integral part of each luminaire and designed for the voltages and lamp types specified in the Drawings or Special Provisions. The current needed to start the lamps shall be less than the operating current.

Ballasts shall be the regulator type with copper windings electrically isolated from each other, and shall start and operate the lamps in temperatures down to -40°F. The allowable line voltage variation shall be at least plus and minus ten percent (10%).

High pressure sodium luminaires, except those with 1000 watt lamps, shall be equipped with magnetic regulator ballasts with the following additional operating characteristics:

1. The lamp wattage regulation spread at any time over the life of the lamp shall not exceed eighteen percent (18%) of nominal lamp watts at plus or minus ten percent (±10%) line voltage variations.

2. With nominal line and lamp voltages, the ballast shall regulate the lamp output to within five percent (5%) of the ballast design center, and sustain lamp operation with a minimum sixty percent (60%) voltage drop lasting four (4) seconds or less.

3. Grounded socket shell.

Luminaires with 1000 watt high pressure sodium lamps shall be equipped with auto-regulator ballasts that provide a maximum thirty percent (30%) lamp regulation spread, a minimum thirty-five percent (35%) voltage dip tolerance, and with nominal line and lamp voltages regulate lamp output to within five percent (5%) of the ballast design center.

Ballasts for use for soffit luminaires shall be furnished with mounting brackets attached and shall be equipped with terminal blocks for primary connections and lamp socket preconnected to the secondary for flush mounted luminaires and with terminal blocks for both primary and secondary connections for use with suspended luminaires.

The Contractor shall submit the ballast manufacturer's volt-watt trace and specification sheets to the Street Light Maintenance Supervisor for review and approval.

Article 24.2 Measurement

This item shall be considered incidental to other Work.

Article 24.3 Basis of Payment

No separate pay item will be allowed for this item.
SECTION 80.25    FALSEWORK LIGHTING

Article 25.1  General

When required by the Special Provisions, falsework lighting shall be installed where
vehicular traffic with or without pedestrian traffic crosses through or under structure
falsework.

Illumination of the portal faces of falsework shall be provided during the hours from dusk to
dawn. Illumination of the pavement and pedestrian openings through or under falsework
shall be provided twenty four (24) hours per day.

The Contractor shall submit a plan of the proposed lighting installations and shall not
commence falsework construction until such Drawings have been reviewed by the
Engineer. A subsequent review shall be made by the Engineer after falsework lights have
been placed in operation.

Fixtures for illumination of roadway pavement between entrances and exit portals shall be
enclosed in units with protective cover lens.

Fixture housing shall be heavy gauge, anodized aluminum and shall have an etched,
anodized aluminum reflecting surface, or equivalent. Beam spread shall be between one
hundred (100) and one hundred thirty (130) degrees.

Fixtures shall be equipped with high temperature glazed porcelain medium base sockets
and six foot (6’) conductors for splicing, approved by UL for outdoor use.

Fixtures shall be fully adjustable with bracket and locking screws on a mounting plate and
shall provide mounting directly to a standard metal junction box.

Fixtures for pedestrian passageways shall be porcelain box receptacles mounted on
standard metal junction boxes and equipped with wire lamp guards. Porcelain box
receptacles shall be rated at six hundred sixty (660) watts, two hundred fifty (250) volts.
Wire lamp guards shall be made of No. 10 AWG wire and shall be suitable for general
construction work.

Lamps shall be of the medium base incandescent type. For pavement illumination, lamps
shall be one hundred fifty (150) watts minimum. For pedestrian passageway illumination,
lamps shall be one hundred (100) watts minimum.

The face of all falsework and forms located within or adjacent to the traveled way, on the
approach side, shall be fully illuminated by a minimum of four (4) three hundred (300) watt
PAR reflector flood lights directed upon the vertical and horizontal supports. The lights
shall be located a minimum of ten feet (10’) and a maximum of fifteen feet (15’) from the
portal faces adjacent to the traveled way and mounted a minimum of twelve feet (12’) and
a maximum of fifteen feet (15’) high. When a median area contains falsework, a minimum
of one (1) additional three hundred (300) watt PAR reflector flood light shall be installed in
the same manner to illuminate the median support. Each flood light shall be aimed in such a manner as to preclude glare to oncoming motorists. Floodlights shall be mounted on temporary wood poles set in the ground and located on both sides of the traveled way.

For illumination of roadway pavement between entrance and exit portals, a continuous row of fixtures shall be installed over the center of each lane beneath the falsework structure at intervals of not more than fifteen feet (15’), with the end fixtures not further than seven feet (7’) inside the portal faces. Mounting height of fixtures over the pavement shall be as directed.

Pedestrian openings, through or under falsework, shall be illuminated with fixtures centered over the passageway at intervals of not more than fifteen feet (15’), with the end fixtures not more than seven feet (7’) inside the portal faces and at a height of ten feet (10’), unless otherwise directed.

For roadway pavement and portal face illumination, No. 12 AWG conductors with Type XHHW insulation shall be used. For pedestrian passageways, conductors shall be No. 12 AWG and enclosed in a one-half inch (1/2”) unpainted, zinc-coated metallic conduit.

Contractor shall provide two branch circuits. Pedestrian passageway lights and roadway pavement lights shall be on one (1) circuit and portal face floodlights shall be on a separate circuit. Each branch circuit shall be fused, not to exceed twenty (20) amperes.

The above-specified portal lighting shall be installed on the day that vertical supports are erected and before traffic is permitted to pass between these supports during the hours from dusk to dawn. The other falsework lights shall be installed as soon as the members on which they are to be supported are in place.

Upon completion of the project or when directed, falsework lighting equipment shall remain the property of the Contractor and shall be removed from the site of the Work.

**Article 25.2 Measurement**
Falsework lighting for each location will be measured as one lot, installed complete, including all labor, equipment, and material required to provide a complete and functioning system.

**Article 25.3 Basis of Payment**
Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Falsework Lighting</td>
<td>Lot</td>
</tr>
</tbody>
</table>
SECTION 80.26  INTERCONNECT CABINET

Article 26.1  General

Install the interconnect cable termination cabinet when called for in the Drawings or when three (3) or more interconnect cables terminate at the controller location. All cables shall be terminated in the interconnect cable cabinet and none in the controller cabinet.

Furnish an interconnect termination cabinet that:

1. Is constructed from 16 gauge galvanized steel with an ANSI 61 gray polyester powder coat inside and out.
2. Meets NEMA Standards for Type 3R enclosures.
3. Has a 16-gauge galvanized steel continuous hinge with stainless steel pin.
4. Has a cover fastened securely with captive plated steel screws.
5. Is provided with a hasp and staple for padlocking.
6. Has no gasketing or knockouts.
7. Has no ventilating cover or louvers.
8. Has a standoff mounted back panel with grounding lug assembly.
9. Provide two Type 66 B3-50 terminal blocks to Traffic Signal Electronics personnel, who will install the terminal blocks.

Article 26.2  Measurement

Interconnect cabinets will be measured as units installed complete and in place, including all labor, equipment, and material to provide a complete and functioning unit.

Article 26.3  Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Interconnect Cabinet</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.27 PROTECTIVE POST ASSEMBLY

Article 27.1 General

Protective post assembly shall be a concrete-filled Schedule 40, steel pipe installed in accordance with the appropriate Standard Details.

Article 27.2 Measurement

Protective post assembly will be measured as units installed complete and in place, including all labor, equipment, and material to provide a complete and functioning unit.

Article 27.3 Basis of Payment

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Protective Post Assembly</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION 80.28 SALVAGING ELECTRICAL EQUIPMENT

Article 28.1 General

Unless otherwise specified or shown on the Drawings or Specifications, existing electrical equipment including but not limited to: luminaires, standards, mast arms, poles, caps, handhole covers, mounting bolts, controllers, cabinets, optical detectors, signal heads, pedestrian heads, service equipment, and junction box lids shall be salvaged and delivered to the Municipality of Anchorage Electronics Warehouse or the Municipality of Anchorage Pole Yard to be placed as directed by the Traffic Engineer.

Traffic Signal Electronics personnel and Street Light Maintenance personnel will be allowed to select the equipment and pole/arm items they would like to salvage. Contractor is responsible for disposal of all remaining items. All poles and arms not selected for salvage shall have the pole plate or arm plate cut off to render the item unusable.

Contractor shall contact the Traffic Signal Electronics Shop Foreman, at 343-8355, one week prior to the tentative delivery date.

Salvaged poles and mast arms shall be stripped of all wire and hardware and any damaged areas, and exposed hole edges shall be cleaned and painted with cold galvanizing paint in accordance with Section 80.16, Article 16.3 - Galvanizing. All caps, hand-hole covers, mast arms and mounting bolts shall be returned with the pole.

Removal, wire and hardware stripping, listed cleaning and cold galvanize painting, and delivery of all salvaged electrical equipment shall be considered incidental to the Contract and no separate payment shall be made.

When a controller assembly is to be salvaged, the salvage material shall include timing modules, switches, detector control units, conflict monitor unit, and all other equipment contained in the controller cabinet prior to award of the Contract.

Care shall be exercised in removing and salvaging electrical equipment so that it will remain in its original form and existing condition. The Contractor will be required to replace, at his expense, any of the above-mentioned electrical equipment which has been damaged or destroyed by his operations.

Unless otherwise specified, underground conduit, conductors, foundations and detectors not reused shall become the property of the Contractor and shall be removed from the project right-of-way. If said materials do not interfere with other construction, they may, with approval from the Engineer, be abandoned in place except that conductors must be removed from conduit prior to abandonment. Foundations abandoned in place shall conform to the requirements of Section 80.03 – Removing and Replacing Improvements.

Holes formed by removing pull boxes and foundations shall be filled with material equivalent to the original and compacted to the same density as the surrounding material.
When existing electrical equipment is to be reused, the Contractor shall furnish and install all necessary materials and equipment, including signal mounting brackets, anchor bolts, nuts, washers and concrete as required to complete the new installation.

All traffic signal, flashing beacon and lighting fixtures to be reinstalled shall be cleaned, relamped, and reconditioned in accordance with Section 80.16, Article 16.4 – Painting for Steel Structures.

Salvaged materials required to be reused and found to be unsatisfactory by the Engineer shall be replaced by new material and the cost will be paid as extra Work as provided in Division 10, Sections 10.05 – Control of Work and 10.07 – Measurement and Payment.

**Article 28.2 Measurement**

Measurement for removal of poles in this Section is per each unit removed; and includes all work and materials necessary to remove poles, hardware disposal, cutting poles to render them unusable, disassemble, salvage, disposal, and delivery to the Municipality of Anchorage Pole Yard as specified in the Drawings or in the Special Provisions. When Drawings are unclear as to the method of pole salvage or disposal, the Contractor shall contact the Traffic Engineer to receive specific instructions.

Removal of the pole foundation, in accordance with Section 80.03 - Removal and Replacing Improvements, and disposal of the pole foundation is incidental to the pay items in this Section. Salvage and delivery of existing signs, signal hardware and illumination hardware shall also be considered incidental to the pole removal pay items.

If Owner declines ownership, the poles, mast arms, and associated hardware become Contractor property.

**Article 28.3 Basis of Payment**

Payment for this Work shall be in accordance with Division 10, Section 10.07 - Measurement and Payment, and shall include full payment for all Work described in this Section.

Payment will be made under the following units:

<table>
<thead>
<tr>
<th>ITEM</th>
<th>UNIT</th>
</tr>
</thead>
<tbody>
<tr>
<td>Remove Luminaire Pole</td>
<td>Each</td>
</tr>
<tr>
<td>Remove Signal Mast Arm or Combination Pole</td>
<td>Each</td>
</tr>
<tr>
<td>Remove Signal Pedestal or Pedestrian Pushbutton Pole</td>
<td>Each</td>
</tr>
</tbody>
</table>
MUNICIPALITY OF ANCHORAGE
STANDARD SPECIFICATIONS

DIVISION 80
TRAFFIC SIGNALS AND ILLUMINATION
STANDARD DETAILS
<table>
<thead>
<tr>
<th>Number</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>80-1</td>
<td>Saw Cut Trench</td>
</tr>
<tr>
<td>80-2</td>
<td>Concrete Foundation Load Center Type 1A</td>
</tr>
<tr>
<td>80-3</td>
<td>Concrete Foundation Load Center Type 1</td>
</tr>
<tr>
<td>80-4</td>
<td>Concrete Foundation Type 1 Load Center Section AA</td>
</tr>
<tr>
<td>80-5</td>
<td>Controller Cabinet Foundation Type “P” or “R”</td>
</tr>
<tr>
<td>80-6</td>
<td>Concrete Foundation for Type “P” or “R” Controller Section AA</td>
</tr>
<tr>
<td>80-7</td>
<td>Concrete Foundation for Type “P” or “R” Controller Section BB</td>
</tr>
<tr>
<td>80-8</td>
<td>Vault Type “M” Controller Cabinet Foundation</td>
</tr>
<tr>
<td>80-9</td>
<td>Poured Concrete Luminaire Pole Foundation</td>
</tr>
<tr>
<td>80-10</td>
<td>Poured Concrete Foundation Signal Mast Arm Pole</td>
</tr>
<tr>
<td>80-11</td>
<td>Poured Concrete Spread Footing Signal Pole Foundation</td>
</tr>
<tr>
<td>80-12</td>
<td>Concrete Foundations for Signal Pedestal Pole and Pedestrian Push Button Pole</td>
</tr>
<tr>
<td>80-13</td>
<td>Driven Steel Pile Light Pole Foundation</td>
</tr>
<tr>
<td>80-14</td>
<td>Driven Steel Pile Signal Pole Foundation</td>
</tr>
<tr>
<td>80-15</td>
<td>Pedestrian Push Button Pole</td>
</tr>
<tr>
<td>80-16</td>
<td>Signal Pedestal Pole</td>
</tr>
<tr>
<td>80-17</td>
<td>Pedestal Signal Pole Slip-Base and Adapter Detail</td>
</tr>
<tr>
<td>80-18</td>
<td>Direct Imbedded Luminaire Pole</td>
</tr>
<tr>
<td>80-19</td>
<td>Flange-Mounted Luminaire Pole</td>
</tr>
<tr>
<td>80-20</td>
<td>Luminaire Arm Detail</td>
</tr>
<tr>
<td>80-21</td>
<td>Driven Steel Pipe Slip Base Pole Assembly</td>
</tr>
<tr>
<td>80-22</td>
<td>Flange-Mounted Detail for Concrete Luminaire Base</td>
</tr>
<tr>
<td>80-23</td>
<td>Concrete Luminaire Base Slip-Base Detail</td>
</tr>
<tr>
<td>80-24</td>
<td>Signal Mast Arm Pole and Combination Signal/Luminaire Pole</td>
</tr>
<tr>
<td>80-25</td>
<td>Signal Mast Arm/Pole Design Loading</td>
</tr>
<tr>
<td>80-26</td>
<td>Details-Signal Mast Arm Pole and Signal/Luminaire Pole</td>
</tr>
<tr>
<td>80-27</td>
<td>Side-Mounted Signal Details</td>
</tr>
<tr>
<td>80-28</td>
<td>Pedestrian Push Button Assembly</td>
</tr>
<tr>
<td>80-29</td>
<td>Post Top and Mast Arm Mounted Signal Details</td>
</tr>
<tr>
<td>80-30</td>
<td>Type I Junction Box</td>
</tr>
<tr>
<td>80-31</td>
<td>Type IA Junction Box</td>
</tr>
<tr>
<td>80-32</td>
<td>Type II Junction Box</td>
</tr>
</tbody>
</table>
80-33 Type III Junction Box
80-34 Post Mounted Load Center - Type 3
80-35 Post Mounted Load Center - Type 2
80-36 Pad Mounted Load Center - Type 1A
80-37 Pad Mounted Load Center - Type 1
80-38 Load Center Wiring Diagram “A”
80-39 Panel Schedule for Wiring Diagram “A”
80-40 Load Center Wiring Diagram “B”
80-41 Panel Schedule for Wiring Diagram “B”
80-42 Load Center Wiring Diagram “C”
80-43 Panel Schedule for Wiring Diagram “C”
80-44 Load Center Wiring Diagram “D”
80-45 Panel Schedule for Wiring Diagram “D”
80-46 Load Center Wiring Diagram “E”
80-47 Panel Schedule for Wiring Diagram “E”
80-48 Load Center Wiring Diagram “F”
80-49 Panel Schedule for Wiring Diagram “F”
80-50 Load Center Wiring Diagram “G”
80-51 Panel Schedule for Wiring Diagram “G”
80-52 Conduit Encased Loop Detector
80-53 Loop Detector Installation Details
80-54 Loop Detector Home Run
80-55 Opticom Detector Installation Details
80-56 Signal Heads
80-57 Speed Limit Sign Beacon
80-58 Mast Arm Mounted Sign Beacons
80-59 Warning Sign Beacon
80-60 Splice Detail Loop Detector Leads
80-61 Signal Head Wiring Details
80-62 Interconnect Cable Termination Cabinet
80-63 Protective Post Assembly
AFTER TRENCH BACKFILL HAS BEEN COMPACTED AN ADDITIONAL 12" OF ASPHALT WILL BE REMOVED FROM EACH EDGE OF THE ORIGINAL CUT. THE ENGINEER MAY REQUIRE MORE THAN A 12" ADDITIONAL CUT IF THE EXISTING PAVEMENT HAS BEEN LIFTED IN THE REMOVAL PROCESS OR IF THE JOINT DOES NOT OCCUR ON UNDISTURBED MATERIAL.
CONFIRM METER MOUNTING HEIGHT REQUIREMENTS WITH POWER SERVICE PROVIDING UTILITY.

VARIES

66"

42" MIN

6" MIN

2" CONDUIT
TO POWER UTILITY

2" CONDUTS
TO J-BOX OR LOAD

TOP VIEW

TYPE 1A LOAD CENTER

#4 BARE COPPER GROUND WIRE BONDED TO BASE REBAR

GROUND ROD

8'-0" MIN

#6 BARE COPPER GROUND WIRE

1" PVC SCHEDULE 40

#4 REBAR

PROVIDE ANCHOR BOLTS AS REQUIRED TO MOUNT LOAD CENTER IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

12" TYPICAL

2" RMC

20" WITH 120/240 SERVICE
25" WITH 240/480 SERVICE

2 EA. 3/4" X 10' COPPER CLAD STEEL DRIVEN GROUND RODS

FRONT VIEW

SECTION AA

NOTES:
1. ORIENTATION OF CONDUIT SWEEPS IS REPRESENTATIVE. CONTRACTOR SHALL COORDINATE CONDUIT ORIENTATION WITH THE ENGINEER AND UTILITY.
2. PROVIDE NON-FROST SUSCEPTIBLE COMPACTED BACKFILL.
3. INSTALL TYPE 1A OR TYPE 2 JUNCTION BOX ADJACENT TO LOAD CENTER FOUNDATION. JUNCTION BOX SIZE TO BE DETERMINED USING THE LATEST VERSION OF MOA DESIGN CRITERIA MANUAL, CHAPTER 6.
3-#4 VERTICAL AS SHOWN
WALL HORIZONTAL STEEL W/90° HOOK
2" SERVICE ENTRANCE RMC
LOAD CENTER CABINET WALL

12" W x 15" H KNOCKOUT (TYPICAL OF 3)
INSTALL 3 EA. 3"x10" LONG RMC NIPPLES THROUGH THE BASE CAP
INSTALL NYLON BUSHINGS ON LOWER END OF NIPPLES

15" W x 12" H KNOCKOUT (TYPICAL OF 3) FOR LOAD CIRCUIT

NOTE:
1. SEE DETAIL 80-4 FOR SECTION AA.
NOTES:

1. STOP HORIZ. & VERT. STEEL AT BLOCK-OUT PANELS & USE 90 HOOK. USE 2-#4 HORIZ. & VERT. EXTRA BARS ALL SIDES AS SHOWN.

2. SEE STANDARD DETAIL 80–3 FOR PLAN VIEW.

3. ADD SECOND 3/4” x 10’ GROUND ROD 8’ FROM LOAD CENTER PER NEC.
PLAN VIEW

NOTES:
1. SEE STANDARD DETAIL 80–6 FOR SECTION AA
2. SEE STANDARD DETAIL 80–7 FOR SECTION BB.
3. ANCHOR BOLTS SHALL NOT PROTRUDE MORE THAN 1–1/2" ABOVE THE TOP OF THE FOUNDATION. USE CABINET MANUFACTURER SPECIFIED ANCHOR BOLT DIMENSIONS.
4. SEAL UNUSED CONDUIT STUBS WITH WATERTIGHT CAPS.
SECTION AA

NOTE:
SEE STANDARD DETAIL 80–7, SECTION BB, FOR REBAR DETAILS.
SECTION BB

NOTE:
STOP HORIZONTAL & VERTICAL STEEL AT THE BLOCK-OUT PANELS & THE JOINT USING 90 DEGREE HOOKS. USE 2 EXTRA #4 HORIZONTAL & VERTICAL BARS ALL SIDES AS SHOWN.
1. SEE DETAIL 80–6 FOR SECTION AA.
2. SEE DETAIL 80–7 FOR SECTION BB.
3. ANCHOR BOLTS SHALL NOT PROTRUDE MORE THAN 1–1/2” ABOVE THE TOP OF THE FOUNDATION. USE CABINET MANUFACTURER SPECIFIED ANCHOR BOLT DIMENSIONS.
4. SEAL UNUSED CONDUIT STUBS WITH WATERTIGHT CAPS.

PLAN VIEW

INSTALL 3–3” AND 1–2” RIGID METAL CONDUIT NIPPLES THROUGH THE SLAB. INSTALL NYLON BUSHINGS ON LOWER END OF NIPPLES.

3/4” ANCHOR BOLT (TYPICAL OF 2)
1” DRAIN
12” W x 15” H KNOCKOUT

15” W x 12” H KNOCKOUT (TYPICAL OF 3)
8” W x 15” H KNOCKOUT (TYPICAL OF 2)
**SECTION AA**

- 22" DIA. BOLT CIRCLE
- (4) ANCHOR BOLTS 2"x60"x6"
- FOR ARMS ≤ 30' OR
- 2"x84"x6" FOR ARMS OVER 30'.
- ANCHOR BOLTS TO BE
- THREADED A MIN. OF 8"

**BASE PLATE DETAIL**

- 22" φ BOLT CIRCLE
- 2 1/4" φ BOLT HOLE
  (LOCATED ON A SQUARE PATTERN)

**RING PLATE DETAIL**

- RING PLATE
  25" O.D. X 17" I.D. X 1" THICK

**FOUNDATION DETAIL**

- SINGLE ARMS ≤ 40', L=9'-0"
- SINGLE ARMS OVER 40' AND ≤ 60', L=12'-0"
- DUAL MAST ARM BOTH ≤ 30', L=9'-0"
- DUAL SEE NOTE 8 FOR ALL OTHERS

**NOTES:**

1. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, WITH GRADE AS SPECIFIED BY THE MANUFACTURER.
2. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 FOR DEFORMED BARS OF BILLET STEEL.
3. EACH SPIRAL REBAR UNIT WITHIN EACH FOOTING SHALL HAVE 1 1/2 TO 2 1/2 TURNS OF THE REBAR AT EACH END OF THE UNIT.
4. GALVANIZING OF ANCHOR BOLTS, NUTS, AND WASHERS SHALL CONFORM TO ASTM A153.
5. STEEL ANCHOR PLATES SHALL CONFORM TO ASTM A36.
6. TOP OF CONCRETE FOUNDATION SHALL HAVE A BROomed FINISH.
7. REFERENCE TOP BACK OF CURB IN ESTABLISHING FOUNDATION TOP ELEVATION, TOP OF FOUNDATION SHALL
   BE FLUSH WITH SIDEWALK/PAVING FINISH GRADE OR 2"-4" ABOVE FINISH GROUND ELEVATION.
8. SINGLE MAST ARMS OVER 60' OR DUAL MAST ARMS WITH ONE OR BOTH ARM(S) OVER 30' REQUIRE PROJECT
   SPECIFIC FOUNDATION DESIGN AND SHALL BE CONSTRUCTED AS SHOWN ON THE DRAWINGS.
NOTES:
1. INSTALL THE SPREAD FOOTINGS SO THE MAST ARM IS PARALLEL WITH THE 10'-0" DIMENSION.
2. CAST ALL FOOTINGS IN PLACE.
3. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, WITH GRADE AS SPECIFIED BY THE MANUFACTURER.
4. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 FOR DEFORMED BARS OF BILLET STEEL.

5. EACH SPIRAL REBAR UNIT WITHIN EACH FOOTING SHALL HAVE 1-1/2 TURNS OF THE REBAR AT EACH END OF THE UNIT.
6. GALVANIZING OF ANCHOR BOLTS, NUTS, AND WASHERS SHALL CONFORM TO ASTM A153.
7. STEEL ANCHOR PLATES SHALL CONFORM TO ASTM A36.
8. FOUNDATION BASE SHALL HAVE A BROOLED FINISH.
9. REFERENCE TOP BACK OF CURB IN ESTABLISHING FOUNDATION TOP ELEVATION. SET TOP OF FOUNDATION FLUSH WITH SIDEWALK/PAVING FINISH GRADE OR 2"-4" ABOVE FINISH GROUND ELEVATION.
10' PEDESTAL SIGNAL POLE FOUNDATION

5' PEDESTRIAN PUSH BUTTON FOUNDATION

NOTES:
1. FOUNDATION BASE SHALL HAVE A BROOMED FINISH.
2. REFERENCE TOP BACK OF CURB IN ESTABLISHING FOUNDATION TOP ELEVATION. SET TOP OF FOUNDATION FLUSH WITH SIDEWALK/PAVING FINISH GRADE OR 2"–4" ABOVE FINISH GROUND ELEVATION.
STEEL SLIP BASE PILE CAP TO MATCH LUMINAIRE BASE ABOVE. PROVIDE 1-1/2" THICK PLATE FOR LUMINAIRE POLES 40' AND LESS IN LENGTH. PROVIDE 2" THICK PLATES FOR LUMINAIRE POLES FROM 40 FEET TO 50 FEET IN LENGTH.

PROVIDE 8-3/4" DIA. HOLE IN CENTER OF STEEL PILE CAP TO ALLOW STEEL PILE TO EXTEND TO THE MID ELEVATION OF THE STEEL PLATE.

WELD IN CONFORMANCE WITH AWS D1.1 BY WELDERS CERTIFIED FOR AWS 6G QUALIFICATION TEST.

USE 3/8" WELD FOR 1-1/2" PLATE & 1/2" WELD FOR 2" PLATE

8"Ø STEEL PIPE PILE:
STEEL PIPE USED FOR PILING SHALL CONFORM TO ASTM A53, GRADE B. CONTRACTOR SHALL PROVIDE A 15' MINIMUM EMBEDMENT OR AS INDICATED IN DRAWINGS FOR PILING INSTALLED IN GRANULAR SOIL, FREE OF ORGANICS AND DEBRIS. FOR OTHER CONDITIONS, PROVIDE A FOUNDATION INVESTIGATION TO DETERMINE EMBEDMENT DEPTH AND OTHER PILE DESIGN CRITERIA.

FOR LUMINAIRE POLES 40 FEET OR LESS IN LENGTH, USE STANDARD STEEL PIPE PILES WITH A WALL THICKNESS OF 0.322".

FOR LUMINAIRE POLES BETWEEN 40 FEET AND 50 FEET IN LENGTH, USE "EXTRA STRONG" STEEL PIPE PILES WITH A WALL THICKNESS OF 0.500".
22" DIA. BOLT CIRCLE

STEEL PLATE SIZED TO MATCH SIGNAL POLE BASE ABOVE. PROVIDE 2-3/4" THICK, ASTM A36 STEEL PLATE.

STEEL PIPE PILE BELOW

10"Ø HOLE FOR CONDUITS

COMPLETE PENETRATION WELD IN CONFORMANCE WITH AWS D1.1 BY WELDERS CERTIFIED FOR AWS 6G QUALIFICATION TEST

PLAN VIEW

SIGNAL MAST ARM POLE

2" x 12" GALVANIZED ALL THREAD STEEL RODS, WITH A NUT AND WASHER ON EACH END OF THE BOLT. STEEL SHALL CONFORM TO ASTM F1554, WITH GRADE AS DETERMINED BY THE POLE MANUFACTURER.

SEE PLAN DETAIL ABOVE FOR STEEL PLATE PILE CAP DETAIL

CONDUIT AS REQUIRED

18"Ø STEEL PIPE PILE (1/2" WALL THICKNESS)

SECTION

NOTES:

STEEL PIPE USED FOR PILING SHALL CONFORM TO ASTM A53, GRADE B. PROVIDE A 25' MINIMUM EMBEDMENT OR AS INDICATED IN CONTRACT DOCUMENTS FOR PILING INSTALLED IN GRANULAR SOIL, FREE OF ORGANICS AND DEBRIS. FOR OTHER CONDITIONS, PROVIDE A FOUNDATION INVESTIGATION TO DETERMINE EMBEDMENT DEPTH AND OTHER PILE DESIGN CRITERIA.
"H" STYLE HOUSING
AMERICAN POLE STRUCTURES, INC.
MODEL H-1

NOTES:
1. SEE DETAIL 80–12 FOR FOUNDATION.
2. SEE DETAIL 80–28 FOR PEDESTRIAN PUSH BUTTON.
NOTES:
1. SEE DETAIL 80–17 FOR SLIP BASE AND ADAPTER.
2. SEE DETAIL 80–12 FOR FLANGE TYPE BASE PLATE.
3. SEE DETAIL 80–12 FOR CONCRETE FOUNDATION.
TOP SLIP BASE PLATE

BOTTOM SLIP BASE PLATE

SLIP BASE INSTALLATION

VIEW AA

VIEW BB

NOTES:
1. TORQUE 7/8" H.S. SLIP BASE PLATE BOLTS TO 800 INCH–LBS.
2. PLATES SHALL CONFORM TO ASTM A36.
3. CAST OPTION SHALL CONFORM TO ASTM A486, CLASS 90.
4. KEEPER PLATE SHALL CONFORM TO ASTM A446, GRADE A.
5. ALL WELDS SHALL CONFORM TO AMERICAN WELDING SOCIETY SPECIFICATIONS.
6. SEE STANDARD DETAIL 80–12 FOR BOLT HOLE CIRCLE PATTERN.

PEDESTAL SIGNAL POLE
SLIP-BASE AND ADAPTER DETAIL

SECTION #
80.05

DETAIL #
80–17
NOTES:

1. EXCAVATE HOLE TO A MINIMUM DEPTH OF 7'-0" BELOW FINISHED GRADE.
2. PLACE ONE FOOT (1') OF COARSE ROCK BEDDING, INSTALLED UNLESS OTHERWISE SPECIFIED.
3. THIS POLE FOR USE WITH ARMS OF 8'-0" MAXIMUM LENGTH.
4. INSTALL JUNCTION BOX BEHIND LIGHT POLE AWAY FROM TRAFFIC. OTHER LOCATIONS AS DIRECTED BY THE ENGINEER.
5. PROVIDE NON-FROST SUSCEPTIBLE COMPACTED MATERIAL AROUND POLE AS REQUIRED.

TAPPED HOLE FOR GROUND STRAP
5-1/2" x 7-9/16" REINFORCED HANDHOLE FRAME
HANDHOLE COVER 12 GA. (MIN)
STAINLESS STEEL SCREWS

DETAIL A
REMOVABLE RAINTIGHT CAP
MAST ARM AS SPECIFIED ON DRAWINGS

SEE STANDARD DETAIL 80–20 FOR ARM DETAILS

SEE DETAIL C STANDARD DETAIL 80–20

TRAFFIC SIDE OF POLE

30° (TYP) 30° (TYP)

Q LUMINAIRE ARM (PERPENDICULAR TO TRAFFIC)

14” BOLT CIRCLE

1½” R

13–3/4”

5–13/16”

10–1/8”

20–1/4”

DETAIL B

HANDHOLE. LOCATE ON DOWNSTREAM TRAFFIC SIDE OF POLE. SEE DETAIL A, STANDARD DETAIL 80–18

SEE DETAIL B THIS DRAWING STANDARD DETAIL 80–21 & 80–22

<table>
<thead>
<tr>
<th>LUMINAIRE POLE DATA</th>
<th>POLE LENGTH (H)</th>
<th>O.D. (MIN.)</th>
<th>U.S.S.</th>
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<tbody>
<tr>
<td></td>
<td>BASE</td>
<td>TOP</td>
<td>GAGE</td>
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<tr>
<td>25’ to 30’</td>
<td>8”</td>
<td>3–7/8”</td>
<td>10</td>
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<tr>
<td>+30’ to 35’</td>
<td>8–3/4”</td>
<td>3–7/8”</td>
<td>10</td>
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<td>+35’ to 40’</td>
<td>9”</td>
<td>3–7/8”</td>
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<tr>
<td>+40’ to 45’</td>
<td>9–1/2”</td>
<td>3–7/8”</td>
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<tr>
<td>+45’ to 50’</td>
<td>10”</td>
<td>3–7/8”</td>
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</table>
LUMINAIRE ARM DETAIL

TAPERED STEEL POLE WITH MAXIMUM TAPER OF 0.15" PER FOOT. END SECTION O.D. OF 2-3/8" FOR MOUNTING LUMINAIRE. STANDARD 2" PIPE EXTENSION OF 1'-0" MAXIMUM FOR 6'-10' ARMS AND 3'-0" MAXIMUM FOR 12'-15' ARMS MAY BE USED.

LENGTH | RISE
-------|-----
6'     | 1.5'
8'     | 2.2'
10'    | 2.5'
12'    | 3.6'
15'    | 4.3'
22'    | 6.0'

DETAIL C

3 EA. 3/4"-10 TAP. POLE PLATE FOR 3/4" BOLTS. BOLTS SHALL CONFORM TO ASTM A325 AND GALVANIZED IN ACCORDANCE WITH ASTM A135.

2" CHASED OUTLET FOR ELECTRICAL CONDUCTORS

DETAIL D

2" STD. PIPE

7-1/2" (MIN)

MUNICIPALITY OF ANCHORAGE

SCALE: NTS
APPROVED:
REVISED: 7/08

SECTION # 80.05
DETAIL # 80-20

LUMINAIRE ARM DETAIL

2" x 5" x 1/4" THK. GUSSET
FLANGE-MOUNTED DETAIL
FOR CONCRETE
LUMINAIRE BASE
LUMINAIRE MAST ARM
LENGTH VARIES
(6'-0" TO 22'-4")

SIGNAL MAST ARM LENGTH VARIES
SIGNAL HEAD OFFSET VARIES

MAST ARM END CAP SHALL BE ATTACHED TO MAST ARM WITH FULL PENETRATION SAFETY BOLT.

SEE STANDARD DETAIL 80-26

SEE STANDARD DETAIL 80-14 OR 80-26 AND SKIRT DETAIL BELOW

SKIRT DETAILS
(TWO REQUIRED PER POLE)

Q 0.157"Ø
HOLE (PUNCH FOR NO. 10 x 5/8" STAINLESS STEEL SELF-TAPPING SLOTTED SHEET METAL SCREWS, 4 PER POLE)

2' - 0"
MATCH BASE PLATE

10 GAGE STEEL
**MINIMUM DESIGN LOADING TABLE**

<table>
<thead>
<tr>
<th>LOAD #</th>
<th>DESCRIPTION</th>
<th>WEIGHT (LBS)</th>
<th>PROJECTED AREA (FT²)</th>
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<tbody>
<tr>
<td>1</td>
<td>SIGN</td>
<td>65</td>
<td>7.5</td>
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<tr>
<td>2</td>
<td>SIGNAL</td>
<td>80</td>
<td>14.7</td>
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<tr>
<td>3</td>
<td>SIGNAL</td>
<td>60</td>
<td>8.7</td>
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<tr>
<td>4</td>
<td>SIGN</td>
<td>80</td>
<td>26</td>
</tr>
<tr>
<td>5</td>
<td>SIGNAL</td>
<td>125</td>
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<tr>
<td>6</td>
<td>STREET NAME SIGN</td>
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<td>2.0</td>
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</table>

**NOTES:**

1. LOAD #5 SHALL BE SHAFT MOUNTED 14 FEET FROM THE BASE OF THE POLE.
2. LOAD #6 IS SHAFT MOUNTED 12 FEET FROM THE BASE OF THE POLE.
MAST ARM DATA TABLE

<table>
<thead>
<tr>
<th>MAST ARM LENGTH</th>
<th>BOLT CIRCLE</th>
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<tbody>
<tr>
<td>≤ 15’</td>
<td>12–1 1/2”</td>
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<tr>
<td>16’ to 26’</td>
<td>13–1 1/2”</td>
</tr>
<tr>
<td>27’ to 39’</td>
<td>15”</td>
</tr>
<tr>
<td>40’ to 45’</td>
<td>15–1 1/2”</td>
</tr>
<tr>
<td>46’ to 55’</td>
<td>18–1 1/2”</td>
</tr>
<tr>
<td>55’ to 65’</td>
<td>19”</td>
</tr>
</tbody>
</table>

INSTALL 1 1/2" ASTM A325 BOLTS ON A SQUARE PATTERN

3" MIN. DIA. HOLE

SEE THE SIGNAL MAST ARM DATA TABLE FOR BOLT CIRCLE DIAMETER

PLAN VIEW OF MAST ARM

PLAN VIEW

(Shown without anchor bolts and nuts for clarity)

NOTES:
1. SEE STANDARD DETAIL 80–24 FOR SIGNAL MAST ARM POLE AND SKIRT DETAILS.
2. SEE STANDARD DETAILS 80–10, 80–11 & 80–14 FOR FOUNDATION DETAILS.
USE A ONE WAY, L.O.D. FRAME FOR INSTALLING ONE FACE

USE A TWO WAY FRAME FOR INSTALLING TWO FACES

USE TWO FRAMES FOR INSTALLING THREE FACES: A TWO WAY AND A ONE WAY R.O.D.

ELBOW SET SCREWS TO OUTSIDE
CONDUIT HANGAR CLAMP

LEFT OF DOOR (L.O.D.)
FRONT OF DOOR (F.O.D.)
RIGHT OF DOOR (R.O.D.)
BACK OF DOOR (B.O.D.)

PEDESTRIAN SIGNAL

HEAD NO. 1 OFFSET L.O.D.
HEAD NO. 2 OFFSET R.O.D.

NOTE: SHOWN WITHOUT BACKPLATES
NOTE:
TAP TOP AND BOTTOM SIGN ATTACHMENT WITH STAINLESS STEEL BOLTS INTO THE POLE.

3/8"x16x1-1/4" STAINLESS STEEL HEX HEAD CAP SCREW

1/2"x1-1/4" CHASE NIPPLE

PEDESTRIAN SIGNAL AND CLAM SHELL BRACKET ON THE FAR SIDE OF THE POLE

START CROSSING
Watch For Vehicles

DON'T START
Finish Crossing If Started

TIME REMAINING
To Finish Crossing

STEADY

DON'T CROSS

TO CROSS
PUSH BUTTON

R10-3E
SIGN DETAIL

PEDESTRIAN HARDWARE

PEDESTRIAN PUSH BUTTON
ASSEMBLY

PEDESTRIAN PUSH BUTTON SWITCH
PELCO "ASTRO BRACKET" CLAMP KIT WITH PLUMBERIZER MOUNT (PART NO. AB-3007-L) INSTALL WITH STAINLESS UPGRADE OPTION (L INDICATES THE LENGTH OF THE STRAPS)

3/8" STAINLESS STEEL BOLT WASHERS, AND NUTS

ELEVATOR PLUMBIZER

2" GALVANIZED RIGID METAL CONDUIT

ELEVATOR STRAP

MAST ARM

SERRATED FITTING

4-1/2" SLIP-FITTER

TWO ROWS OF THREE SQUARE HEAD SET SCREWS

SLIP FITTER

4-1/2" SLIP-FITTER

TWO ROWS OF THREE SQUARE HEAD SET SCREWS

TERMINAL COMPARTMENT WITH SLIP FITTER

POST TOP MOUNTED SIGNAL DETAILS

(SHOWN WITHOUT BACKPLATES)

POST TOP AND MAST ARM MOUNTED SIGNAL DETAILS

MUNICIPALITY OF ANCHORAGE

SCALE: NTS
APPROVED:
REVISED:
10/07

SECTION # 80.19
DETAIL # 80-29
EMBOSSED WITH "ELECTRIC" OR "LIGHTING" AS REQUIRED

9 GA. WELDED WIRE FRAME

30" MIN

GROUNDING BUSHINGS

6" MIN

#8 BARE CU. BONDING WIRE

3/4"x10' COPPER CLAD GROUND ROD (AS REQUIRED)

CONDUIT SIZES AND NUMBER AS REQUIRED

1/2" DRAIN HOLE OR TEE DRAIN WITH APPROVED FILTER CLOTH MATERIAL

STONE DRAIN

6" MIN

5" MIN

6" MIN

12"

8-7/8" 14-1/4"

10-1/2"

17-1/2"

4"

19-3/4"

14-5/8"

3-1/2"

12"

3"

ATTACH GROUND BRAID TO LID USING STAINLESS STEEL NUT AND BOLT

-3' COPPER BOND BRAID W/EYELETS @ 6" INTERVALS
DETAIL A

NOTE: REINFORCEMENT MAY CONSIST OF:
1. 9 GAGE WELDED WIRE FRAME.
2. 3–6 GAGE HORIZONTAL WIRE LOOPS.
3. SYNTHETIC FIBER REINFORCED CONCRETE
   THAT MEETS ASTM C 1116 AND CONTAINS
   FIBER IN PROPORTIONS AS RECOMMENDED
   BY THE FIBER MANUFACTURER.

2 KNOCKOUTS CENTERED ON ONE SIDE
1 1/2" DEEP x 3" HIGH x 7" WIDE
FOR LOOP DETECTOR INSTALLATION

ATTACH GROUND BRAID TO LID USING
STAINLESS STEEL NUT AND BOLT

4' BOND BRAID
W/EYELETS AT 6"

SEE DETAIL A

GROUNDING
BUSHING

#8 BARE CU.
BONDING WIRE

3/4"x10' COPPER CLAD
GROUND ROD (AS REQUIRED)

CONDUIT SIZES AND
NUMBER AS REQUIRED

STONE
DRAIN

1/2" DRAIN HOLE OR TEE DRAIN
WITH APPROVED FILTER CLOTH MATERIAL

SECTION A–A
**TYPE II
JUNCTION BOX**

**PLAN**

- **TRAFFIC**: Embossed with "TRAFFIC" or "LIGHTING" as required.

**SECTION**

- **ATTACH GROUND BRAID TO LID USING STAINLESS STEEL NUT AND BOLT**
- **6’ COPPER BRAID W/EYELETS @ 6” INTERVALS**
- **GROUNDING BUSHING**
- **#8 BARE CU. BONDING WIRE**
- **3/4”x10’ COPPER CLAD GROUND ROD (AS REQUIRED)**
- **CONDUIT SIZES AND NUMBER AS REQUIRED**
- **STONE DRAIN**
- **1 1/2” DRAIN HOLE OR TEE DRAIN WITH APPROVED FILTER CLOTH MATERIAL**

**DIMENSIONS**

- 25” MIN. (Horizontal)
- 6” (Vertical)
- 6” (Vertical)
- 22” MIN. (Horizontal)
- 6” (Horizontal)
PLAN

- EMBOSSED WITH "TRAFFIC" OR "LIGHTING" AS REQUIRED

SECTION

- ATTACH GROUND BRAID TO LID USING STAINLESS STEEL NUT AND BOLT
- 6" BOND BRAID W/EYELETS @ 6" INTERVALS
- GROUNDING BUSHING
- #8 BARE COPPER BONDING WIRE
- STONE DRAIN
- 3/4"x10' COPPER CLAD GROUND ROD (AS REQUIRED)
- 1/2" DRAIN HOLE OR TEE DRAIN WITH APPROVED FILTER CLOTH MATERIAL

CONDUIT SIZES AND NUMBER AS REQUIRED

MUNICIPALITY
OF ANCHORAGE

SCALE:
NTS
APPROVED:
REVISED:
5/08

SECTION #
80.08
DETAIL #
80-33

TYPE III
JUNCTION BOX
NOTES:

1. COORDINATE WITH SERVING UTILITY REGARDING SPECIFIC CONSTRUCTION REQUIREMENTS FOR SERVICE.

2. SET THE BUTT END OF TYPE 3 LOAD CENTER POLES TO THE FOLLOWING MINIMUM DEPTH:
   A. 10 PERCENT OF ITS LENGTH PLUS 2 FEET, OR 5 FEET, WHICHEVER IS GREATER, IF IT IS INSTALLED IN EARTH OTHER THAN SOLID ROCK OR MUSKEG.
   B. 10 PERCENT OF ITS LENGTH, OR 4 FEET, WHICHEVER IS GREATER, IF IT IS INSTALLED IN SOLID ROCK.

3. WHENEVER MORE THAN TWO FEET OF EARTH OVERLAYS ROCK, OR THE DIAMETER OF THE DRILLED HOLE IN ROCK EXCEEDS TWICE THE DIAMETER OF THE POLE AT THE GROUND LINE, CONSIDER THE INSTALLATION AS EARTH.

C. CONSIDER MUSKEG TO BE AIR, AND SET THE BUTT ENDS TO THE DEPTH GIVEN IN A OR B, WHICHEVER APPLIES, IN THE UNDERLYING EARTH OR ROCK.
METER BASE/BREAKER COMBINATION OR SEPARATE METER/DISCONNECT AND DISTRIBUTION SECTION SIZED TO ACCOMMODATE EQUIPMENT INDICATED ON APPROPRIATE ELECTRICAL DIAGRAM. METER FACE EXPOSED OUTSIDE CABINET.

GROUND LUG

6"x8" COATED PENTACHLORPHENOL TREATED WOOD POST OR 4" RIGID STEEL CONDUIT WITH CAP OR STREET LIGHT POLE

#6 BARE COPPER GROUND WIRE

2 EA. 3/4" x 10' COPPER CLAD GROUND RODS (8' MIN. SEPARATION)

2" RMC FOR SERVICE

GRADE AWAY WITH 3% MINIMUM SLOPE

NOTES:

1. ATTACH CONDUITS TO POLE AT 24" INTERVALS.
2. ATTACH GROUND WIRE TO POLE AT 12" INTERVALS.
3. ON STEEL POST, ENCLOSE GROUND WIRE IN 3/4" RMC, BOND EACH END OF CONDUIT TO GROUND WIRE.
4. EMBED LOWER 42" OF STEEL POST IN CONCRETE HAVING A MINIMUM 18" OVERALL DIA. BACKFILL AROUND WOOD POST WITH N.F.S. MATERIAL.
5. LOCATE OUT OF DITCH LINE, 5 FT. MINIMUM FROM BACK OF CURB.
6. COORDINATE WITH SERVING UTILITY REGARDING SPECIFIC CONSTRUCTION REQUIREMENTS FOR SERVICE.
1. METERING SECTION  
2. LOAD SECTION  
3. UTILITY CONNECTION AND TEST BLOCK SECTION  
4. METER READING WINDOW (8"x8")  
5. METER SOCKET W/BYPASS & SAFETY SOCKET  
6. LIFT AWAY METER SECTION COVER  
7. DEADFRONT  
8. STAINLESS STEEL PIN HINGE  
9. PADLOCKING PROVISIONS  
10. DISTRIBUTION PANEL W/ MAIN BREAKER  
11. ACCESSORY MOUNTING EQUIPMENT AREA  
12. SERVICE PULL SECTION.

REAR VIEW  
(W/ DOOR REMOVED)  

15" WITH 120/240 SERVICE  
20" WITH 240/480 SERVICE  

SIDE VIEW  
(W/ METER SECTION OPEN)  

15"  

FRONT VIEW  
(W/ DOOR REMOVED)  

17"  

49"

42.5"
HINGED DEMAND RESET COVER

TEST-BYPASS FACILITIES

CUSTOMER SECTION

42" MIN. 64" MAX.

4 1/2" TYP.

3" MIN.

2" MIN. 5" MAX.

SUPPORT GRADE

11"

METER SOCKET

6" MAX.

LANDING LUGS AND FACTORY CONDUCTORS

17" MIN.

PULL SECTION AND COVER

LOAD LINE

METER SECTION BARRIER TO EXTEND TO EDGE OF TEST BLOCK BARRIER

TEST BYPASS COVER

TEST BYPASS SUPPORT

LIFT AWAY METER SECTION COVER

FRONT VIEW

SIDE VIEW

VIEWING WINDOW

2" MINIMUM DIMENSIONS

HINGED DEMAND RESET WITH POLYCARBONATE VIEWING

MINIMUM DIMENSIONS

<table>
<thead>
<tr>
<th>SERVICE</th>
<th>W</th>
<th>A</th>
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<tbody>
<tr>
<td>1 PHASE</td>
<td>10-1/2&quot;</td>
<td>10&quot;</td>
</tr>
<tr>
<td>3 PHASE</td>
<td>12-1/2&quot;</td>
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</table>
NOTE:
1. SEE PANEL SCHEDULE, STANDARD DETAIL 80–39
LOAD CENTER NO. _______ TYPE ____________________________
LOCATION ____________________________

240/480 VOLTS, SINGLE PHASE, _________ AMP SUPPLY
________________________________ AMPS INTERRUPTING CURRENT

MAIN BREAKER A: 2 POLE, _________ AMPS, 480 VOLTS
MAIN BREAKER B: 2 POLE, _________ AMPS, 240 VOLTS

CONTACTOR RATING: _________ AMPS
TRANSFORMER RATING: 120/240–240/480, _________ KVA

### PANEL A

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<th>AMP</th>
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**NOTE:** SEE CONSTRUCTION DRAWINGS FOR NUMBER & SIZE OF BREAKERS.

**MUNICIPALITY OF ANCHORAGE**

**SCALE:** NTS

**APPROVED:**

**REVISED:** 10/07

**SECTION # 80.14**

**DETAIL # 80–39**

**PANEL SCHEDULE FOR WIRING DIAGRAM *A***
NOTE:
1. SEE PANEL SCHEDULE, STANDARD DETAIL 80–41
LOAD CENTER NO. __________ TYPE __________________________
LOCATION __________________________

120/240 VOLTS, SINGLE PHASE, __________ AMP SUPPLY
_________________________ AMPS INTERRUPTING CURRENT

MAIN BREAKER A: 2 POLE, ___________ AMPS, 240 VOLTS
MAIN BREAKER B: 2 POLE, ___________ AMPS, 480 VOLTS

CONTAC TOR RATING: ___________ AMPS,

TRANSFORMER RATING: 120/240–240/480, ___________ KVA

### PANEL A

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<th>CKT. DESCRIPTION</th>
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NOTE: SEE CONSTRUCTION DRAWINGS FOR NUMBER & SIZE OF BREAKERS.
NOTE:
1. SEE PANEL SCHEDULE, STANDARD DETAIL 80-43
LOAD CENTER NO. _______ TYPE ____________________________
LOCATION ___________________________________________

120/240 VOLTS, SINGLE PHASE, _______ AMP SUPPLY
_____________________________________________ AMPS INTERRUPTING CURRENT
MAIN BREAKER A: 2 POLE, _______ AMPS, 240 VOLTS
MAIN BREAKER B: 2 POLE, _______ AMPS, 240 VOLTS
CONTACTOR RATING: _______ AMPS, 240 VOLTS

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NOTE: SEE CONSTRUCTION DRAWINGS FOR NUMBER & SIZE OF BREAKERS.

---

MUNICIPALITY OF ANCHORAGE

SECTION # 80.14

PANEL SCHEDULE FOR WIRING DIAGRAM "C"

DETAIL # 80–43
NOTE:
1. SEE PANEL SCHEDULE, STANDARD DETAIL 80-45
LOAD CENTER NO. __________ TYPE __________________________
LOCATION ________________________________
240/480 VOLTS, SINGLE PHASE, __________ AMP SUPPLY
_________________________ AMPS INTERRUPTING CURRENT
MAIN BREAKER A : 2 POLE, __________ AMPS, 480 VOLTS
MAIN BREAKER B : 1 POLE, ________15____ AMPS, 240 VOLTS
CONTACTOR RATING: __________ AMPS, 240 VOLTS

<table>
<thead>
<tr>
<th>PANEL A</th>
<th>240/480 VOLTS</th>
<th>SINGLE PHASE</th>
<th>3 WIRE</th>
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<tr>
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<td>AMPS INTERRUPT CAPACITY</td>
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NOTE: SEE CONSTRUCTION DRAWINGS FOR NUMBER & SIZE OF BREAKERS.
NOTE:
1. SEE PANEL SCHEDULE, STANDARD DETAIL 80-47
LOAD CENTER NO. _______ TYPE: __________________________
LOCATION: _________________________________________

120/240 VOLTS, SINGLE PHASE, ________ AMP SUPPLY
_____________________ AMPS INTERRUPTING CURRENT
MAIN BREAKER A: 2 POLE, __________ AMPS, 240 VOLTS
MAIN BREAKER B: 2 POLE, _______ 15 ______ AMPS, 240 VOLTS
CONTACTOR RATING: _______________ AMPS

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NOTE: SEE CONSTRUCTION DRAWINGS FOR NUMBER & SIZE OF BREAKERS.
NOTE:
1. SEE PANEL SCHEDULE, STANDARD DETAIL 80–49
LOAD CENTER NO. ________TYPE: ________________________
LOCATION: ____________________________________________
________POLE, ________ AMP CONTACTOR

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120/240 VOLTS SINGLE PHASE 3 WIRE
____________________ AMPS MAIN LUGS, _______________ AMPS INTERRUPT CAPACITY

NOTE: SEE CONSTRUCTION DRAWINGS FOR NUMBER & SIZE OF BREAKERS.
NOTE:
1. SEE PANEL SCHEDULE, DETAIL 80–51
LOAD CENTER NO. _____________ TYPE: _______________________
LOCATION: ____________________________

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120/240 VOLTS SINGLE PHASE 3 WIRE

AMPS MAIN LUGS, _______________ AMPS INTERRUPT CAPACITY

PANEL SCHEDULE FOR
WIRING DIAGRAM 'G'
TYPICAL SAW CUT OUTLINE, WHEN INSTALLING LOOPS IN EXISTING PAVEMENT THAT WILL BE OVERLAID

1" PVC CONDUIT (SCHEDULE 80)
6" MINIMUM (TYP. ALL SIDES)

3' MIN.

EDGE OF PAVEMENT

CONDUIT TO CONTROLLER

JUNCTION BOX

HOT DIPPED GALVANIZED STEEL TYPE "X" CONDUIT OUTLET BODY OR CONDULET WITH ACCESS SIDE UP

TOP VIEW

FINISHED GRADE

PAVEMENT

LEVELING COURSE

1" PVC CONDUIT (SCHEDULE 80)

BOTTOM OF LEVELING COURSE

1" CONDULET-X

CONDUIT COVER

1" PVC CONDUIT (SCHEDULE 80)

#14 AWG CONDUCTOR (CONFORMING TO IMSA 51-5)

SIDE VIEW

USE 4 TURNS OF A SINGLE PIECE CONDUCTOR IN ALL LOOPS

WND TAIL AT 3 TWISTS PER FOOT MINIMUM TO JUNCTION BOX

LOOP WIRING DETAIL

MUNICIPALITY
OF ANCHORAGE

SCALE:
NTS

APPROVED:

REVISED:
5/08

SECTION #
80.18

DETAIL #
80-52

CONDUIT ENCASED LOOP DETECTOR
TYPICAL LOOP SETBACKS

MEASURE THE SETBACKS FROM THE REFERENCE LINE ALONG THE CENTER OF EACH LANE

NOTES:

1. NO MINIMUM CLEARANCE IS REQUIRED BETWEEN A DETECTOR LOOP AND HOMERUN LOOP WIRES OR BETWEEN HOMERUN LOOP WIRES. HOMERUN LOOP WIRES SHALL NOT CROSS LOOP CONDUITS.

2. WHERE EXISTING PAVEMENT WILL NOT BE OVERLAID, ENCLOZE ALL LOOPS THAT ENTER A COMMON JUNCTION BOX WITHIN A TRAPEZOIDAL SAW CUT. CUT TO WITHIN 1 FOOT OF THE LANE AND EDGE LINES, PRESERVING THESE PAVEMENT MARKINGS; REMOVE THE ASPHALT TO THE LIP OF THE GUTTER WHEN THERE ARE NO EDGE LINES. CUT ACROSS LANE LINES WHEN LOOPS IN ADJACENT LANES ARE SIDE BY SIDE. CUT TRENCHES A MINIMUM OF 3 FEET WIDE WHEN INSTALLING LOOP TAILS ACROSS A LANE; CUT TRENCHES A MINIMUM 1 FOOT WIDE WHEN CROSSING A SHOULDER.
NOTES:

1. SEE THE SIGNAL PLANS FOR THE SIGNAL MAST ARMS SCHEDULED FOR OPTICOM DETECTOR INSTALLATION.

2. FOR EACH OPTICOM INSTALLATION, FURNISH THE FOLLOWING PARTS:
   A. A 3M MODEL 711, 721, OR 722 OPTICOM DETECTOR AS SHOWN ON THE PLANS.
   B. A 3M MODEL 575 CONFIRMATION LIGHT KIT, OR AN APPROVED EQUAL, THAT CONSISTS OF STEEL PARTS WITH A HOT DIP GALVANIZED FINISH.
   C. A 3/4”x6” LONG PIPE NIPPLE AND A 3/4” 90° PIPE ELBOW. FURNISH BOTH PARTS WITH A HOT DIPPED GALVANIZED FINISH.
   D. THREE ADDITIONAL 3/4” LOCKNUTS WITH ZINC PLATED FINISH.
   E. AN ASTRO-MINI-BRAC, MODEL AB-0155-L, MANUFACTURED BY PELCO PRODUCTS, OR AN APPROVED EQUAL.
   F. A 60 WATT, PAR 38, HALOGEN FLOOD LAMP RATED FOR 130 VOLT OPERATION, 1150 INITIAL LUMENS, AND A 3000 HOUR LAMP LIFE.

3. DRILL A 1" HOLE IN THE TOP DEAD CENTER OF THE MAST ARM AT THE OPTICOM DETECTORS PRE-APPROVED LATERAL LOCATION. ASSEMBLE THE PARTS AS SHOWN ON THIS SHEET.

4. BEFORE ATTACHING THE MODEL 138 DETECTOR CABLE TO THE OPTICOM DETECTOR, STRIP THE INSULATION FROM THE THREE INSULATED CONDUCTORS AT THE CONTROLLER CABINET AND ATTACH ALL FOUR CONDUCTORS TO GROUND.
5-LENS HEAD

3-LENS HEAD

4-LENS HEAD

BACKPLATE

BACKPLATE

BACKPLATE

NOTES:
1. SIGNAL FACE DIMENSIONS ARE 8” OR 12” AS SPECIFIED IN THE DRAWINGS.
2. PROVIDE LEFT/RIGHT ARROW INDICATIONS, AS INDICATED IN THE CONTRACT DOCUMENTS.
3. ALL VEHICLE SIGNALS SHALL HAVE BACKPLATES.
SCHOOL FLASHER NOTES:

1. EACH FLASHER SHALL CONSIST OF FOUR 12-INCH SIGNAL FACES WITH YELLOW LENSES AND TUNNEL TYPE VISORS WITH OPEN SLOTS AT THE BOTTOM.

2. THE CONTRACTOR SHALL WIRE SIGNAL FACES 1 AND 2 ON FLASHER CIRCUIT 1 AND SIGNAL FACES 3 AND 4 ON FLASHER CIRCUIT 2.

3. BEACON FRAMEWORK

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<tr>
<th>ITEM NO.</th>
<th>IDENTIFICATION</th>
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<tbody>
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<td>A</td>
<td>1-1/2&quot; 90° SERRATED ELBOW</td>
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<td>B</td>
<td>1-1/2&quot; TEE</td>
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<tr>
<td>C</td>
<td>1-1/2&quot; x VARIES NIPPLE</td>
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<tr>
<td>D</td>
<td>1-1/2&quot; x VARIES NIPPLE</td>
<td>2</td>
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<tr>
<td>E</td>
<td>1-1/2&quot; LOCK NIPPLE</td>
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4. YELLOW SIGNAL FACE:
   12" (POSTED SPEEDS ≥ 40 MPH)
   8" (POSTED SPEEDS ≤ 30 MPH)
   FOR 35 mph SIGNAL FACE SIZE REQUIRES DISCRETION OF ENGINEER.

SEE DETAIL 80-17 FOR SLIP BASE
SEE DETAIL 80-12 FOR FLANGE TYPE BASE PLATE
NOTES:
1. PROVIDE ALL SIGNAL HEADS WITH 5" BACKPLATES.
2. CONTACT MOA SIGN SHOP FOR MOUNTING METHOD.
3. PROVIDE MINIMUM VERTICAL CLEARANCE OF 18' FROM ROADWAY TO BOTTOM OF SIGNAL HEAD HOUSING OR BOTTOM OF SIGN.
4. WIRE SIGNAL FACES 1 AND 2 ON FLASHER CIRCUIT 1 AND SIGNAL FACES 3 AND 4 ON FLASHER CIRCUIT 2.
5. EACH UNIT SHALL CONSIST OF FOUR 12-INCH AMBER L.E.D. SIGNAL FACES AND TUNNEL TYPE VISORS WITH OPEN SLOTS AT THE BOTTOM.
12-INCH YELLOW SIGNAL FACE WITH YELLOW LENS AND TUNNEL TYPE VISOR WITH OPEN SLOT AT THE BOTTOM

1-1/2” x VARIES NIPPLE

OFFSET POST TOP SLIP FITTER

BACK VIEW OF A MUTCD TYPE SIGN. SIZE AND TYPE AS INDICATED ON DRAWINGS.

10’ PEDESTAL POLE (SEE DETAIL 80–16)
SEE DRAWINGS FOR BASE TYPE

SEE DETAIL 80–17 FOR SLIP BASE
SEE DETAIL 80–12 FOR FLANGE BASE

7’ MIN.

WARNING SIGN BEACON
MATERIAL PROPERTIES

LOOP LEAD-IN SPLICED

<table>
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<tr>
<th>TUBING</th>
<th>2&quot; CORE FLOW</th>
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<tr>
<td>CAP SEAL</td>
<td>FERNCO QWIK CAP #QC–102</td>
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<tr>
<td>HOSE CLAMP</td>
<td>STAINLESS STEEL</td>
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<td>SPLICE CONNECTOR</td>
<td>MULTILINK ML56–16 OR APPROVED EQUAL</td>
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<tr>
<td>COMPOUND</td>
<td>RE–ENTERABLE ENCAPSULATION</td>
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</table>

NOTES:
1. FABRICATE LOOP LEAD-IN SPLICED IN THE FIELD AS SHOWN.
2. CAP SEAL ONE END AND COMPLETELY FILL OPEN END WITH RE–ENTERABLE ENCAPSULATION COMPOUND TO EDGE OF CORE FLOW.
3. LEAVE A MINIMUM OF 1/2" CLEARANCE BETWEEN THE ENCLOSURE AND THE SPLICE AT BOTH ENDS OF THE CORE FLOW.
4. EXPOSED FOIL AND DRAIN WIRES, SEAL WITH HEAT SHRINK TUBING (TYP).
5. SECURE CABLE/CONDUCTOR BUNDLE WITH NYLON CABLE TIES.
5 SECTION HEAD

7-CONDUCTOR, 14 GAUGE CABLE (7C#14) WITH HEAD IDENTIFICATION BAND

COMMON WHITE
SPARE WHITE/BLACK

RED
ORANGE
BLUE
GREEN

PEDESTRIAN HEAD

5-CONDUCTOR, 14 GAUGE CABLE (5C#14) WITH HEAD IDENTIFICATION BAND

COMMON ORANGE BLACK
SPARE WHITE

RED
GREEN

WHITE
ORANGE
BLACK

3 SECTION HEAD

7-CONDUCTOR, 14 GAUGE CABLE (7C#14) WITH HEAD IDENTIFICATION BAND

COMMON WHITE/BLACK
SPARE BLACK

RED
ORANGE
GREEN

WHITE
ORANGE
BLUE

MUNICIPALITY OF ANCHORAGE
SCALE: NTS
APPROVED: 5/08
REvised:

SECTION # 80.19 & 80.20
DETAIL # 80-61

SIGNAL HEAD WIRING DETAILS
INSTALL THE CABINET VERTICALLY, SO THE NIPPLE IS LOCATED BETWEEN THE TOP SHELF AND ROOF.

INTERCONNECT TERMINATION CABINET WITH NOMINAL DIMENSIONS OF 16" W X 20" H X 6" D FOR TYPE M CABINET OR 24" W X 24" H X 8" D FOR TYPE P OR TYPE R CABINET

TWO 1−5/8" GALVANIZED UNISTRUT CHANNELS BOLTED TO FOUNDATION

1−3" RIGID METAL CONDUIT FOR 3 CABLES, 2−3" RIGID METAL CONDUITS FOR 4 OR MORE CABLES

NOTES:

1. INSTALL THE INTERCONNECT CABLE TERMINATION CABINET WHEN CALLED FOR IN THE DRAWINGS OR WHenever 3 OR MORE INTERCONNECT CABLES ARE TO BE TERMINATED.

2. COORDINATE INSTALLATION LOCATION OF 1/2" NIPPLE WITH MOA TRAFFIC SIGNAL ELECTRONICS, PRIOR TO DRILLING HOLE IN CABINETS.

3. SEE DRAWINGS TO DETERMINE WHETHER TO INSTALL ON RIGHT OR LEFT SIDE WALL OF CONTROLLER CABINET.

4. PROVIDE TWO TYPE 66B3−50 TERMINAL BLOCKS TO THE MOA FOR INSTALLATION BY OTHERS.
NOTES:

1. PROVIDE 6" STEEL, SCHEDULE #40 PIPE, FILLED WITH CONCRETE.
2. ROUND CONCRETE AT TOP OF POST SMOOTH AND PAINT YELLOW.
3. INSTALL 4–2" BANDS OF YELLOW REFLECTIVE TAPE AS SHOWN.
4. LOCATION AND QUANTITY OF POSTS AS INDICATED ON DRAWINGS.